

Revised Dewatering Management Plan (Temporary Dewatering for Construction)

Newcastle High School Upgrade 25a National Park Street, Newcastle West

Prepared for Hansen Yuncken Pty Ltd

Project 213618.04

15 March 2024



Douglas Partners Pty Ltd ABN 75 053 980 117 douglaspartners.com.au 15 Callistemon Close, Warabrook, NSW 2304 (02) 4960 9600

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The undersigned, on behalf of Douglas Partners Pty Ltd, confirm that this document and all attached drawings, logs and test results have been checked and reviewed for errors, omissions and inaccuracies.

Signature		Date	
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Revised Dewatering Management Plan (Temporary Dewatering for Construction) Newcastle High School Upgrade 25a National Park Street, Newcastle West

1. Introduction

This revised dewatering management plan has been prepared for temporary dewatering for construction for the Newcastle High School Upgrade at 25a National Park Street, Newcastle West. The investigation was commissioned in an email dated 12 February 2024 by Robert Petersen of Hansen Yuncken Pty Ltd (HY) and was undertaken with reference to Douglas Partners Pty Ltd (Douglas) proposal 213618.04.P.002.Rev1 dated 12 February 2024.

This revised DMP provides methods and strategies for temporary dewatering for construction. This revised DMP supersedes the September 2023 DMP prepared by Douglas (Douglas, 2023h) and has been updated:

- Following detailed design and further review of the extent of excavations required by the Construction Contractor (HY) which has resulted in an overall reduction in dewatering requirements;
- In response to the following consent conditions related to dewatering (State Significant Development Application No. SSD-41814831):
 - B32. Prior to the commencement of construction, the Dewatering Management Plan prepared by Douglas Partners dated September 2023 shall be updated in consultation with the Department (DPE Water).
 - B33. The Applicant shall submit the revised required under B32 to the Planning Secretary for approval, together with suitable evidence the updated Dewatering Management Plan meets the requirements of the Department (Water).
- In response to the following consent conditions related to dewatering:

Details of the consultation with NSW Department of Environment Water (DPE Water) in the preparation of this revised DMP is included in Section 3.2.

It is understood that the proposed development comprises a new learning hub building (three-storey), new multipurpose hall, demolition and relocation of some existing structures, ancillary works and implementation of a landscaping strategy for continued secondary school use. Localised and temporary dewatering may be required for installation and/or removal of services up to 3 m where dewatering is unavoidable, depending on groundwater levels at the time. Further details are provided in Section 5.



This revised DMP follows on from previous reports by Douglas for the proposed upgrade:

- Report on Mine Subsidence Assessment (Douglas, 2022a) and Mine Subsidence Investigation (Douglas, 2022b).
- Report on Infiltration Testing (Douglas, 2023a);
- Report on Preliminary Site Investigation and Detailed Site Investigation (Douglas, 2023b);
- Report on Geotechnical Investigation (Douglas, 2023c);
- Desktop Review Soil and Water (Douglas, 2023d);
- Acid Sulfate Soil Management Plan (Douglas, 2023e).
- Remediation Action Plan (Douglas, 2023f);
- Mine Bore Backfill and Grouting (Douglas, 2023g);
- Report on Preliminary Contamination Testing for Borrow Pit and Capping Feasibility (Douglas, 2023i).
- Investigation summary Report on Geotechnical Testing for Proposed Borrow Pit and Capping Material (Douglas, 2023j);
- Memorandum on Results of Groundwater Level Monitoring (Douglas, 2024a).

Relevant information obtained from the previous reports has been utilised in preparation of this DMP, including a seven-week period of groundwater level monitoring which was undertaken to further assess groundwater level conditions (refer Section 4.3).

This DMP includes the following:

- Review of DPE Water comments on the project EIS provided by the client (refer Appendix D);
- Desktop review/summary of previous reports by Douglas including environmental setting, hydrogeological setting, groundwater level and quality data;
- Review of the proposed development and comments on anticipated dewatering requirements;
- Estimates of groundwater inflow rates and volumes for dewatering during construction as a daily inflow rate;
- Comments on the possible dewatering strategies, methods and timing;
- Development of a construction monitoring program including recommendations for test parameters, frequency, sample collection and handling procedures, contingency measures and reporting.

This DMP has been prepared with reference to the following guidelines:

• DPE (2022a). Guidelines for Groundwater Documentation for SSD/SSI Projects. Technical guideline. NSW Department of Planning and Environment;



• DPE (2022b). Minimum requirements for building site groundwater investigations and reporting. NSW Department of Planning and Environment.

2. Site Description

Site Address	25a National Park Street, Newcastle West, NSW
Legal Description	Part Lot 1 Deposited Plan (D.P) 150725
	Part Lot 1 D.P. 575171
	Part Lot 1 D.P. 794827
Area	Site investigation area approximately 21,700 m ² (2.17 ha) – red in Figure 1
	Total area of above lots (overall school) approximately 46,000 m ² (4.6 ha) – yellow in Figure 1. Refer Drawing 1 in Appendix E.
Zoning	Zone R2 Low density residential
Local Council Area	Newcastle City Council
Current Use	Secondary (high) school
Surrounding Uses	North / north-east:
	• Fearnley Dawes Athletic Centre (private recreational field);
	Merewether Scout Hall
	North-east / east:
	• Public netball courts and playing fields (National Park No 5 and 6 Sportsground)
	 Private recreation (Wanderers Rugby Club and National Park No 2 Sportsground
	South-east, south, west and north-west:
	Residential





Figure 1: Newcastle High School boundary in yellow, and site investigation boundary 'the site' in red.

3. **DPE Water**

3.1 EIS Comments (August 2023)

The September 2023 DMP (Douglas, 2023h) was required in response to DPE Water review of the Environmental Impact Statement (EIS) under SSD application No. SSD-41814831. DPE Water comments on the project EIS provided by the client are included in Appendix D.

The key points raised prior to approval and the relevant sections in this revised DMP which have addressed these items are summarised in Table 1.

Table 1: Summary of Water NSW Pre Approval Comments

ltem	Relevant report section
A dewatering management plan is required that details:	This document
• The maximum depth of excavation for construction	Section 5.2 (Table 4)
• The maximum annual volume of water take due to aquifer interference activities required for the project.	Section 6.2



It is noted that DPE also requested that the DMP "should also specifically nominate a drained method for the proposed development". The DMP relates to temporary dewatering for construction. Whilst subsurface installations are required for services, footings, infiltration pits (refer Section 5), permanent excavations for drained or fully tanked basements and the like <u>are not proposed</u>.

3.2 Consultation on Revised DMP (March 2024)

The Draft B version of this revised DMP was provided to DPE Water for review in March 2024. In a response letter dated 15 March 2024, DPE Water advised their acceptance of the revised DMP with no further actions required. A copy of the response is included in Appendix D.

4. Data Summary

4.1 Environmental Setting

Site Topography	Site levels range from about RL 4.5 (AHD) on the southern and western parts to about RL 2.5 on the eastern site of the lot. The land falls gently to the north-east for most of the site, which terraces down to the lowest areas in the north-east near the northern lot boundary.
Regional Topography	Regionally, topography is relatively flat in the order of RL 4 (AHD) to RL 6 with locally lower areas, typically in drainage canals.
Geology	 Reference to the NSW Seamless Geology mapping indicates the site is underlain by the following: Clastic sediment (QP_u) in the southern and central portion of the site which typically comprises clay, silt and marine sand; Anthropogenic deposits (Q_h) in the northern portion of the site which typically comprises anthropogenic fill; and Alluvial floodplain deposits (QH_af) in the north western portion of the site which typically comprises silt, sand and clay.



Soil landscape	Reference to the NSW Soil Landscape index indicates the site is underlain by soils typical of the Hamilton soil landscape. General limitations of this soil landscape include wind erosion, groundwater pollution, strong acidity and non-cohesive soils.
Acid Sulfate Soils (ASS)	The site is mapped as a low probability occurrence of ASS greater than 3 m below the ground surface. ASS typically occur at levels of RL 5 or below, but typically at elevations less than 1 AHD in coastal environments. ASS testing has been undertaken at the site by Martens (2021) which was limited to depths of 5.6 m and 8.3 (RL -4.3 to 2.9). The testing indicated net acidity results did not exceed the action criteria. Because proposed CFA piles extending below the depth of investigation by Martens, Douglas prepared an ASS management plan (Douglas, 2023e) which detailed treatment of ASS below 8.3 m depth (approximate RL-4.3) as a precautionary measure. The ASSMP also included provisions for ASS monitoring for any site dewatering which have the potential to lower the groundwater table. It is noted that Douglas conducted further ASS testing in conjunction with contamination testing which confirmed the presence of ASS conditions below RL -4 requiring management. Update of the ASSMP was recommended to confirm liming rates and procedures. The recommended management option, as detailed in the ASSMP, is neutralisation of ASS by full lime treatment and pH adjustment of waters, if required.
Surface Water	There are no permanent or ephemeral surface water bodies located on the site. An unnamed water course is located to the north (off-site) and comprises a concrete lined stormwater channel/drain. The mapping is based on data by NSW Department of Lands (2006) and indicates the feature is a "natural water course" and is "non perennial" (ie surface water is not likely to be present at all times). Review of the NSW Department of Environmental and Industry web mapping portal indicates that the stormwater channel is classified as First Order Strahler stream. This indicates that the stream is at the top of the catchment and would flow to Cottage Creek Stormwater Channel (Second Order stream) located about 200 m east of the site, which then drains to Throsby Creek and ultimately the Hunter River (estuarine environment).



is likely.

	2023)
	This watercourse is expected to be a freshwater environment and receives some runoff from the school campus. Whilst the stream / stormwater channel does not continue further upgradient of the site, it is expected that the stream / stormwater channel receives stormwater runoff from areas to the north, west and south-west which are upgradient
	of the site via stormwater infrastructure.
Groundwater	A search of the publicly available registered groundwater bore database indicated a number of registered groundwater bores within a 500 m radius of the site. These bores are reviewed in detail in Douglas Report on Detailed Site Investigation (DSI) (Douglas, 2023b). In summary, most bores are registered for 'water supply' and are listed as functioning. The installation depths are generally shallow (typically < 7 m depth) and would be expected to be screened within the alluvial water bearing zone. The majority of bores were installed in the 1980s or prior and it would be expected that they may have been for domestic irrigation which was common at this time. A number of bores were also registered for 'irrigation' for playing fields in the local area (north / north-west). Based on the urban setting, availability of potable sources and

information published in the Water Sharing Plan for the North Coastal Sands Groundwater Sources 2016, potable use of water in the local area is expected to be unlikely. Beneficial use of the groundwater for irrigation

Figure 3: Hydroline spatial data mapping for under Water Management (General) Regulation 2018 (Source DPIE Web App, 22 Feb 2023)

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Water Management (General) Regulation 2018 hydroline spatial data 1.0

Hydroline



	ConstantCo
Groundwater	A review of the Bureau of Meteorology Groundwater Dependant
Dependant	Ecosystems Atlas indicates that there are no mapped aquatic,
Ecosystems (GDE)	subterranean or terrestrial GDEs within 1 km of the site.
	comprises a moderate potential for terrestrial GDE (South Coast Sands Dry Sclerophyll Forests).
	No springs were listed within 500 m of the NSW Hydrography Spatial
	Data site (The Central Resource for Sharing and Enabling Environmental Data in NSW, SEED).
Salinity	A review of NSW Government online eSPADE mapping indicates limited
	salinity data within the Newcastle area. Mapping indicates one data point approximately 400 m north of the site as "no salting evident"
	A review of the Australia Dryland Salinity Assessment Spatial Data
	(1:2,500,000) - NLWRA 2001 indicates the site lies outside a mapped high
	located approximately 7 km away.
	Groundwater field testing of monitoring wells and the adjacent surface
	water drain as part of the DSI (Douglas, 2023b) indicated electrical
	data (ie EC 243 to 642 μ S/cm).
Applicable	North Coast Coastal Sand Groundwater Sources 2016 Water Sharing Plan
Sharing Plan	Area (Hawkesbury to Hunter Coastal Sands Groundwater Source).
	1



4.2 **Previous Investigations**

4.2.1 Martens - Geotechnical Investigation (Martens, 2021)

A geotechnical investigation by Martens Consulting Engineers (Martens) included drilling of 11 bores to depths up to 9 m, collection of soil samples for ASS and geotechnical testing for the proposed development. Relevant ASS findings are summarised in Section 4.1.

Pertinent results from this investigation include:

- Subsurface conditions at the site consist of:
 - o Fill (mainly sand) to depths ranging between 0.2 m and 2.5 m; underlain by
 - Alluvial soils initially comprising sand which transitioned into sandy clay below about 7.5 m depth.
- Deepest fill was observed in the north-eastern portion of the site;
- Groundwater was encountered at depths ranging between 2.0 m and 5.6 m bgl.

It is noted that the groundwater depth range of 2 m to 5.6 m bgl correlate to RL -2.6 to 2 AHD based on the interpolated survey levels on logs. It is noted that groundwater levels recorded during drilling are not always representative of stabilised conditions. Some of the levels reported by Martens (2021) are likely to be spurious based on subsequent monitoring by Douglas as summarised below.

4.2.2 Douglas – Geotechnical Investigation (Douglas, 2022a)

Douglas has undertaken a geotechnical and mine subsidence investigations at the site. These included seven cone penetration tests (CPTs) to depths ranging between 12.34 m and 32.10 m, shallow bores ranging between 1.1 m and 2.2 m and deeper bores drilled into abandoned mine workings up to 55 m.

Pertinent findings include the following:

- Subsurface conditions at the site consists of mainly sandy fill up to 1.2 m depth overlying alluvial soils to approximately 30 m depth. The alluvial soils consisted of predominately sand with a clay layer at about 6 m to 8 m depth. The sand layer continued to depths of 12.3 m to 14.4 m and was underlain by a layer of clay to the top of weathered rock at depths of approximately 29 m to 35 m;
- Bores confirmed the presence of abandoned mining within the Borehole Seam at a depth of approximately 55 m;
- Deepest fill was observed in the north-eastern portion of the site; and
- Groundwater was encountered at depths ranging between 0.5 m and 2.5 m. These groundwater levels may not have reflected actual groundwater levels as the groundwater level measurements were undertaken during or shortly after drilling / testing and groundwater levels may not have had sufficient time to stabilise when the measurement was completed.



4.2.3 Douglas – Infiltration Testing (Douglas, 2023a)

Preliminary infiltration testing was undertaken at the location of three proposed stormwater infiltration systems to assess subsurface conditions, the permeability of soils and their suitability for stormwater infiltration. It is noted that infiltration pit locations were relocated from the initial testing locations.

The investigation comprised drilling of three boreholes (Bores 201 to 203) and in-situ testing comprising constant head permeameter and double ring infiltrometer tests. Borehole logs and in situ testing reports are included in Appendix A, with the test results summarised in Table 2 and Table 3.

Bore	Test Range (m bgl)	Dominant Material Type	Saturated Hydraulic Conductivity (m/sec)	Saturated Hydraulic Conductivity (m/day)
201	0.1 to 0.5	Fill / silty sand	1.9 x 10⁻⁵	1.65
202	0.1 to 0.5	Fill / silty sand	1.8 x 10 ⁻⁵	1.5
203	0.1 to 0.5	Fill / gravelly sand and sand	2.7 x 10⁻⁵	2.3

Table 2: Summary of Constant Head Permeameter Testing (Douglas, 2023a)

Note to Table:

m bgl – metres below ground level

Table 3: Summary of Double Ring Infiltrometer Tests (Douglas, 2023a)

Bore	Depth (m bgl)	Dominant Material Type	Saturated Hydraulic Conductivity (m/sec)	Saturated Hydraulic Conductivity (m/day)
201	0.3	Fill / silty sand	9.9 x 10⁻⁵	21
202	0.3	Fill / silty sand	1.1 x 10 ^{-3*}	240*
203	0.3	Fill / gravelly sand and sand	1.5 x 10 ⁻⁴	32

Note to Table:

m bgl – metres below ground level

* Refer to comments below

The results of the double ring infiltration testing at Bore 202 (0.3 m) was unusually high and inconsistent with the other results and is high for a typical silty sand. It is possible that water within the double ring was leaking from the system resulting in an abnormally high result.



Douglas (2023a) noted that the proposed infiltration systems could result in localised shortterm mounding of the groundwater following periods of rainfall. The systems were expected to have negligible effect on the regional groundwater levels and adjacent structures such as the stormwater channels, provided the systems are designed for appropriate infiltration rates and have sufficient redundancy to account for factors such as long-term clogging. Further design advice on infiltration rates is presented in Douglas report (Douglas, 2022b).

4.2.4 Douglas – Preliminary and Detailed Site Investigation (Contamination) (DSI) (Douglas 2023b) and Remediation Action Plan (Douglas, 2023f)

Douglas completed a concurrent PSI and DSI for contamination which included a desktop/site history review, site inspection, and extensive subsurface investigation comprising 86 soil testing locations (pits / boreholes/surface samples), six groundwater monitoring wells, two surface water locations, landfill gas screening and laboratory testing. Contamination reporting was subject to a statutory Audit review by NSW EPA Auditor Fiona Robinson.

The PSI/DSI identified fill to depths of 0.1 m and 3.1 m, and deepest in the north-eastern part of the site associated with historic fill placement.

The investigations concluded that remediation and/or management of the site will be required to address the heavy metals, TRH, BTEX, PAH and asbestos impacted soil/fill at the site. A Remediation Action Plan (Douglas, 2023g) was prepared for on-site management / capping of contamination as the most practical remediation approach (ie capping of contamination beneath new buildings / permanent pavements and landscaped areas). It is noted that location of proposed stormwater infiltration pits was relocated to areas which would allow infiltration into natural ground, rather than deep fill.

Generally low-level contaminants were identified in groundwater, however, groundwater remediation was not considered to be required. Groundwater was not suitable for ongoing irrigation at the school.

The results of groundwater quality testing from the PSI/DSI are discussed in Section 7 with regards to dewatering disposal options.

4.2.5 **Douglas - Mine Bore Backfill and Grouting (Douglas, 2023g)**

Deep bores drilled to the underlying mine workings (Bores 1 to 6) were grouted to the surface in April and July 2023 to seal the potential pathway for potential hazardous ground gases (HGG) to reach the surface. The work meets the recommendations of the DSI (Douglas, 2023g) and RAP (Douglas, 2023f) and the requirements of the appointed Site Auditor.

4.2.6 Douglas – Borrow Pit Feasibility (Douglas, 2023i; 2023j; 2024a)

Douglas conducted further preliminary contamination testing, geotechnical testing and groundwater level monitoring in November 2023 to January 2024 for proposed excavation of borrow pit(s) in the central area of the site. The pits were proposed for sourcing of capping material and to allow placement / capping of on-site contaminated materials,

proposed borrow pit locations are shown on Drawing 1 in Appendix F.



including topsoil, to minimise off-site waste disposal and imported fill requirements. The

The preliminary contamination and geotechnical testing indicated the general suitability of re-use of natural materials (underneath upper fill) as capping in other parts of the site, subject to segregation and validation as per the RAP (Douglas, 2023f). Some restrictions on re-use of the material were noted regarding geotechnical properties. From a practical perspective, it was recommended that the base level of the borrow pits remains above the long term / average groundwater level in order to allow for excavation in sandy materials and appropriate reinstatement with respect to geotechnical requirements (refer Douglas (2023j)).

The results of groundwater level monitoring for a seven-week period from 27 November 2023 to 16 January 2024 are shown on Figure F1 in Appendix D and discussed in Section 4.3 below.

4.3 Summary of Groundwater Levels

Douglas

As reported in Douglas' *Desktop Review - Soil and Water* report (Douglas, 2023d), the sandy soils in the upper 12 m to 14 m of the alluvium are considered to represent an unconfined aquifer and therefore water levels in these layers are expected to fluctuate with climatic and drainage conditions. The deeper clay layer is considered to represent the base of the unconfined aquifer and therefore the total unconfined aquifer thickness at this site is about 12 m to 14 m (including the unsaturated zone above the measured groundwater level). In this regard, a continuous thin (<2.2 m) clay layer was encountered in each CPT within the sand layer. Previous investigations completed by Douglas outside of the school site suggests that the clay layer was discontinuous but for the purposes of this specific site, the thin clay layer represents the base of the unconfined aquifer. The top of this clay layer was encountered at RL-3.3 to RL-4.1 AHD.

A summary of groundwater levels is provided in Table C1 in Appendix C and included data from:

- Martens (2021) from bores / test pits;
- Douglas in 2022 and 2023 from bores / test pits;
- Douglas in 2023 and 2024 for groundwater monitoring wells.

Groundwater level monitoring using automated dataloggers was undertaken in three wells near and downgradient of the proposed borrow pits (Wells 402, 404 and 405) from 27 November 2023 to 16 January 2024. Groundwater levels versus rainfall are shown on Figure F1 in Appendix C. Contours from 23 March 2023 from Douglas (2023e) are shown in Figure 5.





Figure 5: Inferred groundwater contours – 23 March 2023 (Douglas, 2023b)

It is noted groundwater levels recorded during drilling or from CPTs are not always representative of stabilised conditions and levels in monitoring wells would generally be considered more accurate. The levels reported by Martens (2021) which are 0 AHD or less are likely to be spurious, and as a minimum would be expected > RL 0.5.

It is noted that rainfall over the seven-week datalogger monitoring period (27 November 2023 to 16 January 2024) was minimal with a maximum daily rainfall of 24.8 mm on 27 November 2023. A minor groundwater level increase of about 0.05 m was recorded for all loggers in response to the rainfall event. Levels declined slowly and this trend occurred for most of the overall monitoring period due to the low frequency and magnitude of the daily rainfall events.

A review of the overall groundwater level monitoring data indicated the following:

- North-easterly groundwater flow direction and a 0.7 m to 0.8 m gradient change across the site which was consistent for each monitoring event in 2022, 2023 and 2024;
- Groundwater levels have generally been RL 2 AHD or less which corresponds to:
 - Minimum 2 m below ground level for the southern parts of the site which are more elevated and have ground levels at about RL 4;
 - About 0.8 to 1 m below ground level for the northern parts of the site which are more low lying and have ground levels at about RL 2;

• The groundwater levels recorded by the automated data loggers were within range reported in the previous assessments, exclusive of the spurious levels noted above.

It is noted that the monitoring to date has not established long-term/average groundwater level across the site, however, indicate that it would be reasonable to assume that groundwater levels at the site could be in the order of RL 1.8 to 2 considering average climatic conditions.

Based on the data collected to date and experience in the area, groundwater could be expected to rise to about ground level for the lower parts of the site to around RL 2 to 2.8 in periods of high and/or sustained rainfall such as encountered in the 2022 investigations where climatic conditions were considered wetter than average. It is possible that groundwater levels could be at or near the ground surface during periods of excessive rain or flood events. The time for groundwater levels to return to average conditions would vary and could be in the order of days or weeks.

It is noted that neither of the above groundwater levels are considered a design groundwater level as defined in Structural Design Actions Standard AS1170.1, 2002.

5. **Proposed Development**

5.1 **Overview**

It is understood that the development at the Newcastle High School (Newcastle Education Campus) seeks update and to provide improved facilities to meet the educational needs of staff and students. The upgrades will cater for a total student population of 1420 and will include the following scope:

- Demolition of eight (8) existing buildings;
- Construction of a new three (3) storey learning hub located on the southwestern corner of the campus, including a new library, canteen, covered outdoor learning area (COLA), support learning unit, general learning spaces, hospitality teaching spaces, and science labs;
- Construction of a new multi-purpose facility (MPF) located in the north-eastern corner of the campus including a gymnasium, stage, fitness lab, flexible learning spaces, outdoor courts, and end-of-trip (EOT) facilities;
- Internal refurbishment works within the existing administration building on Parkway Ave to form a new student hub;
- New student entry from Parkway Avenue;
- Relocation of Block H approximately 50 m south;
- Ancillary works to enable the proposed upgrades and include new civil infrastructure and a comprehensive landscaping strategy.



Plans of the proposed development and drainage plans are shown in Appendix E. It is understood that the works will be undertaken in five stages as shown on the Staging Site Plan in Appendix E.

5.2 Earthworks and Dewatering Requirements

The final bulk earthworks plan (Appendix E) indicates substantial cuts (4342 m^3) and fill (4607 m^3) for an overall balance of fill at 265 m^3 . It is understood that this plan has considered the capping requirements outlined in the RAP. The north-eastern part of the site will remain at similar levels for flooding requirements.

Further review of the required civil and subsurface works were undertaken by HY (Construction Contractor) to assist with refinement of dewatering requirements. HY provided the following information:

- Infiltration pits are required to be above the water table, meaning the system and associated civil pipe runs will not be expected to encounter groundwater based on interim groundwater level advice provided by Douglas (Douglas, 2023i);
- Borrow pits will not be excavated below the groundwater table;
- Service trenches and pits are anticipated to be above the water table;
- The sewer mains connection for new installations is expected to be above the groundwater table;
- The sewer holding/pumping tanks is under design review to be above the water table, with a contingency for possible localised minor dewatering;
- Removal of existing in-ground services may require excavations up to 2 m to 3 m depth, depending on installations, and may require localised dewatering.

It is understood that during or following periods of high or above average monthly rainfall and /or known elevated groundwater level conditions (i.e. > RL 1.8 to 2.0), HY will implement the following strategies:

- A. Review excavation requirements to assess whether excavation/dewatering can be delayed to when groundwater levels return to more average conditions, therefore negating/minimising the need to dewater. Delays could typically be in the order of days or weeks, subject to conditions;
- B. Implement the strategies/procedures in this DMP where localised / temporary dewatering is unavoidable.

Based on the information provided by the contractor and the supplied plans, site excavation, depths and dewatering requirements are summarised in Table 4.



Table 4: Summary of Site Excavation and Dewatering Works

ltem	Description	Anticipated Depth (m bgl)	Dewatering Requirements
General shallow excavations for site redevelopment	Stripping of topsoil, excavation of fill / soils for shallow footings for buildings / structures, excavation of fill for placement of required capping layers for on site management of contamination (refer RAP (Douglas, 2033f))	Bulk cuts will typically be between 0.25 m and 0.75 m depth.	Not expected, unless in periods of inclement weather / flooding and elevated groundwater table. Contractor to implement Strategy A above.
Borrow pits for sourcing of capping material and capping	Stripping and segregation of topsoil, fill and underlying natural soils above the groundwater table. Placement / capping of on-site contaminated materials back into the excavation (refer RAP (Douglas, 2033f)).	Up to about 2.0 to 2.3 m, depending on groundwater level. Base of borrow pits to remain above groundwater table.	Not required.
Continuous flight auger (CFA) piles and/or screw piles	Proposed for the MPF building in the north- eastern corner	Depths of 8 m to 10.5 m below ground level (up to approx. RL -8.5 RL) for CFA or screw piles	Piling method (if adopted) not expected to require dewatering.
Infiltration pits	Excavation of underground infrastructure pits	Depths of up to 1.4 m to be founded into natural soils. These pits vary in size (12 x 40 m, 9 x 31 m, 3 x 27 m long)	Pits are designed to be installed above the groundwater table. Contractor to implement Strategy A above.
Installation of general service pits and trenches	Pits and trenches for underground service installations including drains.	Trenches in general can be up to about 2 m depth subject to specific service requirements. HY has advised that service depths for Stages 1 to 3 have been reviewed and are expected to be above the groundwater table level (RL 1.8 to 2.0).	Not expected, unless in periods of inclement weather / flooding and elevated groundwater table. Contractor to implement Strategy A above. Implement procedures in this DMP for localised dewatering where unavoidable (Strategy B).



ltem	Description	Anticipated Depth	Dewatering
item	Description	(m bgl)	Requirements
Removal of existing inground services	Removal of services including sewer lines e.g. sewer under Building B	Excavations possibly up to about 2 m to 3 m depth, depending on service depth.	Contractor to implement Strategy A above. Implement procedures in this DMP for localised dewatering where unavoidable (Strategy B).
Installation of sewer connections – National Park Street	Excavation for sewer connection to an invert level of RL 1.435 based on drainage plan.	Approximate excavation depth of 2.6 m to achieve invert.	Invert is close to or slightly below groundwater table based on monitoring to date (RL 1.4-1.6). HY have indicated that connection will be coordinated after a period of no rain to reduce the likelihood of works intersecting the groundwater table. Localised dewatering may be required as a contingency (several days duration only).
Installation of sewer holding/pump tank – MPB in north-eastern part of site.	Excavation for sewer holding tank to RL 2.17 based on drainage plan.	Approximate excavation depth of 2.6 m subject to tank size.	HY have indicated that the excavation depth will be dependent upon tank size. The tank size is to be designed to be above the groundwater table. HY have indicated that minor localised dewatering may still be required in for the tank as a contingency (several days duration only)

Note to Table:

m bgl – metres below ground level

It is noted that basements or other permanent excavations are not proposed which would require permanent or long-term dewatering at the site (i.e. "drained method" as defined by Water NSW).

The requirements for dewatering will depend on groundwater level conditions at the time, which are affected by climatic conditions and soil permeability, confirmation of service installation depths and the staging of the works. As noted above, it is understood that the site excavations would be minimised and following periods of prolonged dry weathered to reduce the likelihood of intersecting the groundwater and the need for dewatering.



6. **Groundwater Inflow Estimates**

Douglas

6.1 **Conceptual Groundwater and Model**

As noted in Section 5.2, dewatering is proposed to be avoided / minimised by delaying works during or following periods of high or above average rainfall or where groundwater levels are elevated (i.e. > RL 1.8 to 2.0). Localised dewatering may be required as outlined in Table 4 in Section 5.2 where localised / temporary dewatering is unavoidable.



Based on the previous subsurface investigations, the following preliminary conceptual hydrogeological (CHM) model has been prepared for the purposes of estimating groundwater inflows, in the instance that dewatering is required. The various layers are illustrated in Figure 6. Estimated ranges of hydraulic conductivities presented in Table 5 based on measured parameters and published literature by Fetter (2001) and experience with similar soils.



Figure 6: CPT plot showing cone resistance compared to elevation (AHD)



Unit	Material Type	Description	Level at Top of Unit (AHD)	Estimated Permeability Range (m/s)
1	Fill	Variable (sand, silty sand, clayey sand, gravelly sand, sandy gravel, silt, clay, silty clay) but predominately sand / silty sand	2.5 to 4.5 [3.0]	Modelled as same properties as Unit 2.1 sand.
2.1	Sand / Silty Sand	and / Silty Sand Found in most test locations beneath fill		2 x 10 ⁻⁵ to 3 x 10 ⁻⁵
3.1	Clay / Sandy Clay / Clayey Sand	CPTs 101 to 107	-3.2 to -3.8 [-3.5]	Modelled as impervious ie base of unconfined aquifer

Table 5: Soil Units and Hydraulic Conductivity Parameters

Notes to table:

Numbers in [x] are the adopted soil layer in the groundwater inflow model (refer Section 6.2)

The deeper Unit 2.2 sand has not been considered in the model as all in ground service excavations will be in the Unit 1 or Unit 2.1 sand and the flow estimates will be governed by the unconfined aquifer thickness extending to the Unit 3.1 clay unit.

6.2 Inflow Estimates

Estimates of inflow have been based on radial flow into a circular excavation in an unconfined aquifer which provides inflow from both sides of the excavation as shown in Figure 7.





Figure 7: Radial unconfined flow into a circular excavation

The following assumptions have been made regarding site excavations:

- Average typical groundwater level of 1.8 AHD for modelling purposes;
- The excavations for would be up to 2 m wide (radius, R=1 m) and for the following two scenarios:
 - Case 1 dewatering to a depth of 0.5 m below the groundwater table (corresponding to RL 1.3) and within the Unit 2 sand / silty sand;
 - Case 2 dewatering to a depth of 1 m below the groundwater table corresponding to RL 0.8 and within the Unit 2 sand / silty sand.
- The excavation would be dewatered to the level of the base of the excavation to the above RLs indicated above;
- Excavations would be supported by a temporary shoring wall system that would not provide an impermeable barrier along the side wall of the excavation;
- A lower bound (LB) permeability of $k_{sat} = 2 \times 10^{-5}$ m/s for Unit 2.1 (water bearing zone);
- An upper bound (UB) permeability of $k_{sat} = 3 \times 10^{-5}$ m/s for Unit 2.1 (water bearing zone);
- A vertical to horizontal permeability (k_y/k_x) ratio of 1.0;



- The boundary conditions for the model were set to a distance of 20 m from the centre of the excavation (R) for long term steady state dewatering. The boundary conditions were modelled as 3 m to represent short term dewatering;
- Aquifer thickness of 6 m (to RL -3.5).

The estimates of inflow rates are summarised in Table 6.

Table 6: Estimated inflow into circular excavation

Case	Description	Estimated Inflow Volume (Short Term < 1 week)		Estimated Inflow Volume (Long Term – Steady State)	
		m³/day	ML/day	m³/day	ML/day
1	Drawdown of groundwater table by 0.5 m (RL 1.3)	25 to 35	0.025 to 0.035	10 to 15	0.010 to 0.015
2	Drawdown of groundwater table by 1 m (RL 0.8)	50 to 70	0.050 to 0.070	15 to 30	0.015 to 0.030

The modelling indicates that inflows and dewatering volumes per day are expected to be minor for both cases and the permeability ranges. Given the duration of dewatering, where required, may be in the order of days only, dewatering is expected to be significantly less than 3 ML /year which would trigger a Water Access Licence (WAL) under the Water Management Act 2000.

It is considered that the works may meet exemption requirements for construction projects that require temporary dewatering where water take is less than 3 ML/year.

7. **Groundwater Quality and Disposal Options**

Groundwater quality sampling was undertaken from six monitoring wells (Bores 401 to 406) in March 2023 for the PSI/DSI (Douglas, 2023e). The adjacent surface water drain was also tested at two locations (SW1 and SW2). Testing was undertaken for the following identified potential contaminants, rather than a broad assessment of water quality parameters for groundwater disposal assessment:

- Dissolved metals (groundwater only) arsenic (As), aluminium (Al), beryllium (Be), boron (B), cadmium (Cd), chromium (Cr), cobalt (Co), copper (Cu), lead (Pb), manganese (Mn), mercury (Hg), molybdenum (Mo), nickel (Ni), silver (Ag), selenium (Se), zinc (Zn);
- Total metals (groundwater only) as above;



- Benzene, toluene, ethylbenzene, xylene (BTEX);
- Total recoverable hydrocarbons (TRH);
- Polycyclic aromatic hydrocarbons (PAH);
- Volatile organic compounds (VOC);
- Per- and polyfluoroalkyl substances (PFAS).

The results are shown in Tables B1 in Appendix B against the applicable fresh water discharge criteria. Field results and observations during sampling are shown in Table B2.

The testing indicated the general absence of gross contamination, however, several contaminants exceeded the adopted site criteria for fresh water systems summarised in Table 7. The remainder of analytes not listed were all below the adopted site assessment criteria (SAC).

Analyte	Analyte SAC Groundwater Lo		Surface Water Locations
Dissolved arsenic	ANZG fresh water DGVs	402	-
Dissolved copper	ANZG fresh water DGVs	401, 402, 404,405, 405	SW1 SW2
Dissolved copper	ANZECC (2000) Irrigation Long Term	GW1	-
Total copper	ANZG fresh water DGVs	Not tested	SW1, SW2 (all locations)
Dissolved manganese	ANZECC (2000) Irrigation Long Term	401	-
Dissolved nickel	ANZG fresh water DGVs	403	-
Dissolved zinc	ANZG fresh water DGVs	401, 402, 404,405, 405,GW1 (all locations)	SW1, SW2 (all locations)
Total zinc	ANZG fresh water DGVs	Not tested	SW1, SW2 (all locations)
Anthracene and Benzo(a)pyrene (PAH)	ANZG fresh water DGVs	402, 404	-
Phenanthrene (PAH)	ANZG fresh water DGVs	404	-
PFAS	PFAS NEMP 2020 Freshwater 99% LOP	401 to 406 (All locations except GW1)	SW1, SW2 (all locations)

Table 7: Summary of Groundwater Results and Surface water Results that Exceededthe SAC

Notes to table:

DGV – default guideline value

LOP – level of protection

PFAS – per- and polyfluoroalkyl substances (PFAS) SAC – site assessment criteria

ANZG (2018) Australian and New Zealand Guidelines for Fresh and Marine Water Quality.

NEMP (HEPA (2020) PFAS National Environmental Management Plan (NEMP).

ANZECC (2000): Australian and New Zealand Guidelines for Fresh and Marine Water Quality.



GROUNDED EXPERTISE

Water requiring disposal needs to be managed and disposed of in accordance with the requirements of the *Protection of the Environment Operations Act 1997* (POEO Act).

The options for management of excavation pump-out water or dewatered groundwater, dependent upon the test results, are for disposal of the water as follows:

- Discharge to stormwater with prior approval from Council. Provided the test results comply with ANZG fresh water criteria (ANZG, 2018), or any other compliance requirements stipulated by Council. On site treatment for metals/PAH/PFAS may be required to comply with water quality criteria;
- Discharge to sewer, as industrial trade wastewater, with prior approval from Hunter Water. This option would require the analysis of a larger list of analytes, and compliance with the Hunter Water acceptance standards and discharge conditions. It is noted this option is typically only considered when all other options have been assessed; or
- Pumping by a liquid waste contractor for removal of the water off-site to an aqueous treatment plant, in accordance with regulatory requirements.

The three management options, as outlined above, are required to meet different assessment criteria and associated licences/approvals. It is anticipated that disposal would be via stormwater, however, the water would require treatment prior to disposal to stormwater to:

- Adjust pH;
- Reduce total suspended solids (TSS);
- Reduce the concentrations of metals, PAH and PFAS and any other contaminants such that they meet the criteria.

Groundwater quality testing as part of the DSI indicated that groundwater was not suitable for irrigation at the surface or on landscape areas. Reinfiltration of groundwater in a temporary sump or basin is therefore not likely to be feasible, unless a location can be identified that is:

- Away / downgradient from the excavation and buildings;
- Excavated below upper contaminated fill (i.e. will not induce leaching of contaminants in soil);
- Not in areas which have already been remediated / capped as per the RAP;
- Acceptable to the Site Auditor.

Douglas

8. Dewatering Strategy and Water Management

As extensive dewatering of deep excavations is not proposed, it is anticipated that the following general set-up may be applicable for the dewatering system:

- Dewater from suction pumps or submersible pump(s) installed within the excavation and/or internal sumps; and/or
- Water from should be collected in a multicell holding tank;
- A flow meter should be attached the holding tank to record flow volumes as a total measurement of volume (i.e. outlet volume monitoring);
- The holding tank would allow for monitoring of the water quality and treatment as required prior to discharge (i.e. dosing/pH adjustment or removal of sediments);
- A water treatment system could be connected to the system as required;
- Extracted groundwater could be discharged to suitable location, such as a nominated stormwater location subject to regulatory approvals.

Dewatering spears around the perimeter of an excavation could also be considered, if required, and would similarly require connection to the holding tank.

Dewatering requirements should be reassessed by the Construction Contractor as the works progress. This may include periodic review of groundwater levels in monitoring wells (Wells 401 to 406) and review of excavation depths and dewatering methods prior to excavations. Should the actual requirements or groundwater level conditions differ from those outlined in Sections 5 and 6, review is recommended to ensure volumes remain minimal and do not exceed 3 ML annually. Prolonged dewatering at any specific location during construction (ie more than say 3 weeks) should be avoided to minimise the total volume extracted from the aquifer.

It is noted that excavation methods and dewatering are also required to follow the procedures in the following documents:

- Remediation Action Plan (Douglas, 2023e) regarding excavation, capping and validation. Discharge / disposal of groundwater will need to be away from progressively capped / validated areas;
- Acid Sulfate Soil Management Plan (Douglas, 2022b) applicable for excavations / dewatering below RL -4.

Additional specific requirements may rise from the NSW EPA Site Auditor pertaining to groundwater disposal during the audit process.



9. Monitoring Program

9.1 **Objectives**

The objectives of the DMP are to ensure that the proposed dewatering does not adversely affect the surrounding environment, the structural integrity of neighbouring structures or result in environmental pollution because of the discharge of water of unacceptable quality.

This DMP outlines the responsibilities, monitoring requirements, reporting requirements and contingency plans to be followed during temporary dewatering for construction of the proposed development. The requirements in this plan should be followed during all excavation and construction works on-site and until all temporary dewatering pumps are switched off.

Water requiring disposal needs to be managed and disposed of in accordance with the requirements of the *Protection of the Environment Operations Act 1997* (POEO Act).

9.2 **Responsibilities**

The Principal Contractor for the proposed development is responsible for:

- Overall implementation of all the measures outlined in this plan;
- Engagement of qualified and experienced Dewatering Contractors and Environmental Consultants, and
- Compliance with any relevant conditions of consent or approvals, including requirements of the ASSMP and Site Auditor.

The site-specific activities and responsibilities required as part of this DMP are summarised in Table 8.

Table 8: Activities and Responsibilities

Activity	Responsibility		
Licensing and approvals	Client or nominated Contractor		
Installation of collection tanks and flow meters on pumps	Dewatering Contractor		
Measurement of groundwater inflow/dewatering volumes	Dewatering Contractor - collect data and supply to Environmental Consultant		
Measurement of volumes of water disposed off- site	Dewatering Contractor - collect data and supply to Environmental Consultant		
Chemical testing of water from site prior to disposal	Environmental Consultant		
Review of contractor supplied information	Environmental Consultant		
Preparation of periodic reports	Environmental Consultant		
Preparation of dewatering compliance report (if required)	Environmental Consultant		



It is recommended that a pre-start meeting including all relevant parties is held to confirm the requirements of site plans, including this DMP, and roles and responsibilities.

9.3 Monitoring Systems and Frequencies

9.3.1 **Overview**

The required monitoring systems, frequencies, review procedures and reporting requirements for monitoring of the temporary dewatering as listed in Table 9. The systems should be integrated with the final design by the dewatering contractor and any specific conditions of Council, regulatory authorities, Site Auditor or licence requirements.



Table 9: Monitoring Frequency and Reporting Requirements

ltem	Monitoring System	Monitoring Frequency	Monitoring Parameters	Investigation Level	Review Process	Reporting
Chemical testing of water from site prior to discharge/disposal	Samples to be collected from holding tank(s)	Twice daily, or once holding tank is filled (whichever is more frequent), for the first two weeks. After steady groundwater inflow rates are established then daily. Prior to discharge. Weekly quality testing for the duration of dewatering.	Field parameters: - Visual inspection - pH - EC - Turbidity - ORP - Dissolved oxygen - Temperature Laboratory analytes: - Total Suspended Solids - Heavy Metals (total) ⁽¹⁾ - Oil and grease~ - TRH, PAH, BTEX ⁽²⁾ - Ammonia - Total phosphorus - PFAS BOD5 (trado wasto only)	Visual - No visual indicators of contamination observed (slicks, staining, odours). For off-site disposal to stormwater, comparison against ANZG guidelines for fresh water as summarised in Table 11. For off-site disposal to sewer, comparison against trade waste agreement levels.	Tabulate and chart results over time for each parameter, compare to ANZG and check for compliance. Implement water treatment measures if water quality objectives are not met.	After each test event and then weekly
Measurement of volumes of water for disposal	Calibrated flow meters connected to the pump- out system	Continuously using flow meter at holding tank outlet	Flow rate (L/day) / total volume (ML)	Extracted volume not to exceed 3 ML annually (July to June) for construction exemption. Extracted volume not to exceed WAL conditions	Tabulate results and review against predicted / approved volumes . Implement measures to reduce inflows (trench.	Daily

Notes to Table:

Refer to Table 11 for disposal/treatment criteria.

 Heavy metals – arsenic (As), aluminium (Al), beryllium (Be), boron (B), cadmium (Cd), chromium (Cr), cobalt (Co), copper (Cu), lead (Pb), manganese (Mn), mercury (Hg), molybdenum (Mo), nickel (Ni), silver (Ag), selenium (Se), zinc (Zn)
 The ongoing requirement for analysis of TRH/BTEX/PAH and oil and grease may be reviewed by the dewatering/environmental consultant if the initial monitoring period records results below PQL for these analytes and no visual indicators are observed.



The results are be reviewed on an ongoing basis and include checking for exceedance of investigation levels as well as for general trends, which may be indicative of a potential issue.

As extensive dewatering / long term dewatering is not proposed, a reduction in the monitoring program outlined in Table 9 (e.g. weekly / monthly frequency) is not considered appropriate.

Table 9 should be readopted for each excavation / dewatering task.

In addition to the above, visual inspection of the water leaving the discharge point is to be undertaken by the site manager, or nominated representative, twice daily to confirm the water is not displaying signs of contamination (e.g. discolouration, odours, sheen).

9.3.2 Holding Tank Sampling Methods

Monitoring and sampling should be conducted by suitably qualified and experienced personnel under strict QA/QC protocols. The sample should be collected from the holding tank (near the outlet) using a new laboratory prepared bottle (non-preserved), with/without a telescopic pole as required. A new pair of disposable nitrile gloves are worn at each sample site to minimise potential for cross-contamination.

All sampling data should be recorded on chain of custody (COC) sheets, with the general sampling procedure comprising:

- Decontamination of all non-disposable sampling equipment using a 3% solution of phosphate free detergent (Decon 90) and tap water prior to collecting each sample;
- The use of new disposable gloves for each sampling location;
- Placement of the samples into analyte specific laboratory-prepared containers filled with zero-headspace and capping immediately to avoid loss of volatiles. Care should be taken to ensure that containers with preservatives are not overfilled;
- Collection of 10% replicate samples for QA/QC purposes;
- Labelling of sample bottles with individual and unique identification, including project number, sample location and sampling date;
- Placement of the sample bottles into a cooled, insulated and sealed container for transport to the laboratory within recommended holdings times for analysis;
- Use of COC documentation ensuring that sample tracking and custody could be crosschecked at any point in the transfer of samples from the field to the laboratory. COC documentation should include sample location, sample ID, time / date collected, container type (i.e. glass / plastic), storage location, who collected the sample and sample despatch details;
- Adherence to laboratory recommended holding times for samples and use of appropriately preserved sample bottles. It should be noted that sample holding times vary depending on the analyte to be tested;
- Calibration of all hand-held monitoring equipment (i.e. pH, EC and multi-parameter meter) undertaken prior to each sampling round;



- Record the field parameters and observations on a sampling record sheets using a calibrated multi-parameter meter as follows:
 - o Time;
 - o Date;
 - o pH;
 - Electrical Conductivity (EC);
 - Oxidation-Reduction Potential (ORP);
 - Dissolved Oxygen;
 - o Turbidity;
 - o Temperature;
 - Comments / observations / climatic conditions / sampler name.

9.3.3 Quality Assurance/Quality Control

Quality Control (QC) of the field and laboratory program should be achieved by the following means:

- Check replicate a specific sample is split in the field, placed in separate containers, labeled with different sample numbers, and sent to the laboratory for analysis. Comparison of laboratory results (RPD analysis) of the two samples is subsequently undertaken (10% of primary samples);
- Check rinsate a rinsate water sample is taken and sent to the laboratory at the completion of sampling to ensure decontamination of sampling equipment was adequate (5% of primary samples). Only required where non-disposable equipment is used;
- Method blanks laboratory reagent blanks to confirm the equipment and standard used were uncontaminated;
- Laboratory replicates laboratory split samples internally and conduct tests on separate extracts;
- Laboratory spikes samples were spiked by the laboratory with a known concentration of contaminants and subsequently tested for percent recovery.

The Relative Percent Difference (RPD) between replicate results is used as a measure of laboratory reproducibility and is given by the following:

 $RPD = \frac{ABS (Replicate result 1 - Replicate result 2)}{(Replicate result 1 + Replicate result 2)/2} \times 100$

The RPD can have a value between 0% and 200%. An RPD data quality objective of up to 50% is generally considered to be acceptable for organic analysis, and 35% for inorganics (i.e. Metals).

The recommended Data Quality Indicators (DQIs) to assess the achievement of the Data Quality Objectives are summarised in Table 10.



Table 10: Data Quality Indicators

DQI	Achievement Evaluation Procedure	Target
Completeness	Critical Samples collected - location and depth Experienced field staff Documentation correct Critical samples analysed Chemical analysis per DMP Complied with holding times	95%
Comparability	Consistent sampling procedures Climatic / physical site conditions Experienced field staff Same laboratories used	Qualitative
Representativeness	Appropriate media sampled All analytes in DMP sampled All samples analysed per DMP	Qualitative
Precision (variability or reproducibility)	Analysis of field and laboratory replicates and achievement of acceptable RPDs, acceptable levels for laboratory QC criteria.	RPD 0 - 50% ¹ RPD 0 -35% ²
Accuracy (bias)	Analysis of field blanks, rinsate blanks, matrix spikes, surrogate spikes, etc.	60 - 140% ³ 70 - 130% ⁴

Notes to table:

1. Relative Percentage Difference for Organics for results > 5xPQL

2. Relative Percentage Difference for Inorganics for results > 5xPQL

3 Laboratory Recovery of Spikes for Organics for results > 5xPQL

4 Laboratory Recovery of Spikes for Inorganics for results > 5xPQL

9.3.4 Assessment of Holding Tank Water Quality

Extracted groundwater should be assessed against the criteria shown in Table 11 below and the following:

- The magnitude of an exceedance with respect to criteria
- Previous baseline monitoring (Table B1 in Appendix B) and the trend of the parameter leading up the exceedance;
- Results of QA/QC testing on the samples and reliability of results (may require re-sampling and testing for confirmation);
- Potential for adverse environmental impact associated with the exceedance;
- If warranted, undertake an additional round of sampling as soon as practical and analyse for parameter(s) of concern.



Table 11: Assessment Criteria for Laboratory Testing of Discharge Waters

Analyte	Measurement Units	PQL	Environmental Screening Values - Fresh Water 95% Level of Protection ⁽¹⁾	Treatment Option
pH (field)	рН	0.1	6.5-8.0 ⁽²⁾	Lime or similar
Turbidity (field)	NTU	0.1	50 ⁽²⁾	Use flocculants to
TSS			50 ⁽²⁾	remove suspended sediments.
Ammonia	mg/L	0.1	0.9	Use flocculants to
Total Phosphorus	mg/L	0.05	0.025 (low land rivers) ⁽²⁾	remove nutrients in suspended sediments.
Aluminium	mg/L	0.01	0.055	
Arsenic	mg/L	0.001	0.013	
Cadmium	mg/L	0.0001	0.0002	Use flocculants to
Chromium	mg/L	0.001	0.001	remove metals
(+ ∨)				suspended in
Copper	mg/L	0.001	0.0014	sediments.
Lead	mg/L	0.001	0.0034	lon exchange or
Manganese	mg/L	0.005	1.9	activated carbon
Mercury	mg/L	0.00005	0.00006	absorption
Nickel	mg/L	0.001	0.011	
Zinc	mg/L	0.001	0.008	
Benzene	mg/L	0.001	0.6	
Ethylbenzene	mg/L	0.001	0.08	Activated carbon and
Toluene	mg/L	0.001	0.18	aeration air stripping
Xylene (o)	mg/L	0.001	0.05	
Anthracene	mg/L	0.0001	0.00001	
Benzo(a) pyrene	mg/L	0.0001	0.0001	
Naphthalene	mg/L	0.0001	0.016	
Fluoranthene	mg/L	0.001	0.001	
Phenanthrene	mg/L	0.0001	0.0006	
Oil and Grease	mg/L	0.0001	0.3	
PFOS (99% LOP)	mg/L	0.0000023	0.000002	Activated carbon
PFOA (99% LOP)	mg/L	0.00000023	0.019	

Note to Table:

1. ANZG, Australian and New Zealand Guidelines for Fresh and Marine Water Quality (2018) for the 95% level of protection for marine ecosystems.

2. ANZECC, Australian Water Quality Guidelines (2000), Tables 3.3.2-3.3.3 Default trigger values for physical and chemical stressors in south-east Australia for slightly disturbed ecosystems (ANZECC 2000).

3. 99% level of protection (LOP) as recommended in NEPM (HEPA, 2020) for bio accumulative contaminants


9.4 Contingencies

The following possible site events have been identified as contingency items and nominated actions.

Table 12: Contingency Plan

ltem	Action		
	Groundwater Flows		
Overflow of the water collection tank.	Cease works The site supervisor is to contact the Principal Contractor and Environmental Consultant; If necessary, notify NCC; Implement measures to ensure overflow does not occur again.		
Higher groundwater levels and/or flow rates than	Delayed release of water, and/or temporarily turn off or reduce pumping rates if possible;		
expected	Implement back-up storage, such as additional holding tanks or undertake disposal via off site tanker to a licensed facility;		
	Environmental Consultant to review flow and estimates and assess against licencing / exemption conditions;		
	Undertake additional submissions to Water NSW for additional water licence extraction if required.		
	Water Quality		
Water quality criteria not being met	Review site procedures, dewatering set up and areas for improvement to system; Implement treatment measures / systems for water if required; Off site disposal via water truck to licensed facility.		
pH outside the disposal criteria range pH 6.5-8.0	pH adjustment using various additives.		
Elevated sediments / turbidity	Flocculants used to reduce turbidity and suspended solids.		
Elevated metals	Treatment system such as ion exchange or activated carbon absorption.		
Other contaminants such as TRH, BTEX	Treatment system such as activated carbon and aeration air stripping.		

Monitoring, management and treatment (where required) will be conducted to minimise the risk of adverse impacts on groundwater aquifers and downgradient aquatic environments.

Advice should be sought from an environmental consultant to determine a suitable course of action and/or water treatment for implementing the contingency procedures above (i.e. advice on pH adjustment, flocculants, removal of metals if required).



The Construction Contractor is responsible for implementing contingency plans, as required.

9.5 **Monitoring and Reporting**

As above, the results be reviewed on an ongoing basis. This should include checking for exceedance of investigation levels and investigations levels as well as for general trends, which may be indicative of a potential issue as outlined in Table 9.

At the completion of the dewatering, a final dewatering compliance report that includes a discussion on the results of dewatering monitoring should be compiled. The report should include the following:

- Periods of pumping;
- All the water testing results;
- Actual volumes of dewatering;
- Water treatment measures;
- Inclement weather (i.e. preventing pumping or resulting in high flows);
- Implementation of contingency measures;
- Summary of compliance against approvals.

A return submissions to Water NSW will be required for dewatering to confirm compliance with the temporary dewatering for construction exemption (< 3 ML per annum) under the *Water Management* (*General*) *Regulation 2018*).

10. Conclusions and Recommendations

This revised DMP provides methods and strategies for temporary dewatering for construction for the Newcastle High School Upgrade. This revised DMP has been prepared in consultation with DPE Water.

It is considered that implementation of this plan can achieve the objective to ensure that dewatering does not adversely affect the surrounding environment or result in environmental pollution because of the discharge of water of unacceptable quality.

Modelling indicates that inflows and dewatering volumes are expected to be minor given the localised dewatering proposed and short duration. It is considered the proposed dewatering may meet exemption requirements for construction projects as the volume is not expected to exceed 3 ML/year.

Dewatering requirements should be reassessed by the Construction Contractor as the works progress. This may include periodic review of groundwater levels in monitoring wells and review of excavation depths and dewatering methods prior to excavations. Should the actual requirements or groundwater level conditions differ from those outlined in Sections 5 and 6, review is recommended to ensure volumes remain minimal and do not exceed 3 ML annually.



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12. Limitations

Douglas Partners (Douglas) has prepared this report for this project at 25a National Park Street, Newcastle West in accordance with Douglas' proposal 213618.04.P.002.Rev1 dated 12 February 2024 and acceptance received from Robert Petersen dated 12 February 2024. The work was carried out under Douglas' Engagement Terms.

This report is provided for the exclusive use of Hansen Yuncken Pty Ltd for this project only and for the purposes as described in the report. It should not be used by or relied upon for other projects or purposes on the same or other site or by a third party. Any party so relying upon this report beyond its exclusive use and purpose as stated above, and without the express written consent of Douglas, does so entirely at its own risk and without recourse to Douglas for any loss or damage. In preparing this report Douglas has necessarily relied upon information provided by the client and/or their agents.



The results provided in the report are indicative of the sub-surface conditions on the site only at the specific sampling and/or testing locations, and then only to the depths investigated and at the time the work was carried out. Sub-surface conditions can change abruptly due to variable geological processes and also as a result of human influences. Such changes may occur after Douglas' field testing has been completed.

Douglas' advice is based upon the conditions encountered during previous investigations. The accuracy of the advice provided by Douglas in this report may be affected by undetected variations in ground conditions across the site between and beyond the sampling and/or testing locations. The advice may also be limited by budget constraints imposed by others or by site accessibility.

The assessment of atypical safety hazards arising from this advice is restricted to the groundwater components set out in this report and based on known project conditions and stated design advice and assumptions. While some recommendations for safe controls may be provided, detailed 'safety in design' assessment is outside the current scope of this report and requires additional project data and assessment.

This report must be read in conjunction with all of the attached and should be kept in its entirety without separation of individual pages or sections. Douglas cannot be held responsible for interpretations or conclusions made by others unless they are supported by an expressed statement, interpretation, outcome or conclusion stated in this report.

This report, or sections from this report, should not be used as part of a specification for a project, without review and agreement by Douglas. This is because this report has been written as advice and opinion rather than instructions for construction.

Appendix A

About this Report Terminology, Symbols and Abbreviations Soil Descriptions Rock Descriptions Sampling, Testing and Excavation Methodology Borehole Logs (Bores 1a, 5a and 107a) Borehole Log (Bore 4) Borehole Logs (Bores 201A to 220 and 301 to 303, 401 to 406) Test Pit Logs (304 to 316) Borehole / Test Pit Logs (501 to 525 and 601 to 605) Cone Penetration Tests (CPT 101 to 107)

Introduction

These notes have been provided to amplify DP's report in regard to classification methods, field procedures and the comments section. Not all are necessarily relevant to all reports.

DP's reports are based on information gained from limited subsurface excavations and sampling, supplemented by knowledge of local geology and experience. For this reason, they must be regarded as interpretive rather than factual documents, limited to some extent by the scope of information on which they rely.

Copyright

This report is the property of Douglas Partners Pty Ltd. The report may only be used for the purpose for which it was commissioned and in accordance with the Conditions of Engagement for the commission supplied at the time of proposal. Unauthorised use of this report in any form whatsoever is prohibited.

Borehole and Test Pit Logs

The borehole and test pit logs presented in this report are an engineering and/or geological interpretation of the subsurface conditions, and their reliability will depend to some extent on frequency of sampling and the method of drilling or excavation. Ideally, continuous undisturbed sampling or core drilling will provide the most reliable assessment, but this is not always practicable or possible to justify on economic grounds. In any case the boreholes and test pits represent only a very small sample of the total subsurface profile.

Interpretation of the information and its application to design and construction should therefore take into account the spacing of boreholes or pits, the frequency of sampling, and the possibility of other than 'straight line' variations between the test locations.

Groundwater

Where groundwater levels are measured in boreholes there are several potential problems, namely:

- In low permeability soils groundwater may enter the hole very slowly or perhaps not at all during the time the hole is left open;
- A localised, perched water table may lead to an erroneous indication of the true water table;

- Water table levels will vary from time to time with seasons or recent weather changes. They may not be the same at the time of construction as are indicated in the report; and
- The use of water or mud as a drilling fluid will mask any groundwater inflow. Water has to be blown out of the hole and drilling mud must first be washed out of the hole if water measurements are to be made.

More reliable measurements can be made by installing standpipes which are read at intervals over several days, or perhaps weeks for low permeability soils. Piezometers, sealed in a particular stratum, may be advisable in low permeability soils or where there may be interference from a perched water table.

Reports

The report has been prepared by qualified personnel, is based on the information obtained from field and laboratory testing, and has been undertaken to current engineering standards of interpretation and analysis. Where the report has been prepared for a specific design proposal, the information and interpretation may not be relevant if the design proposal is changed. If this happens, DP will be pleased to review the report and the sufficiency of the investigation work.

Every care is taken with the report as it relates to interpretation of subsurface conditions, discussion of geotechnical and environmental aspects, and recommendations or suggestions for design and construction. However, DP cannot always anticipate or assume responsibility for:

- Unexpected variations in ground conditions. The potential for this will depend partly on borehole or pit spacing and sampling frequency;
- Changes in policy or interpretations of policy by statutory authorities; or
- The actions of contractors responding to commercial pressures.

If these occur, DP will be pleased to assist with investigations or advice to resolve the matter.

continued next page



Site Anomalies

In the event that conditions encountered on site during construction appear to vary from those which were expected from the information contained in the report, DP requests that it be immediately notified. Most problems are much more readily resolved when conditions are exposed rather than at some later stage, well after the event.

Information for Contractual Purposes

Where information obtained from this report is provided for tendering purposes, it is recommended that all information, including the written report and discussion, be made available. In circumstances where the discussion or comments section is not relevant to the contractual situation, it may be appropriate to prepare a specially edited document. DP would be pleased to assist in this regard and/or to make additional report copies available for contract purposes at a nominal charge.

Site Inspection

The company will always be pleased to provide engineering inspection services for geotechnical and environmental aspects of work to which this report is related. This could range from a site visit to confirm that conditions exposed are as expected, to full time engineering presence on site.

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Terminology, Symbols and Abbreviations

Douglas Partners' reports, investigation logs, and other correspondence may use terminology which has quantitative or qualitative connotations. To remove ambiguity or uncertainty surrounding the use of such terms, the following sets of notes pages may be attached Douglas Partners' reports, depending on the work performed and conditions encountered:

- Soil Descriptions;
- Rock Descriptions; and
- Sampling, insitu testing, and drilling methodologies

In addition to these pages, the following notes generally apply to most documents.

Abbreviation Codes

Site conditions may also be presented in a number of different formats, such as investigation logs, field mapping, or as a written summary. In some of these formats textual or symbolic terminology may be presented using textual abbreviation codes or graphic symbols, and, where commonly used, these are listed alongside the terminology definition. For ease of identification in these note pages, textual codes are presented in these notes in the following style Xw. Code usage conforms with the following guidelines:

- Textual codes are case insensitive, although herein they are generally presented in upper case; and
- Textual codes are contextual (i.e. the same or similar combinations of characters may be used in different contexts with different meanings (for example PL is used for plastic limit in the context of soil moisture condition, as well as in PL(A) for point load test result in the testing results column)).

Data Integrity Codes

Subsurface investigation data recorded by Douglas Partners is generally managed in a highly structured database environment, where records "span" between a top and bottom depth interval. Depth interval "gaps" between records are considered to introduce ambiguity, and, where appropriate, our practice guidelines may require contiguous data sets. Recording meaningful data is not always appropriate (for example assigning a "strength" to a concrete pavement) and the following codes may be used to maintain contiguity in such circumstances.

Term	Description	Abbreviation Code
Core loss	No core recovery	KL
Unknown	Information was not available to allow classification of the property. For example, when auguring in loose, saturated sand auger cuttings may not be returned.	UK
No data	Information required to allow classification of the property was not available. For example if drilling is commenced from the base of a hole predrilled by others	ND
Not Applicable	Derivation of the properties not appropriate or beyond the scope of the investigation. For example providing a description of the strength of a concrete pavement	NA

Graphic Symbols

Douglas Partners' logs contain a "graphic" column which provides a pictorial representation of the basic composition of the material. The symbols used are directly representing the material name stated in the adjacent "Description of Strata" column, and as such no specific graphic symbology legend has been provided in these notes.

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August 2020



Introduction

All materials which are not considered to be "in-situ rock" are described in general accordance with the soil description model of AS 1726-2017 Part 6.1.3, and can be broken down into the following description structure:



The "classification" comprises a two character "group symbol" providing a general summary of dominant soil characteristics. The "name" summarises the particle sizes within the soil which most influence it's behaviour. The detailed description presents more information about the soil's composition, condition, structure, and origin.

Classification, naming and description of soils requires the relative proportion of particles of different sizes within the whole soil mixture to be considered.

Particle size designation and Behaviour Model

Solid particles within a soil are differentiated on the basis of size.

The engineering behaviour properties of a soil can subsequently be modelled to be either "fine grained" (also known as "cohesive" behaviour) or "coarse grained" ("non cohesive" behaviour), depending on the relative proportion of fine or coarse fractions in the soil mixture.

Particle	Particle	Behavi	our Model	
Size	Size	Behaviour	Approximate	
Fraction	(mm)		Dry Mass	
Boulder	>200	Excluded from particle beh		
Cobble	63 - 200	aviour model as "oversize"		
Gravel ¹	2.36 - 63	Coarso	>65%	
Sand ¹	0.075 - 2.36	Coarse	>00%	
Silt	0.002 - 0.075	Fino	> 250/	
Clay	< 0.002		>JJ /0	
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¹ – refer grain size subdivision descriptions below

The behaviour model boundaries defined above are not precise, and the material behaviour should be assumed from the name given to the material (which considers the particle fraction which dominates the behaviour, refer "component proportions" below), rather than strict observance of the proportions of particle sizes. For example, if a material is named a "Sandy CLAY", this is indicative that the material exhibits fine grained behaviour, even if the dry mass of coarse grained material may exceed 65%.

Component proportions

The relative proportion of the dry mass of each particle size fraction is assessed to be a "primary", "secondary", or "minor" component of the soil mixture, depending on it's influence over the soils behaviour.

Component	Definition ¹	Relative Proportion	
Proportion		In Fine Grained Soil	In Coarse Grained
Designation			Soil
Primary	The component (particle size designation, refer above) which dominates the engineering behaviour of the soil	The clay/silt component with the greater proportion	The sand/gravel component with the greater proportion
Secondary	Any component which is not the primary, but is significant to the engineering properties of the soil	Any component with greater than 30% proportion	Any granular component with greater than 30%; or
			Any fine component with greater than 12%
Minor ²	Present in the soil, but not significant to it's engineering properties	All other components	All other components

¹ – As defined in AS1726-2017 6.1.4.4

 2 – in the detailed material description, minor components are split into two further sub categories. Refer "identification of minor components" below

Composite Materials

In certain situations a lithology description may describe more than one material, for example, collectively describing a layer of interbedded sand and clay. In such a scenario, the two materials would be described independently, with the names preceded or followed by a statement describing the arrangement by which the materials co-exist. For example "INTERBEDDED Silty CLAY AND SAND".

Classification

The soil classification comprises a two character group symbol. The first symbol identifies the primary component. The second symbol identifies either the grading or presence of fines in a coarse grained soil, or the plasticity in a fine grained soil. Refer AS1726-2017 6.1.6 for further clarification.

Soil Name

For most soils the name is derived with the primary component included as the noun (in upper case), preceded by any secondary components stated in an adjective form. In this way the soil name also describes the general composition and indicates the dominant behaviour of the material.

Component ¹	Prominence in Soil Name
Primary	Noun (eg "CLAY")
Secondary	Adjective modifier (eg "Sandy")
Minor	No influence

¹ – for determination of component proportions, refer component proportions on previous page

For materials which cannot be disaggregated, or which are not comprised of rock or mineral fragments, the names "ORGANIC MATTER" or "ARTIFICIAL MATERIAL" may be used, in accordance with AS1726-2017 Table 14.

Commercial or colloquial names are not used for the soil name where a component derived name is possible (for example "Gravelly SAND" rather than "CRACKER DUST").

Identification of minor components

Minor components are identified in the soil description immediately following the soil name. The minor component fraction is usually preceded with a term indicating the relative proportion of the component.

Minor Component	Relative Proportion	
Proportion Term	In Fine Grained Soil	In Coarse Grained Soil
With	All fractions: 15-30%	clay/silt: 5-12%
		sand/gravel: 15-30%
Trace	All fractions: 0-15%	clay/silt: 0-5%
		sand/gravel: 0-15%

Soil Composition

Plasticity		
Descriptive	Laboratory liquid limit	
Term	ra	ange
	Silt	Clay
Non-plastic	Not	Not
materials	applicable	applicable
Low plasticity	≤50	≤35
Medium	Not	>35 and ≤50
plasticity	applicable	
High	>50	>50
plasticity		

Note, Plasticity descriptions generally describe the plasticity behaviour of the whole of the fine grained soil, not individual fine grained fractions.

Grain Size

-				
	Туре		Particle size (mm)	
	Gravel	Coarse	19 - 63	
		Medium	6.7 - 19	
		Fine	2.36 - 6.7	
	Sand	Coarse	0.6 - 2.36	
		Medium	0.21 - 0.6	
		Fine	0.075 - 0.21	

<u>Grading</u>

-		
	Grading Term	Particle size (mm)
	Well	A good representation of all
		particle sizes
	Poorly	An excess or deficiency of
		particular sizes within the
		specified range
	Uniformly	Essentially of one size
	Gap	A deficiency of a particular
		particle size with the range

Note, AS1726-2017 provides terminology for additional attributes not listed here.

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Soil Condition

Moisture

The moisture condition of soils is assessed relative to the plastic limit for fine grained soils, while for coarse grained soils it is assessed based on the appearance and feel of the material. The moisture condition of a material is considered to be independent of stratigraphy (although commonly these are related), and this data is presented in its own column on logs.

Applicability	Term	Tactile Assessment	Abbreviation code
Fine	Dry of plastic limit	Hard and friable or powdery	<pl< td=""></pl<>
	Near plastic limit	Can be moulded	≈PL
	Wet of plastic limit	Water residue remains on hands when handling	>PL
	Near liquid limit	"oozes" when agitated	≈LL
	Wet of liquid limit	"oozes"	>LL
Coarse	Dry	Non-cohesive and free running	D
	Moist	Feels cool, darkened in colour, particles may stick	Μ
		together	
	Wet	Feels cool, darkened in colour, particles may stick	W
		together, free water forms when handling	

The abbreviation code NDF, meaning "not-assessable due to drilling fluid use" may also be used.

Note, observations relating to free ground water or drilling fluids are provided independent of soil moisture condition.

Consistency/Density/Compaction/Cementation/Extremely Weathered Rock

These concepts give an indication of how the material may respond to applied forces (when considered in conjunction with other attributes of the soil). This behaviour can vary independent of the composition of the material, and on logs these are described in an independent column and are generally mutually exclusive (i.e it is inappropriate to describe both consistency and compaction at the same time). The method by which the behaviour is described depends on the behaviour model and other characteristics of the soil as follows:

- In fine grained soils, the "consistency" describes the ease with which the soil can be remoulded, and is generally correlated against the materials undrained shear strength;
- In granular materials, the relative density describes how tightly packed the particles are, and is generally correlated against the density index;
- In anthropogenically modified materials the compaction of the material is described qualitatively;
- In cemented soils (both natural and anthropogenic), the cemented "strength" is described qualitatively, relative to the difficulty with which the material is disaggregated; and
- In soils of extremely weathered rock origin, the engineering behaviour may be governed by relic rock features, and expected behaviour needs to be assessed based the overall material description

Quantitative engineering performance of these materials may be determined by laboratory testing, or estimated by correlated field tests (for example penetration or shear vane testing), or by tactile methods, as appropriate.

Consistency Term	Tactile Assessment	Undrained Shear Strength (kPa)	Abbreviation Code
Very soft	Extrudes between fingers when squeezed	<12	VS
Soft	Mouldable with light finger pressure	>12 - ≤25	S
Firm	Mouldable with strong finger pressure	>25 - ≤50	F
Stiff	Cannot be moulded by fingers	>50 - ≤100	ST
Very stiff	Indented by thumbnail	>100 - ≤200	VST
Hard	Indented by thumbnail with difficulty	>200	Н
Friable	Easily crumbled or broken into small pieces by hand	-	FR

Consistency (fine grained soils)

Relative Density (coarse grained soils)

Tactile assessment of relative density is difficult, and generally requires penetration testing, hence a tactile assessment guide is not provided.

Relative Density Term	Density Index	Abbreviation Code
Very loose	<15	VL
Loose	>15-≤35	L
Medium dense	>35-≤65	MD
Dense	>65-≤85	D
Very dense	>85	VD



Compaction (anthropogenically modified soil)	
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Compaction Term	Abbreviation Code
Well compacted	WC
Poorly compacted	PC
Moderately compacted	MC
Variably compacted	VC

Cementation (natural and anthropogenic)

Cementation Term	Abbreviation Code
Moderately cemented	MCE
Weakly cemented	WKCE
Cemented	CE
Strongly bound	SB
Weakly bound	WB
Unbound	UB

Extremely Weathered Rock

AS1726-2017 considers weathered rock material to be soil if the unconfined compressive strength is less than 0.6 MPa (i.e. very low strength rock). These materials may be identified as "extremely weathered rock" in reports and by the abbreviation code XWR on log sheets. This identification is not correlated to any specific qualitative or quantitative behaviour, and the engineering properties of this material must therefore be assessed according to engineering principles with reference to any relic rock structure, fabric, or texture described in the description.

Soil Origin

oon ongin			
Term	Description	Abbreviation Code	
Residual	Derived from in-situ weathering of the underlying rock	RES	
Extremely weathered material	Formed from in-situ weathering of geological formations. Has strength of less than 'very low' as per as1726 but retains the structure or fabric of the parent rock.	XWM	
Alluvial	Deposited by streams and rivers	ALV	
Estuarine	Deposited in coastal estuaries	EST	
Marine	Deposited in a marine environment	MAR	
Lacustrine	Deposited in freshwater lakes	LCS	
Aeolian	Carried and deposited by wind	AEO	
Colluvial	Soil and rock debris transported down slopes by gravity	COL	
Topsoil	Mantle of surface soil, often with high levels of organic material	TOP	
Fill	Any material which has been moved by man	FILL	
Littoral	Deposited on the lake or sea shore	LIT	
Unidentifiable	Not able to be identified	UID	

Cobbles and Boulders

The presence of particles considered to be "oversize" may be described using one of the following strategies:

- Oversize encountered in a minor proportion (when considered relative to the wider area) are noted in the soil description; or
- Where a significant proportion of oversize is encountered, the cobbles/boulders are described independent of the soil description, in a similar manner to composite soils (described above) but qualified with "MIXTURE OF".

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Rock Strength

Rock strength is defined by the unconfined compressive strength and it refers to the strength of the rock substance and not the strength of the overall rock mass, which may be considerably weaker due to defects.

The Point Load Strength Index $I_{s(50)}$ is commonly used to provide an estimate of the rock strength and site specific correlations should be developed to allow UCS values to be determined. The point load strength test procedure is described by Australian Standard AS4133.4.1-2007. The terms used to describe rock strength are as follows:

Strength Term	Unconfined Compressive Strength (MPa)	Point Load Index ¹ I _{s(50)} MPa	Abbreviation Code
Very low	0.6 - 2	0.03 - 0.1	VL
Low	2 - 6	0.1 - 0.3	L
Medium	6 - 20	0.3 - 1.0	М
High	20 - 60	1 - 3	Н
Very high	60 - 200	3 - 10	VH
Extremely high	>200	>10	EH

¹ Assumes a ratio of 20:1 for UCS to $I_{s(50)}$. It should be noted that the UCS to $I_{s(50)}$ ratio varies significantly for different rock types and specific ratios may be required for each site.

On investigation logs only, the following data contiguity codes may be in rock strength tables for layers or seams of material "within rock", but for which the equivalent UCS strength is less than 0.6 MPa.

Scenario	Abbreviation
The material encountered has an equivalent UCS strength of less than 0.6 MPa, and therefore is considered to be soil (as per Note 1 of Table 20 of AS 1726-2017). The properties of the material encountered over this interval are described in the "Description of Strata" and soil properties columns.	SOIL
The material encountered has an equivalent UCS strength of less than 0.6 MPa, and therefore is considered to be soil (as per Note 1 of Table 20 of AS 1726-2017). The prominence of the material is such that it can be considered to be a seam (as defined in Table 22 of AS1726-2017) and the properties of the material are described in the defect column.	SEAM

Degree of Weathering

The degree of weathering of rock is classified as follows:

Weathering Term	Description	Abbreviation Code
Residual	Material is weathered to such an extent that it has soil properties. Mass	RS
Soil ^{1,2}	structure and material texture and fabric of original rock are no longer visible,	
	but the soil has not been significantly transported.	
Extremely	Material is weathered to such an extent that it has soil properties. Mass	XW
weathered ^{1,2}	structure and material texture and fabric of original rock are still visible	
Highly	The whole of the rock material is discoloured, usually by iron staining or	HW
weathered	bleaching to the extent that the colour of the original rock is not recognisable.	
	Rock strength is significantly changed by weathering. Some primary minerals	
	have weathered to clay minerals. Porosity may be increased by leaching, or	
	may be decreased due to deposition of weathering products in pores.	
Moderately	The whole of the rock material is discoloured, usually by iron staining or	MW
weathered	bleaching to the extent that the colour of the original rock is not recognisable,	
	but shows little or no change of strength from fresh rock.	
Slightly	Rock is partially discoloured with staining or bleaching along joints but shows	SW
weathered	little or no change of strength from fresh rock.	
Fresh	No signs of decomposition or staining.	FR
Note: If HW and MW cannot be differentiated use DW (see below)		
Distinctly	Rock strength usually changed by weathering. The rock may be highly	DW
weathered	discoloured, usually by iron staining. Porosity may be increased by leaching	
	or may be decreased due to deposition of weathered products in pores.	

¹ – AS1726-2017 6.1.9 provides similar definitions for "residual soil" and "extremely weathered material" as soil origins. Generally, the soil origin terms would be used above the depth at which very low strength or stronger rock material is first encountered, while both soil origin and weathering should may be stated for soil encountered below the first contact with rock material, where appropriate.

 2 –The parent rock type, of which the residual/extremely weathered material is a derivative, will be stated in the description (where discernible).



Degree of Alteration

The degree of alteration of the rock material (physical or chemical changes caused by hot gasses or liquids at depth) is classified as follows:

Term	Description	Abbreviation Code
Extremely altered	Material is altered to such an extent that it has soil properties. Mass structure and material texture and fabric of original rock are still visible.	ХА
Highly altered	The whole of the rock material is discoloured, usually by staining or bleaching to the extent that the colour of the original rock is not recognisable. Rock strength is changed by alteration. Some primary minerals are altered to clay minerals. Porosity may be increased by leaching, or may be decreased due to precipitation of secondary materials in pores.	HA
Moderately altered	The whole of the rock material is discoloured, usually by staining or bleaching to the extent that the colour of the original rock is not recognisable but shows little or no change of strength from fresh rock.	MA
Slightly altered	Rock is slightly discoloured but shows little or no change of strength from fresh rock	SA
Note: If HA and	MA cannot be differentiated use DA (see below)	
Distinctly altered	Rock strength usually changed by alteration. The rock may be highly discoloured, usually by staining or bleaching. Porosity may be increased by leaching, or may be decreased due to precipitation of secondary minerals in pores.	DA

Degree of Fracturing

The following descriptive classification apply to the spacing of natural occurring fractures in the rock mass. It includes bedding plane partings, joints and other defects, but excludes drilling breaks. These terms are generally not required on investigation logs where fracture spacing is presented as a histogram, and where used are presented in an unabbreviated format.

Term	Description
Fragmented	Fragments of <20 mm
Highly Fractured	Core lengths of 20-40 mm with occasional fragments
Fractured	Core lengths of 30-100 mm with occasional shorter and longer sections
Slightly Fractured	Core lengths of 300 mm or longer with occasional sections of 100-300 mm
Unbroken	Core contains very few fractures

Rock Quality Designation

The quality of the cored rock can be measured using the Rock Quality Designation (RQD) index, defined as:

RQD %= $\frac{\text{cumulative length of 'sound' core sections} \ge 100 \text{ mm long}}{\text{total drilled length of section being assessed}}$

where 'sound' rock is assessed to be rock of low strength or stronger. The RQD applies only to natural fractures. If the core is broken by drilling or handling (i.e. drilling breaks) then the broken pieces are fitted back together and are not included in the calculation of RQD.

Stratification Spacing

These terms may be used to describe the spacing of bedding partings in sedimentary rocks. Where used, these terms are generally presented in an unabbreviated format

Term	Separation of
	Stratification Planes
Thinly laminated	< 6 mm
Laminated	6 mm to 20 mm
Very thinly bedded	20 mm to 60 mm
Thinly bedded	60 mm to 0.2 m
Medium bedded	0.2 m to 0.6 m
Thickly bedded	0.6 m to 2 m
Very thickly bedded	> 2 m



Defect Descriptions

Defect Type

Term	Abbreviation Code		
Bedding plane	В		
Clay seam	CS		
Cleavage	CV		
Crushed zone	CZ		
Decomposed seam	DS		
Fault	F		
Joint	J		
Lamination	LAM		
Parting	PT		
Sheared zone	SZ		
Vein	VN		
Drilling/handling	DB , HB		
break			
Fracture	FCT		

Rock Defect Orientation

Term	Abbreviation Code
Horizontal	Н
Vertical	V
Sub-horizontal	SH
Sub-vertical	SV

Rock Defect Coating

Term	Abbreviation Code
Clean	CLN
Coating	CO
Healed	HE
Infilled	INF
Stained	STN
Tight	TI
Veneer	VEN

Rock Defect Infill

Term	Abbreviation Code					
Calcite	CA					
Carbonaceous	CBS					
Clay	CLY					
Iron oxide	FE					
Manganese	MN					
Silty	SLT					

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Rock Defect Shape/Planarity

Term	Abbreviation Code				
Curved	CU				
Irregular	IR				
Planar	PL				
Stepped	ST				
Undulating	UN				

Rock Defect Roughness

Term	Abbreviation Code
Polished	PO
Rough	RO
Slickensided	SL
Smooth	SM
Very rough	VR

Other Rock Defect Attributes

Term	Abbreviation Code				
Fragmented	FG				
Band	BND				
Quartz	QTZ				

Defect Orientation

The inclination of defects is always measured from the perpendicular to the core axis.

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Terminology Symbols Abbreviations



August 2020

Sampling and Testing

A record of samples retained and field testing performed is usually shown on a Douglas Partners' log with samples appearing to the left of a depth scale, and selected field and laboratory testing (including results, where relevant) appearing to the right of the scale, as illustrated below:



Sampling

The type or intended purpose for which a sample was taken is indicated by the following abbreviation codes.

Sample Type	Code
Auger sample	Α
Acid sulfate sample	ASS
Bulk sample	В
Core sample	C
Disturbed sample	D
Sample from SPT test	SPT
Environmental sample	E
Gas sample	G
Jar sample	J
Undisturbed tube sample	U ¹
Water sample	W
Piston sample	Ρ
Core sample for unconfined	UCS

¹ – numeric suffixes indicate tube diameter/width in mm

The above codes only indicate that a sample was retained, and not that testing was scheduled or performed.

Field and Laboratory Testing

A record that field and laboratory testing was performed is indicated by the following abbreviation codes.

Test Type	Code			
Pocket penetrometer (kpa)	PP			
Photo ionisation detector	PID			
Standard Penetration Test	SPT			
Shear vane (kpa)	V			
Unconfined compressive	UCS			
strength, (MPa)				
Point load test, axial (A),	PLT(_)			
diametric (D), irregular (I)				

Field and laboratory testing (continued)

Test Type	Code
Dynamic cone penetrometer,	DCP/150
followed by blow count	
penetration increment in mm	
(cone tip, generally in accordance	
with AS1289.6.3.2)	
Perth sand penetrometer, followed	PSP/150
by blow count penetration	
increment in mm	
(flat tip, generally in accordance	
with AS1289.6.3.3)	

Groundwater Observations

\triangleright	seepage/inflov	v								
∇	standing or ob	standing or observed water level								
NFGWO	no free ground	lwater obse	rved							
OBS	Observations fluids	obscured	by	drilling						

Drilling or Excavation Methods/Tools

The drilling/excavation methods used to perform the investigation may be shown either in a dedicated column down the left hand edge of the log, or stated in the log footer. In some circumstances abbreviation codes may be used.

Method	Abbreviation Code				
Excavator/backhoe bucket	B ¹				
Toothed bucket	TB ¹				
Mud/blade bucket	MB ¹				
Ripping tyne/ripper	RT				
Rock breaker/hydraulic hammer	RB				
Hand auger	HA1				
NMLC series coring	NMLC				
HMLC series coring	HMLC				
NQ coring	NQ				
HQ coring	HQ				
PQ coring	PQ				
Push tube	PT 1				
Rock roller	RR ¹				
Solid flight auger. Suffixes (TC)	SFA1				
and (V) indicate tungsten					
carbide or v-shaped tip					
respectively					
Sonic drilling	SON ¹				
Vibrocore	VC ¹				
Wash bore (unspecified bit type)	WB ¹				
Existing exposure	X				
Hand tools (unspecified)	HT				
Predrilled	PD				
Specialised bit (refer report)	SPEC ¹				
Diatube	DT ¹				
Hollow flight auger	HFA1				
Vacuum excavation	VE				

 1 - numeric suffixes indicate tool diameter/width in mm



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 160-200 Parkway Avenue, Hamilton South

SURFACE LEVEL: 3.2 AHD COORDINATE E:383998 N: 6355595.5 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 1a PROJECT No: 213618.01 DATE: 08/07/22 SHEET: 1 of 1





 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 160-200 Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.1 AHD COORDINATE E:383845 N: 6355630 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 5a PROJECT No: 213618.01 DATE: 08/07/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED	1		1		SAM	PLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
-	-4	0.0 0.1 - -	FILL/ (ML) Sandy SILT; dark brown; low plasticity; trace rootlets and organics FILL/ (ML) Sandy SILT; dark brown; silt fraction low plasticity; sand fraction fine to medium 0.2-0.3m: with concrete rubble—		FILL	NA NA	<pl <pl to<br="">=PL</pl></pl 					Ī	5 10 15
-		0.3 -	(SP) SAND, with silt; grey; fine to medium 0.35-0.4m: with fine gravel (SR)—{		ALV	D	М		D	(- 0.5 -		
-		0.7 -	(SP) SAND; pale grey; fine to medium		ALV	MD	м						
-		0.9 -	(SP) SAND, with silt; brown; fine to medium		ALV	MD	м	-	D	-{	-0.9-		
-	- 0	1.0-	(SP) SAND; grey; fine to medium		ALV	MD	М				-1.0-		
-		1.3 -	1.2-1.3m: with fine to medium gravel (SR)		} • •			-	D	-(-1.3-	SP/150	
-		-	(SP) SAND, with clay; brown dark brown; sand fraction fine to medium; clay fraction fine to medium, sub-rounded		ALV	MD	м						
-	-	1.5- - - 2- 21-	(SP) SAND; pale grey yellow; fine to medium		ALV	MD	M to W	-	D		- 1.9 - - 2 -		
-		-	Borehole discontinued at 2.10m depth Virtual refusal due to hole collapse at 2.1m									_	
-	-	-									 	_	
	: ""S IT: 10	oil orig PT D: F	in is "probable" unless otherwise stated. ¹⁷ Consistency/Relative density shadi R PT to 2.1m	ng is for v	isual refer C	PERA CASING	TOR: C	tion between co	ohesive	and gra	anular ma	aterials	is implied.



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 160-200 Parkway Avenue, Hamilton South

SURFACE LEVEL: 4 AHD **COORDINATE E**:383821.8 N: 6355566.4 **DATUM/GRID**: MGA94 Zone 56 **DIP/AZIMUTH**: 90°/--- LOCATION ID: 107a PROJECT No: 213618.01 DATE: 08/07/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED		1			SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
observed		0.0	FILL/ (ML) Sandy SILT; dark brown; silt fraction fine; sand fraction fine to medium; with trace rootlets and organics		FILL	NA	<pl< td=""><td></td><td>D</td><td>-(_</td><td>-0.1-</td><td></td><td>5 10 15</td></pl<>		D	-(_	-0.1-		5 10 15
No free groundwater	-	J.15 - - -	(SP) SAND, with silt; grey; fine to medium		ALV	MD	М		D	-(0.3 0.4		
	-	0.5 -	(SP) SAND; pale grey; fine to medium		ALV	MD	М		D	-(0.6 0.7		
	m	1-	(SP) SAND, with silt, trace gravel; dark brown; sand fraction fine to medium; gravel fraction fine to medium, sub-rounded		ALV	MD	м		D	-(-0.9- -1.0-		
	-	1.1 -	(SP) SAND; pale grey; fine to medium		ALV	MD	м						
	-	1.2 -	(SP) SAND, with silt, trace gravel; grey; sand fraction fine to medium; gravel fraction fine to medium, sub-rounded		ALV	MD	M to W		D	{	- 1.3-		
	-	-	(SP) SAND; pale grey yellow; fine to medium		ALV	МО	M to W		D	-(- 1.5		
	5-	2-	1.8-2.2m: pale grey—						D		- 2.0 -		
NOTES		- 2.2 - - - - - - - - - - - - - - - - - -	Borehole discontinued at 2.20m depth Virtual refusal due to hole collapse at 1.2m	ing is for vi	sual refer	ence only	- no correla	tion between	cohesive	e and or	2.2 -	aterials	is implied.
PLA MET REM	NT: HO	PTI D: F	R R PT to 2.2m Groundwater likely between 1.2m and 1.4m depth	Ing is for VI	C C	PERA ASING	TOR: B	Butcher	JULIESIVE	and gr	anular mi	aterials	LOGGED: Chaplin



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CLIENT: School Infrastructure NSW

PROJECT: Newcastle High School Upgrade LOCATION: 160-200 Parkway Avenue, Hamilton South

BOREHOLE LOG

SURFACE LEVEL: 4 AHD COORDINATE E:383791 N: 6355598 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

LOCATION ID: 4 PROJECT No: 213618.01 DATE: 06/07/22 SHEET: 2 of 7

			COND	ITIONS EN	ICOUNTE	RED							SAM	PLE				TESTING
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA		HW HW MW SW FR	DEPTH (m)	LL MM STRENGTH	RECOVERY (%)	RQD	600 800 800 800 800 800 800 800 800 800	1.00 (m)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
1.5	-		Silty CLAY; medium plastic (continued)	xity 1 1 1 1 1 1												· · ·		
		- 11-														- 11 -		
	-	-														· · ·		
	_α	° 12-														- 12 -		
	-															· · ·		
	- 0	ን 13-														- 13 -		
	-	13.5 -	Clayey SAND; fine to medi	um												· · ·		
		2 ₁₄ -														- 14 -		
	-	14.75	Silty CLAY: dork grov													· · ·		
		15	Sity CLAT, dark grey													- 15 -		
		-		$\begin{array}{c}1\\1\\1\\1\end{array}$												· · ·		
		- 16														- 16 -		
JULKENG	13	<u>2</u> 17														- 17 -		
00.20.20 0		-														· · ·		
		<u>†</u> 18-														- 18 -		
		-														· · ·		
PT:01 77/8		<u>9</u> 19-														- 19 -		
UKIED 03/0		-														· · ·		
	= =S: (#	")Soil orig	jin is "probable" unless otherwise stated.								<u>ii i</u>							

PLANT: Hanjin 114

OPERATOR: Total Drilling

METHOD: SFA to 2.5m, then PD to 33.3m, then HQ core to 62.0m CASING: PQ to 2.5m, HWT to 36.2m REMARKS: Soil description and depths are based on drillers logs. Information on soil should be obtained fron nearby Cone Penetration Tests (CPT)



CLIENT: School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: 160-200 Parkway Avenue, Hamilton South

SURFACE LEVEL: 4 AHD COORDINATE E:383791 N: 6355598 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

BOREHOLE LOG

LOCATION ID: 4 PROJECT No: 213618.01 DATE: 06/07/22 SHEET: 3 of 7

				CON	NDITIC	NS EN		RED					_		SAN	IPLE				TESTING
		RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	SOIL STRENGTH (where encountered)	GRAPHIC	TW THW MW SW FR	DEPTH (m)	L M STRENGTH	RECOVERY (%)	RQD	0.01 FRACTURE	010 SPACING 100 (m)	DEFECTS & REMARKS	SAMPLE	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
	2		-	Silty CLAY; dark grey (continued)		$\left \frac{1}{1} \right $														
	F		-																	
	-	-17	21 -															21		
			-																	
		~	-																	
	-	÷	22 -															- 22 -		
		-	-																	
		-19	23															- 23 -		
			-																	
	-		-																	
	-	-20	24 -															24		
			-																	
		-21	25															- 25 -		
	-		-																	
	-		-																	
	-	-22	26															- 26 -		
	-	-	-																	
OCKLOG		-23	27 -															- 27 -		
.02.00_R	-		-																	
DP_102	-		-																	
TE ID:		-24	28															28		
TEMPLA			-																	
2 10:14.	-	-25	- 29										 					- 29 -		
03/08/2	-		-																	
PORTED	-	-	-																	
	TES:	^(#) Sc	- oil orig	in is "probable" unless otherwise state	ed.]	

PLANT: Hanjin 114

OPERATOR: Total Drilling

 METHOD: SFA to 2.5m, then PD to 33.3m, then HQ core to 62.0m
 CASING: PQ to 2.5m, HWT to 36.2m

 REMARKS: Soil description and depths are based on drillers logs. Information on soil should be obtained fron nearby Cone Penetration Tests (CPT)



CLIENT: School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: 160-200 Parkway Avenue, Hamilton South

SURFACE LEVEL: 4 AHD COORDINATE E:383791 N: 6355598 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

BOREHOLE LOG

LOCATION ID: 4 PROJECT No: 213618.01 DATE: 06/07/22 SHEET: 4 of 7



 METHOD: SFA to 2.5m, then PD to 33.3m, then HQ core to 62.0m
 CASING: PQ to 2.5m, HWT to 36.2m

 REMARKS: Soil description and depths are based on drillers logs. Information on soil should be obtained fron nearby Cone Penetration Tests (CPT)



CLIENT: School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: 160-200 Parkway Avenue, Hamilton South

BOREHOLE LOG SURFACE LEVEL: 4 AHD COORDINATE E:383791 N: 6355598 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

LOCATION ID: 4 PROJECT No: 213618.01 DATE: 06/07/22 SHEET: 5 of 7

				CON	DITIC	ONS EN	COUNTE	RED					SAM	PLE				TESTING
GROUNDWATER	RL (m)		DEPTH (m)	DESCRIPTION OF STRATA	 SolL STRENGTH (where encountered) SolL MOISTURE 	GRAPHIC	RS HWW MWW SW FSS FSS FSS FSS FSS FSS FSS FSS FS	DEPTH (m)	M M STRENGTH	RECOVERY (%)	RQD	600 FRACTURE 88% SPACING 14% (m) 560 (m) DEFECTS & REMARKS	SAMPLE REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
1.5				SILTSTONE; grey (continued)											-			
	37	-21	41 -			· _ · · _ · · · · · · · · · · · · · · ·			м	100	71	 ↓↓↓ ↓↓↓ →_4 <u>1</u> ,6-41.64	m: J x3 60° PL,		-	41 -		
		00	42 -	42.36-42.45m: lenticu bedd	ılar –(ing ∏			42.3 6				SM 	5m: fragmented			42 -		
		20 -	43	42.36-42.68m: pale g	rey [/]		FR								- - - - - - - - - - - - - - - - - - -	43 -		
		-40	44 -							100	92				-	- 44 -		
		-4-	45 -													45 -		
oth		-42	46 -			· · · · · · · · · · · · · · · · · ·	FS	-46.15	н				2m: fragmented 60° PL, FE			46		
om 46.5m to 55.58m dep	13	-45	47 -					-47.09		100	89	 47.09m: J 2 	20° IR, RO, FE			47 -		
20% water loss fr		-++-	48 -				FR					=== 1 			- - - - - - - - - - - - - - - - - - -	48 -		
.41.01 77/00/C0 U	1	C+-	49 -							100	79				-	49 -		
NOTE	ES: (#	^{#)} Soi	l orig	n is "probable" unless otherwise stated	d.	· _ · _									-			

METHOD: SFA to 2.5m, then PD to 33.3m, then HQ core to 62.0m

RAIOR: Iotai Drii

CASING: PQ to 2.5m, HWT to 36.2m REMARKS: Soil description and depths are based on drillers logs. Information on soil should be obtained fron nearby Cone Penetration Tests (CPT)



CLIENT: School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: 160-200 Parkway Avenue, Hamilton South

BOREHOLE LOG SURFACE LEVEL: 4 AHD COORDINATE E:383791 N: 6355598 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

LOCATION ID: 4 PROJECT No: 213618.01 DATE: 06/07/22 SHEET: 6 of 7



 METHOD: SFA to 2.5m, then PD to 33.3m, then HQ core to 62.0m
 CASING: PQ to 2.5m, HWT to 36.2m

 REMARKS: Soil description and depths are based on drillers logs. Information on soil should be obtained fron nearby Cone Penetration Tests (CPT)



CLIENT: School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: 160-200 Parkway Avenue, Hamilton South

SURFACE LEVEL: 4 AHD COORDINATE E:383791 N: 6355598 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

BOREHOLE LOG

LOCATION ID: 4 PROJECT No: 213618.01 DATE: 06/07/22 SHEET: 7 of 7

			CONDITIONS ENCOUNTERED		SAMPLE			TESTING
GROUNDWATER	RL (m)	DEPTH (m)		High Strength Recovery (%) RQD RQD RQD RQD RQD RQC RRCING RRCING RRCING RRCING RRCING RRCING RRCING RRCING RRCING RC	SAMPLE REMARKS TYPE INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
1.5		60.25 60.63 5 61	COAL; black (continued) SILTSTONE; grey SANDSTONE; pale grey; fine FR	H 100 81 111 60.0-60. 100 81 111 111 111 111 111 111 111 111 1	25m: fragmented 1.55m: J SV PL, 1.58m: fragmented	- 61	-	
		62.0	Borehole discontinued at 62.00m depth			62	-	
		63				- 63 -		
		64				- 64 ·	-	
		65				- 65		
	-62	66				- 66	-	
		67				- 67	- - - - - - - - - - -	
10: 00_102.02.0		5 68				- 	-	
2 10:14. IEMPLAIE	-65	69				- 69	-	
						- - - -	-	
	S: (#)	Soil or	gin is "probable" unless otherwise stated. anjin 114 SEA to 2.5m, those DD to 22.2m, those HO correcto 62.0m	OPERATOR: Total Drilling	L L	OGGE	D: N	fillard

REMARKS: Soil description and depths are based on drillers logs. Information on soil should be obtained fron nearby Cone Penetration Tests (CPT)



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.0 AHD COORDINATE E:384059.0 N: 6355615.0 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 201 DATE: 16/08/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED	1		-		SAI	MPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
No free groundwater observed	-	0.0	FILL/ (SP) Silty SAND, trace gravel; dark brown; sand fraction fine to medium; gravel fraction fine to coarse, sub-angular to sub-rounded; trace plastic, glass, rootlets, ceramic		FILL	NA	М		D E D E		-0.05-	-PID-	<1
	- ~	o.s-	Borehole discontinued at 0.50m depth Limit of investigation								- 0.5		
EATON FLU 20/04/25 12:10. IEM-LALE 10: UF_UAL 02:00_SUILLUU		J 2-	- - - - - - - - - - - - - - - - - - -	ding is for visu	ual refere	ence only -	no correla	tion between	cohesive	e and gra	- 2 -		is implied.
PL4		ויים: וייסכ	nd Auger Hand auger to 0.5m		0	PERAT	'OR: k	Kramer		- 91			LOGGED: Kramer



TEST PIT LOG

 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.1 AHD COORDINATE E:384063.3 N: 6355617.1 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 201A PROJECT No: 213618.02 DATE: 30/11/22 SHEET: 1 of 1

But wordspace Image: Stress of the second	RESULTS AND REMARKS
0.0 FILL/ (SP) SINU SAND, with gravels: grey brown in the initiation fine to coarse sub-angular to sub-rounded (crushed natural rock); trace roctiets, glass, tape, ceramic, plastic, slag, coal 1 1 1 1 1 1 0.05 PID <1	
0.6 FILL/ (SP) SAND, trace gravel; intermixed brown grey pale grey; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded; trace coal, shells, slag NA D D -0.5 PID <1	
Sub-rounded; trace coal, shells, slag FILL M 0.9m: fine to coarse sub-angular to sub-rounded gravels FILL M 1 Test pit discontinued at 1.10m depth Hand refusal on gravels	
Test pit discontinued at 1.10m depth Hand refusal on gravels	
NOTES: ^(#) Soil origin is "probable" unless otherwise stated. ^(*) Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.	

METHOD: Shovel to 0.3m then hand auger to 1.1m

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 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.0 AHD COORDINATE E:383962.0 N: 6355686.0 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

LOCATION ID: 202 PROJECT No: 213618.02 DATE: 16/08/26 SHEET: 1 of 1

End End DESCRIPTION OF STRATA 0.0 FILL/ (SP) SAND; brown; fine to medi rootlets 0.15 FILL/ (SP) SINY SAND, trace gravel; or sand fraction fine to medium; gravel fit to coarse, sub-angular to sub-rounde ceramic, glass, rootlets 0.5 Borehole discontinued at 0.50m depti Limit of investigation 0.5 Borehole discontinued at 0.50m depti Limit of investigation	OUNTERED	TESTING AND REMARKS	
9 0.0 FILL/ (SP) SAND; brown; fine to median; gravel; or rootlets 0.15 FILL/ (SP) Silty SAND, trace gravel; or sand fraction fine to medium; gravel for to coarse, sub-angular to sub-rounde ceramic, glass, rootlets 0.5 Borehole discontinued at 0.50m depti Limit of investigation 0.6 Borehole discontinued at 0.50m depti Limit of investigation -co 1 -co -co -co	GRAPHIC	(m) H H H H H H H H H H H H H H H H H H H	DEPTH (m) TEST TYPE
0.5 Borehole discontinued at 0.50m depti Limit of investigation	dark brown; fraction fine led; trace ash,	-0.05 - PID- <1 0.3 - PID- <1	-0.05 PID-
4/23 12	th		- 0.5 - PID-
NOTES: ^{(%} Soil origin is "probable" unless otherwise stated. ^(*) Consistency/Re PLANT: Hand Auger	Relative density shading is for	ranular materials is implied.	inular materials



TEST PIT LOG

CLIENT:School Infrastructure NSWPROJECT:Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.3 AHD COORDINATE E:383968.0 N: 6355688.9 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 202A PROJECT No: 213618.02 DATE: 30/11/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
ter observed		0.0	FILL/ (SP) SAND; brown; fine to medium; trace rootlets, fine sub-angular to sub-rounded gravels (crushed natural rock)		FILL		D		D E	\prec	-0.05-	-PID-	<1
free groundwa	-2	0.2 -	FILL/ (SP) Silty SAND; dark grey; fine to medium; trace glass, ceramic, coal, brick fragments, slag			NA			D E		-0.25-	-PID-	<1
No	-	-			FILL		Μ		D E		-0.5-	PID	<1
	-	0.7 - 0.75 -	FILL/ (SP) SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to		FILL		Μ		D		-0.72-	-PID-	<1
	-	-	Test pit discontinued at 0.75m depth Hand refusal on gravels										
	-	1 -									- 1 -		
		-											
	-	-											
	-	-											
	-	-											
	-	2-									- 2 -		
	-0	-											
	-	-											
	_	-											
	-	-											
NOTES	- S: ^(#) S	- Soil orig	in is "probable" unless otherwise stated. $^{\circ}$ Consistency/Relative density shac	ling is for vi	isual refer	ence only -	no correla	ation between o	cohesive	e and gra	anular m	aterials i	s implied.
PLA	NT	Sho	ovel and Hand Auger		C	PERA	FOR:	Kramer					LOGGED: Kramer

METHOD: Shovel to 0.3m then hand auger to 0.75m

EXPORTED 20/04/23 12:11. TEMPLATE ID: DP_101.02.00_SOILLOG



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.0 AHD COORDINATE E:383920.0 N: 6355606.0 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 203 DATE: 18/08/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SA	MPLE				TESTING AND REMARKS
observed GROUNDWATER	r RL (m)	0.0	DESCRIPTION OF STRATA FILL/ SAND, with silt, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium, sub-angular to sub-rounded; trace	GRAPHIC	(#) NISINO FILL		D to M	REMARKS	т УРЕ	INTERVAL	DEPTH (m)		RESULTS AND REMARKS
ee groundwater	-	0.2 - 0.3 -	rootlets FILL/ Gravelly SAND; pale brown; sand fraction fine to medium; gravel fraction fine to medium, sub-angular to sub-rounded		FILL	NA	D to M	-	D E		-0.25-	-PID-	<1
No fr	-	0.5 -	Borehole discontinued at 0.50m depth Limit of investigation		ALV		M		D		- 0.5 -	PID	<1
EXPORTED 20/04/23 12:12. TEMPLATE 10: UP_101.05.00_SUILUGS		- - - - - - - - - - - - - - - - - - -											
PLA	NT:	Hai	nd Tools (75mm diameter hand auger)	ung io IUI VIS		PER		Kramer	- JUNESIVE	, anu gr	anuidi III		LOGGED: Kramer



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.1 AHD COORDINATE E:383922.4 N: 6355610.1 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 203A DATE: 29/11/22 SHEET: 1 of 1

	1			CONDITIONS ENCOUNTERED	T	1	1		SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)			DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
Iter observed	-2	0.0)	FILL/ (SP) Silty SAND; brown; fine to medium; trace roots / rootlets, plastic		FILL		D to M		D E	X	~0.0 ~ -0.05-	-PID-	<1
ee groundwa	-	0.2	3	FILL/ (SP) Gravelly SAND; brown pale brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded (crushed natural rock)	0	FILL		D		D E		-0.25-	-PID-	<1
No fr	-		-	(SP) SAND; pale grey; fine to medium		ALV	NA	D to M		D		- 0.5 -	-PID-	<1
		0.8	3 -	Borehole discontinued at 0.80m depth Limit of investigation	<u></u>	<u>, </u>		II		<u> </u>				
			-									- 1 -		
	-		-											
	-		-									- ·		
	-		-											
	-		-											
	-	2	2									- 2 -		
90 TILL 06	-0		-											
.20.101_4U	-		-											
MPLAIE ID:	-		-											
8 12:17. IE	-													
ED 20/04/2	-													
	- =S ^{. (#),}	Soil	riair	n is "probable" unless otherwise stated ""Consistency/Relative density shad	ing is for vi	isual refer	ence only	- no correla	tion between	cohesive	and or	anular m	ateriale	is implied
PLA		: H	an H	d Auger and auger to 0.8m		C	PERA	TOR: K	Gramer		9'			LOGGED: Kramer



School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South

CLIENT:

EXPORTED 20/04/23 12:13. TEMPLATE ID: DP_101.02.00_SOILLOG

SURFACE LEVEL: 3.0 AHD COORDINATE E:383944.9 N: 6355699.7 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 204 PROJECT No: 213618.02 DATE: 11/11/22 SHEET: 1 of 1

Bit Mark End of the transmission of the transmissis of transmission of the transmission of the transmissio				CONDITIONS ENCOUNTERED	1		-		SAN	IPLE				TESTING AND REMARKS
0 0.0 FILU (SP) SND: brown grey, fine to medium; subcounded gravel (crushed natural rock), glass, dry 0.1 0.4m; fine to medium subcongular to subcounded gravels (crushed natural rock) NA D to M D t	GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
Subrounded gravels (crushed natural rock) 0.7 FILL/(SP) Silty SAND; dark brown; fine to subrounded gravels (crushed natural rock) NA Dis M Dis M </td <td>free groundwater observed</td> <td>-</td> <td>0.0</td> <td>FILL/ (SP) SAND; brown grey; fine to medium; trace fine to medium grained, subangular to subrounded gravel (crushed natural rock), glass, dry</td> <td></td> <td>FILL</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>-0.05-</td> <td>-PID-</td> <td><1</td>	free groundwater observed	-	0.0	FILL/ (SP) SAND; brown grey; fine to medium; trace fine to medium grained, subangular to subrounded gravel (crushed natural rock), glass, dry		FILL						-0.05-	-PID-	<1
medium; frace fine to medium subangular to subrounded graves (crushed natural rock), ceramic, slag, coal reject, brick fragments, dry Borehole discontinued at 0.95m depth Hand refusal on cobbles	No	-		FILL/ (SP) Silty SAND; dark brown; fine to			NA	D to M		Ē		- 0.5 -	-PID-	<1
Porchole discontinued at 0.85m depth 1 Hand refusal on cobbles 1 1 Hand refusal on cobbles 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		-	0.05	medium; trace fine to medium subangular to subrounded gravels (crushed natural rock), ceramic, slag, coal reject, brick fragments, dry	· · · · · ·	FILL				D E		-0.8-	-PID-	<1
NOTES: ⁽⁹⁾ Soil origin is "probable" unless otherwise stated. ⁽¹⁾ Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.	NOTE		1	Borehole discontinued at 0.95m depth Hand refusal on cobbles	ding is for v	isual refer	rence only	- no correla	tion between 0	cohesive	and gran	- 1 - 		s implied.
, , ,, ,,,,,,	PLA	NT	: Hai	nd Auger	Jo. v	C	OPERA	TOR: H	Kramer		91			LOGGED: Kramer
	PLA MET	NT: HO	: Hai D: H	nd Auger land auger to 0.95m		C	PERA ASING	TOR: H	Kramer ased					LOGGED: Kramer



CLIENT:School Infrastructure NSWPROJECT:Newcastle High School UpgradeLOCATION:Parkway Avenue, Hamilton South

BOREHOLE LOG

SURFACE LEVEL: 3.2 AHD COORDINATE E:383930.1 N: 6355710.6 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 205 PROJECT No: 213618.02 DATE: 11/11/22 SHEET: 1 of 1





CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South SURFACE LEVEL: 3.6 AHD COORDINATE E:383907.7 N: 6355729.6 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 206 DATE: 11/11/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED	1	[-		SAM	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
oundwater observed	-	0.0	FILL/ (SP) SAND; grey brown; fine to medium; trace fine to medium subangular to subrounded gravels (crushed natural rock), ash, rootlets, dry		FILL				D E		-0.05-	-PID-	<1
No free gr		-	(SP) SAND; grey; fine to medium; trace rootlets, dry		ALV	NA	D		D E		- 0.5 -	PID	<1
	- (0.65	(SP) Silty SAND; brown to dark brown; fine to medium; dry (possible indurated sand)		ALV				D E		- 0.7 -	-PID-	<1
	- 1	0.95	(SP) SAND; pale brown; fine to medium; dry		ALV						_10_		<1
	s: ‴S NT•	ioil orig	n is "probable" unless otherwise stated. ^{\/} Consistency/Relative density shared Auroper	ding is for vi	sual refer			Ation between	cohesive	e and gra	anular m	aterials i	s implied.
MET	HO	D: Har	land auger to 1.0m		C	ASING	Unc	ased				l	

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using a differential GPS, typical accuracy ±0.1m.



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TEST PIT LOG

CLIENT:

School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 3.6 AHD COORDINATE E:383907.4 N: 6355729.8 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 206A PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1




BOREHOLE LOG

SURFACE LEVEL: 3.4 AHD COORDINATE E:383855.8 N: 6355704.8 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 207 PROJECT No: 213618.02 DATE: 11/11/22 SHEET: 1 of 1



REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using a differential GPS, typical accuracy ±0.1m.



EXPORTED 20/04/23 12:15. TEMPLATE ID: DP_101.02.00_S0ILL0G

CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South

BOREHOLE LOG

SURFACE LEVEL: 3.7 AHD COORDINATE E:383885.2 N: 6355697.1 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 208 PROJECT No: 213618.02 DATE: 11/11/22 SHEET: 1 of 1



REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using a differential GPS, typical accuracy ±0.1m.



EXPORTED 20/04/23 12:15. TEMPLATE ID: DP_101.02.00_S0ILL0G

CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.0 AHD COORDINATE E:383812.0 N: 6355647.1 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 209 DATE: 11/11/22 SHEET: 1 of 1

	_		CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
No free groundwater observed	-	0.0	FILL/ (SP) SAND; grey; fine to medium; with fine to coarse subangular to subrounded gravel (crushed natural rock), trace glass, sandstone cobbles, coal reject, dry 0.2-0.3m: fibro fragment observed— FILL/ (SP) Gravelly SAND, with gravel; grey brown; sand fraction fine to medium; gravel fraction fine to medium, subangular to subrounded; crushed natural rock, dry FILL/ (SP) SAND; brown; fine to medium; trace fine to medium subangular to subrounded gravels (crushed natural rock), dry (SP) SAND; pale grey; fine to medium; dry to moist		FILL FILL FILL	ΝΑ	М				-0.05 -0.25 -0.35 -0.45 -0.45 -0.7 -	PID - PID - -PID - - -PID	<1 <1 <1 <1 <1 <1 <1 <1 <1 <1 <1 <1 <1 <
	-	0.85	(SP) SAND; dark brown; fine to medium; dry to moist (indurated sand)		ALV								
NOTES		- - - - - - - - - - - - - - - - - - -	n is "probable" unless otherwise stated. "Consistency/Relative density shace	ling is for vi	sual refer	rence only -	no correla	ation between o		- and grade and gra			is implied.
		Har	nd Auger					LOGGED: Kramer					

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using a differential GPS, typical accuracy ±0.1m.



EXPORTED 20/04/23 12:16. TEMPLATE ID: DP_101.02.00_SOILLOG

TEST PIT LOG

 SURFACE LEVEL:
 4.0 AHD

 COORDINATE
 E:383810.8 N: 6355648.0

 DATUM/GRID:
 MGA94 Zone 56

LOCATION ID: 209A PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED	1	1	1		SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
groundwater observed	-4	0.0	FILL/ (SP) SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded (crushed natural rock); trace rootlets, ash		FILL	NA	D		D E B D E		-0.05	-PID-	<1
No free		0.35 0.45	FILL/ (SP) Gravelly SAND; brown pale brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed /	0	FILL		D		D E		-0.35- -0.4-	PID- PID-	<1 <1
	-	-	natural rock) (SP) SAND; pale grey; fine to medium		ALV		D to M					-	
		2 - - - - - - - - - - - - - - - - - - -	Test pit discontinued at 0.65m depth Limit of investigation								- 1 -		
	-	-											
NOTES	5: ^(#) S	oil orig	in is "probable" unless otherwise stated. ^{(*/} Consistency/Relative density shadi	ing is for vi	isual refer	ence only -	- no correla	tion between	cohesive	e and gr	anular m	aterials	is implied.
PLA	NT: HO	T: Shovel OPERATOR: Kramer LOGGED: Kramer IOD: Shovel to 0.65m 0.65m											

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using differential GPS, typical accuracy ±0.1m.



EXPORTED 20/04/23 12:16. TEMPLATE ID: DP_101.02.00_SOILLOG

School Infrastructure NSW

PROJECT: Newcastle High School Upgrade **LOCATION:** Parkway Avenue, Hamilton South

CLIENT:

EXPORTED 20/04/23 12:16. TEMPLATE ID: DP_101.02.00_SOILLOG

TEST PIT LOG

SURFACE LEVEL: 4.1 AHD COORDINATE E:383813.1 N: 6355646.8 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 209B PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED			1		SAN	IPLE	1			TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
idwater observed	-4	0.0	FILL/ (SP) SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); trace rootlets, brick, plastic		FILL		D		D E B D E		-0.05-	-PID-	<1 <1
No free grour	-	-	FILL/ (SP) SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded (crushed natural rock); trace brick, slag		FILL	NA	D		B D E		- 0.5 -	-PID-	<1
	-	0.6	(SP) SAND; pale grey; fine to medium; trace rootlets		ALV	-	D			L	-0.6-	-PID-	<1
		1	Test pit discontinued at 0.70m depth Limit of investigation										
NOTE	 S: ^(#) S	Soil orig	in is "probable" unless otherwise stated. ^(?) Consistency/Relative density sha	ding is for vi	sual refer	ence only -	no correl	ation between	cohesive	e and gr	anular m	aterials	is implied.
PLA MET	NT: HC	NT: Shovel OPERATOR: Kramer LOGGED: Kramer HOD: Shovel to 0.7m 0.7m											



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 Parkway Avenue, Hamilton South

SURFACE LEVEL: 3.9 AHD COORDINATE E:383810.1 N: 6355646.7 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 209C PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED	1				SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
observed	-	0.0 0.1 -	FILL/ (SP) SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed	 	FILL		D		D E B		-0.05-	-PID-	<1
oundwater	- ().25	natural rock); trace rootlets FILL/ (SP) SAND, with silt; brown; fine to medium; trace metal, glass, plastic, fine to	 	FILL		D		D E	Z	- 0.2 - -0.25-	-PID- -PID-	<1 <1
Vo free gr	-	-	medium sub-angular to sub-rounded gravels (crushed natural rock) FILL/ (SP) SAND, trace gravel; brown dark			NA							
	-	-	brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock)		FILL		D		D E		- 0.5 -	-PID-	<1
	-	.05	(SP) SAND; pale grey; fine to medium		ALV	-	D to M		D		-0.7-	-PID-	<1
	-	0.8 0.85	(SP) SAND; dark brown; fine to medium		ALV		D to M						
	- 0	1-	Limit of investigation								- 1 -		
		-											
	-	-										-	
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	L	-											
NOTE) S: ^(#) S	oil oria	n is "probable" unless otherwise stated. ^{(*} Consistency/Relative density shad	ling is for v	isual refer	ence only	- no correlati	ion between r	ohesive	and or	anular m	aterials i	s implied.
PLA	NT:	Sho	hovel OPERATOR: Kramer										LOGGED: Kramer

METHOD: Shovel to 0.85m

EXPORTED 20/04/23 12:17. TEMPLATE ID: DP_101.02.00_SOILLOG



TEST PIT LOG

SURFACE LEVEL: 4.0 AHD COORDINATE E:383809.5 N: 6355649.2 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 209D PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1





CLIENT:

School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 3.9 AHD COORDINATE E:383812.4 N: 6355649.9 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 209E DATE: 20/12/22 SHEET: 1 of 1

	1		CONDITIONS ENCOUNTERED	1	1			SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
ndwater observed	-	0.0	FILL/ (SP) Silty SAND, trace gravel; brown grey; sand fraction fine to medium; gravel fraction fine to coarse sub-angular, sub-rounded, angular (crushed natural rock); rootlets, asphalt, brick fragments	· · · · ·	FILL		D		D E D E		-0.05-	PID-	<1
No free grour	-	-	FILL/ (SP) SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded (crushed natural rock); trace slag and ash		FILL	NA	м		B		- 0.5 -	PID	<1
	-	0.55	(SP) SAND; pale brown; fine to medium		ALV	2	D		Ш		0.7	BID	-1
	ł	0.75	Test pit discontinued at 0.75m depth Limit of investigation	<u>.</u>	[_ 0.7 _		
	 -	1-									- 1 -	-	
	-	-										-	
	-	-											
		-											
	-											-	
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27.00_SUILLU	-	-										-	
	-	-											
. IEMPLAIE I	-	-										-	
11:71 27/70	-	-											
EXPURIED 20/		-									-		
	S: #	Soil orig	in is "probable" unless otherwise stated. ^(*) Consistency/Relative density shac DVCI 2 Doviel to 0.75m	ling is for vi	isual refer	ence only -	no correla	ation between o	cohesive	e and gr	anular m	aterials	is implied. LOGGED: Kramer



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.0 AHD COORDINATE E:383909.5 N: 6355620.0 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 210 DATE: 11/11/22 SHEET: 1 of 1

	CONDITIONS ENCOUNTERED			-		SAN	IPLE				TESTING AND REMARKS
GROUNDWATER RL (m) DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#) CONSIS. ^(*)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
Part of the second seco	FILL/ (GP) Sandy GRAVEL; fine, sub-angular to sub-rounded; with fine to medium grained sand, dry (crusher dust) 0.07m: black hessian dividing layer FILL/ (SP) Silty SAND; brown grey; fine to medium; trace fine to medium subangular to subrounded gravels (crushed natural rock), ash, dry (SP) SAND; grey; fine to medium; trace rootlets, dry 0.5m: pale grey, dry to moist		FILL FILL	NA	D		D E E D E		-0.25-	-PID- -PID-	<1 <1 <1
- m 1 - m 1	Borehole discontinued at 0.75m depth Limit of investigation	ng is for visua	al reference	e only -	no correla	ation between o	cohesive	e and gra			s implied.
PLANT: Ha	nn is proveoue unless otherwise stated. "Consistency/Relative density shadi nd Auger Hand auger to 0.75m	ng is for visua			OR: 1	Kramer	UNESIVE	and gr	anular ma	alerials i	s mpried. LOGGED: Kramer

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using a differential GPS, typical accuracy ±0.1m.



EXPORTED 20/04/23 12:18. TEMPLATE ID: DP_101.02.00_SOILLOG

CLIENT:

School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 3.7 AHD COORDINATE E:383929.6 N: 6355659.2 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 211 PROJECT No: 213618.02 DATE: 11/11/22 SHEET: 1 of 1





School Infrastructure NSW

PROJECT: Newcastle High School Upgrade **LOCATION:** Parkway Avenue, Hamilton South

CLIENT:

EXPORTED 20/04/23 12:19. TEMPLATE ID: DP_101.02.00_S0ILL0G

BOREHOLE LOG

SURFACE LEVEL: 8.2 AHD COORDINATE E:383936.9 N: 6355670.8 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 212 PROJECT No: 213618.02 DATE: 11/11/22 SHEET: 1 of 1





CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South SURFACE LEVEL: 3.7 AHD COORDINATE E:383864.8 N: 6355643.5 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 213 DATE: 29/11/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED	1	1	- <u>-</u>		SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
No free groundwater observed	-	0.0	FILL/ (SP) SAND, with silt, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock) FILL/ (SP) Gravelly SAND, with silt; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); trace brick	00000000	FILL	NA	D			₹ 	-0.05 -0.25 -0.25	PID	<1 <1
		0.6 - - - - - - - - - - - - - - - - - - -	Borehole discontinued at 0.60m depth Hand refusal										
NOTES	5: ^(#) S	oil orig	in is "probable" unless otherwise stated. ^(*) Consistency/Relative density shar	ling is for vi	isual refer	ence only -	no correl	ation between	cohesive	and gr	anular m	aterials i	s implied.
PLA MET	NT: HO	Haı D: H	Hand AugerOPERATOR: KramerD: Hand auger to 0.6mCASING:										LOGGED: Kramer

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using a differential GPS, typical accuracy ±0.1m.



EXPORTED 20/04/23 12:19. TEMPLATE ID: DP_101.02.00_SOILLOG

CLIENT:School Infrastructure NSWPROJECT:Newcastle High School UpgradeLOCATION:Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.0 AHD COORDINATE E:383863.7 N: 6355620.9 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

LOCATION ID: 214 PROJECT No: 213618.02 DATE: 29/11/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED	I	1	-		SAM	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ⁽¹⁾	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
No free groundwater observed	-	0.0	FILL/ (SP) Silty SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); abundant rootlets 0.1m: possible pavement Borehole discontinued at 0.10m depth Hand refusal		FILL	NA	D		E	<u> </u>	-0.05	-PID-	<1
		- - 1-											
	-	-									 		
22.00_S01LL05											- 2 -		
04/23 12:20. IEMPLAIE 1U: UP_101.4	-	-									· · ·		
		Soil orig	in is "probable" unless otherwise stated. ^{*/} Consistency/Relative density shad DVEI	ing is for v	isual refer		no correla OR: K	tion between	cohesive	and gr	anular ma	terials i	is implied. LOGGED: Kramer

REMARKS: Bore through garden bed. Location coordinates are in MGA94 Zone 56. Coordinates obtained using a differential GPS, typical accuracy ±0.1m.



TEST PIT LOG

 SURFACE LEVEL:
 3.9 AHD

 COORDINATE
 E:383864.6 N: 6355623.4

 DATUM/GRID:
 MGA94 Zone 56

LOCATION ID: 214A PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED			-		SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
oundwater observed	-	0.0	FILL/ (SP) Silty SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium, sub-angular to sub-rounded (crushed natural rock); abundant rootlets	· · · · · · · · ·	FILL		М		D	$ \prec $	-0.05	-PID-	<1
No free arc	-	0.1	Test pit discontinued at 0.10m depth Limit of investigation		~~~~								
	-												
	-												
	-												
	-	-											
0	-												
101.02.00_S0ILL0	-												
0. TEMPLATE IU: U	-												
TED 20/04/23 12:24	-0												
	ES: #	Soil orig	in is "probable" unless otherwise stated. ^(*) Consistency/Relative density shac	ling is for visu	al referei	nce only -	no correla	tion between o	cohesive	e and gra	anular ma	aterials	is implied.
PL	ANT	: Ha	nd Auger		OF	PERAT	OR: K	Gramer		-			LOGGED: Kramer



TEST PIT LOG

 SURFACE LEVEL:
 4.0 AHD

 COORDINATE
 E:383866.0 N: 6355621.3

 DATUM/GRID:
 MGA94 Zone 56

LOCATION ID: 214B PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED				SAN	IPLE				TESTING AND REMARKS
oundwater observed GROUNDWATER	4 RL (m)	DEPTH (m)	DESCRIPTION OF STRATA FILL/ (SP) Silty SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium, sub-angular to sub-rounded (crushed natural rock); abundant rootlets	GRAPHIC		MOISTURE	REMARKS	TYPE		(m) DEPTH (m)		RESULTS AND REMARKS
		Soil origination	Test pit discontinued at 0.10m depth Limit of investigation	ling is for vis	nce only -	no correla ī OR: k	tion between (ramer	cohesive	and gra	 	aterials	is implied.

METHOD: Hand auger to 0.1m



TEST PIT LOG

SURFACE LEVEL: 2.3 AHD COORDINATE E:383989.8 N: 6355656.7 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 215 PROJECT No: 213618.02 DATE: 30/11/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
No free groundwater observed		0.0	FILL/ (SP) Silty SAND, with gravel; brown grey; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); trace rootlets, asphalt, glass, ceramic, metal		FILL		D				-0.25 -0.5-	-PID- -PID-	<1 <1
	-	0.55	FILL/ (SP) SAND, trace gravel; pale grey pale brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock)		FILL	NA	D		E D E		- 0.7 -	-PID-	<1
	-	- 1 - -	FILL/ (SP) SAND, with silt; intermixed brown pale brown orange; fine to medium 1.1m: with fine to coarse sub-angular to		FILL		Μ		D E		- 1.0-	-PID-	<1
	- 0		Test pit discontinued at 1.30m depth Hand refusal on gravels										
	s: [#] 's N T :	"Soil origin is "probable" unless otherwise stated. "Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied. IT: Shovel and Hand Auger OPERATOR: Kramer											

METHOD: Shovel to 0.3m then hand auger to 1.3m

EXPORTED 20/04/23 12:21. TEMPLATE ID: DP_101.02.00_SOILLOG



TEST PIT LOG

SURFACE LEVEL: COORDINATE E: N: DATUM/GRID: MGA94 Zone 56 LOCATION ID: 216 PROJECT No: 213618.02 DATE: 29/11/22 SHEET: 1 of 1

Bit Mono Description Discription Discription <thdiscription< th=""> <thdiscription< th=""> <t< th=""><th>PLE</th><th>TESTING AND REMARKS</th></t<></thdiscription<></thdiscription<>	PLE	TESTING AND REMARKS
PILU (SP) Sitty SAND. trace gravel; brown; sand <u>i i i i i Piu</u> raction fine to medium; guere fraction fine to medium; guere fraction; gue	TYPE INTERVAL DEPTH (m)	E L RESULTS AND E L REMARKS
NOTES: ^{(III}) Solid origin is "probable" unless otherwise stated. ⁽¹⁾ Consistency/Relative density shading is for visual reference only - no correlation between col		

METHOD: Shovel to 0.1m



TEST PIT LOG

SURFACE LEVEL: COORDINATE E: N: DATUM/GRID: MGA94 Zone 56 LOCATION ID: 216A PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1

				CONDITIONS ENCOUNTERED	1 1		_		SAN	IPLE				TESTING AND REMARKS
	GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
	undwater observed		0.0	FILL/ (SP) Silty SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium, sub-angular to sub-rounded (crushed natural rock)		FILL		D to M		D E	-	-0.05-	-PID-	<1
	No free g		-	Test pit discontinued at 0.10m depth Limit of investigation										
			-											
			_											
: DP_101.02.00_S01LL0G			-											
ED 20/04/23 12:21. TEMPLATE ID.			-											
EXPORTE		5: ^{##} S	oil orig	in is "probable" unless otherwise stated. ^{I*/} Consistency/Relative density shac	ling is for visi	ual refere	nce only -	• no correlat	ion between	cohesive	e and gra	anular ma	aterials i	s implied.
Ň		но	ים. חי										1	



TEST PIT LOG

SURFACE LEVEL: COORDINATE E: N: DATUM/GRID: MGA94 Zone 56 LOCATION ID: 217 PROJECT No: 213618.02 DATE: 29/11/22 SHEET: 1 of 1

				CONDITIONS ENCOUNTERED	1	1			SAM	/IPLE				TESTING AND REMARKS
	GROUNDWALEK	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
		<u>2</u> 0.		FILL/ (SP) Silty SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium subangular to subrounded; (crushed natural rock), dry to moist Test pit discontinued at 0.10m depth Limit of investigation		FILL	NA	D to M			= <u></u>		<u>-</u> РID-	<1
ID: DP_101.02.00_S0ILL06			2-									- 2 -		
EXPORTED 20/04/23 12:22. TEMPLATE <	DIES	(^{#)} Soil 4	- - - -	n is "probable" unless otherwise stated ^(*) Consistency/Relative density shard	ing is for v	jsual refer	ence only -	No correla	tion between	cohesive	and or		aterials	s implied.
PL		IT: 8	Shc Shc	ivel	-	C	PERAT	r or : K	Cennedy				I	LOGGED: Kramer



TEST PIT LOG

SURFACE LEVEL: COORDINATE E: N: DATUM/GRID: MGA94 Zone 56 LOCATION ID: 217A PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1

				CONDITIONS ENCOUNTERED	1 1		_		SAN	IPLE				TESTING AND REMARKS
	GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	ΤΕ ST ΤΥΡΕ	RESULTS AND REMARKS
	undwater observed		0.0	FILL/ (SP) Silty SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium, sub-angular to sub-rounded (crushed natural rock)	· · · · · · · · · ·	FILL		D to M		D E	-	-0.05-	-PID-	<1
	No free grou			Test pit discontinued at 0.10m depth Limit of investigation	<u>}</u> ×					<u> </u>		· · ·		
EXPORTED 20/04/23 12:22. TEMPLATE ID: DP_101.02.00_SOILLOG	OTES	S: ^(#) S	- - - oil orig	in is "probable" unless otherwise stated. ⁿ Consistency/Relative density shac	ling is for visu	ual referen	nce only -	no correlat	ion between	cohesive	e and gra		aterials	is implied.
F			Hai •	nd Tools		O	PERAT	for: K	ramer					LOGGED: Kramer



TEST PIT LOG

SURFACE LEVEL: COORDINATE E: N: DATUM/GRID: MGA94 Zone 56 LOCATION ID: 218 PROJECT No: 213618.02 DATE: 29/11/22 SHEET: 1 of 1

	_		CONDITIONS ENCOUNTERED		1			SAM	IPLE				TESTING AND REMARKS
		RL (m) DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	ΤΕ ST ΤΥΡΕ	RESULTS AND REMARKS
		0.0 0.15	FILL/ (SP) Silty SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium subangular to subrounded; (crushed \natural rock), dry to moist	· · · · ·	FILL	NA	D to M		E	~	⊂0.0 ∽ -0.05-	-PID-	<1
			Test pit discontinued at 0.15m depth Limit of investigation										
		1									- 1 -		
00_DULLUU		2.									- 2 -		
. 101_101.02.													
IEMPLAIE IU.													
04/23 12:23.													
EXPURIED 20/1													
NO PL	TES: .AN	"Soil ori	jin is "probable" unless otherwise stated. ^{("} Consistency/Relative density shac OVel Showel to 0.15m	ing is for vi	isual refer	PERA	no correlat	tion between	cohesive	and gr	anular ma	aterials	is implied.



TEST PIT LOG

SURFACE LEVEL: COORDINATE E: N: DATUM/GRID: MGA94 Zone 56 LOCATION ID: 219 PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1

	1		CONDITIONS ENCOUNTERED	I		-		SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	ΤΕ ST ΤΥΡΕ	RESULTS AND REMARKS
No free groundwater observed GROUND	Br (m)	HEA30 0.0 0.05 - - - - - - - - - - - - - -	PILL/ (SP) Silty SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium; sub-angular to sub-rounded (crushed hatural rock) Test pit discontinued at 0.05m depth Limit of investigation	GRAPH	NIDINO			REMAR					RESULTS AND REMARKS
NOTE	S: (#)S	- Soil orig	in is "probable" unless otherwise stated. ^{**} Consistency/Relative density shad	ing is for v	isual refer	rence only	• no correlat	ion between c	cohesive	e and gra	anular ma	aterials	is implied.
PLA	NT: HC	: Har סוי.	nd Tools		C	OPERA	for : K	ramer					LOGGED: Kramer

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using differential GPS, typical accuracy ±0.1m.



EXPORTED 20/04/23 12:23. TEMPLATE ID: DP_101.02.00_SOILLOG

TEST PIT LOG

SURFACE LEVEL: COORDINATE E: N: DATUM/GRID: MGA94 Zone 56 LOCATION ID: 220 PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED		-		SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
observed		0.0	FILL/ (SP) Silty SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium, sub-angular to sub-rounded (crushed		×	D to M		D	\nearrow	-0.05-	-PID-	<1
No free groundwater o		0.05	Test pit discontinued at 0.05m depth Limit of investigation							-0.05		
NOTES	S: ^(#) S	Soil orig	in is "probable" unless otherwise stated. $^{\circ \circ}$ Consistency/Relative density shad	ling is for visual refe	erence only -	no correlat	ion between o	cohesive	e and gra	anular ma	aterials i	s implied.
PLA	NT HC	: Hai חוי.	nd Tools		OPERA	for: K	ramer					LOGGED: Kramer

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using differential GPS, typical accuracy ±0.1m.



EXPORTED 20/04/23 12:24. TEMPLATE ID: DP_101.02.00_S0ILL0G

CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 3.0 AHD COORDINATE E:383938.6 N: 6355703.5 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 221 DATE: 20/12/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED		1			SAM	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
No free groundwater observed		0.0 .06 .11 -	FILL/ (SP) Silty SAND; brown; fine to medium; with rootlets FILL/ (GP) Sandy GRAVEL, with silt, with slag; brown; medium to coarse, angular to sub-angular, (crushed natural rock); trace brick fragments, glass shards, ceramic shards FILL/ (SP) Silty SAND, with gravel; brown dark brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); trace coal reject, slag (possibly coal tar asphalt), brick fragments		FILL	NA	M M M	coal tar sample			-0.05 -0.05 -0.1- -0.3- -0.5- -0.6- -0.7-	PID PID PID PID	<1 <1 <1 <1 <1 <1
NOTE		1	Test pit discontinued at 0.75m depth Refusal on brick	ting is for vi	isual refere	ence only -	no correla	tion between c	cohesive	e and gra	- 1 - 		s implied.
PLA	אד: אד:	Har	nd Auger	ıy iə iUI VI	0	PERAT	OR: 1	Kramer/He	elbig	. unu yli	andial III	aondið	LOGGED: Kramer

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using differential GPS, typical accuracy ±0.1m.



EXPORTED 20/04/23 12:24. TEMPLATE ID: DP_101.02.00_S0ILL0G

 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.8 AHD COORDINATE E:383951.7 N: 6355696.3 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 222 PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
undwater observed GROUNDWATER	RL (m)	0.0 DEPTH (m)	DESCRIPTION OF STRATA FILL/ (SP) Silty SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); with rootlets	GRAPHIC			■ MOISTURE	REMARKS	TYPE		(u) ■ DEBTH ■ -0.0		RESULTS AND REMARKS
No free gro		0.3 -	FILL/ (SP) SAND, with silt, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular (crushed natural rock); trace slag, concrete pieces, brick (half bricks and fragments), possible coal tar asphalt fragments		FILL	NA	Μ		D E E		- 0.4	PID-	<1
	-	0.9 -	(SP) SAND; brown pale brown; fine to medium		ALV		М		D		- 1.0	PID	<1
NOTES		- - - - - - - - - - - - - - - - - - -	n is "probable" unless otherwise stated. ^{*/} Consistency/Relative density shace	ling is for v	isual refer	rence only -	no correla	ation between o	cohesive	and grad	- 2 - - 2 - 		s implied.
PLAI		Sho	ovel to 0 5m then hand auger to 1 1m			PERAT		Kramer/He	elbia	and yld	anondi ille		LOGGED: Kramer

METHOD:

EXPORTED 20/04/23 12:25. TEMPLATE ID: DP_101.02.00_SOILLOG



CLIENT:School Infrastructure NSWPROJECT:Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

 SURFACE LEVEL:
 1.1 AHD

 COORDINATE
 E:384002.6 N: 6355660.0

 DATUM/GRID:
 MGA94 Zone 56

LOCATION ID: 223 PROJECT No: 213618.02 DATE: 20/12/22 SHEET: 1 of 1

				CONDITIONS ENCOUNTERED					SAM	PLE				TESTING AND REMARKS
GROUNDWATER	RL (m)		UEPIH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
oserved		0.0 0.1	0 1 -	FILL/ (SP) Silty SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to		FILL		М		E	\prec	-0.05	-PID-	<1
vater of	-	0.2	2 -	natural rock); trace rootlets		FILL		М		D E	Γ	-0.15- -0.2-	-PID- -PID-	<1 <1
ground	-		-	rootlets	• • • • • • • • •		NA			D	\rightarrow	-0.3-	-PID-	<1
No free	-		-	to medium; trace glass shards, brick fragments, ceramic shards, ash, slag, bolts, copper coil, plastic (hard)	· [· [·] ·] · [·] ·] ·]	FILL		Μ	D2/LAH	B D E		- 0.5 -	-PID-	<1
	ŀ	0.6	6 +	Test pit discontinued at 0.60m depth	1.1.1.1							-0.6-	-PID-	<1
	-		-	Limit of investigation										
	Ī		-											
			-											
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	s: ^(#)	Soil	oriai	n is "probable" unless otherwise stated ^(*) Consistency/Relative density shar	ling is for v	visual refer	ence only -	no correle	ation between c	ohesive	and or	anular m	aterials	is implied.
PLA	NT	: S	Shc	ovel to 0.6m		C	PERA		Kramer/He	lbig	und gi			LOGGED: Kramer

METHOD:

EXPORTED 20/04/23 12:25. TEMPLATE ID: DP_101.02.00_SOILLOG



School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South

CLIENT:

SURFACE LEVEL: 1.8 AHD COORDINATE E:384022.4 N: 6355645.2 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 224 DATE: 20/12/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED			-		SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
No free groundwater observed	-	0.0	FILL/ (SP) Silty SAND; brown; fine to medium; trace rootlets, ceramic shards, slag, metal shards, glass shards, coal reject		FILL	NA	М		B		-0.05 -0.05 	PID	<1
	- 	0.00 - - - 1 0 -	FILL/ (SP) SAND; pale brown; fine to medium; trace rootlets		FILL		М		D E		-0.7-	PID	<1 <1
	-	1.0	FILL/ (SP) SAND, with clay, with gravel; dark brown dark grey; sand fraction fine to medium;	_	FILL		М		D		- 1.1 -	-PID-	<1
	-	1.2 -	gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock) 1.1m: From 1.1m. trace ceramic and ash				W		E				
	s: "'S	oil orig	in is "probable" unless otherwise stated. 'Consistency/Relative density shar	aing is for v	ISUAL REFER	ence only -	no correla	tion between	elbia	and gra	anular ma	aterials i	s implied.

METHOD:

EXPORTED 20/04/23 12:26. TEMPLATE ID: DP_101.02.00_S0ILL0G



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South SURFACE LEVEL: 2.9 AHD COORDINATE E:384047.2 N: 6355627.6 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 225 DATE: 20/12/22 SHEET: 1 of 1

	_		CONDITIONS ENCOUNTERED	1 1				SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEDTH (m)	DESCRIPTION OF STRATA	GRAPHIC		CONSIS. ^(*)	MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
	-	0.0	FILL/ (SP) Silty SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded (crushed natural rock); trace rootlets, slag, ash, ceramic shards, glass shards		FILL		М		D E B D E		- 0.5 -	PID	<1
	7	0.6	FILL/ (SP) SAND; pale brown; fine to medium; trace rootlets, ash, ceramic shards		FILL	NA -	М		D		- 0.6 -	-PID-	<1
observed	-	1.0	FILL/ (SP) SAND, with clay, trace gravel; dark brown dark grey; sand fraction fine to medium; gravel fraction fine to medium sub-rounded (crushed natural rock); pockets of red brown low plasticity sandy clay		FILL		M W	D3/LAH	D E		- 1 -	-PID-	<1
04/23 12:26. LEMPLALE 10: DP_101.02.00_501LL06 Free ground		2	<pre>I lest pit discontinued at 1.20m depth Limit of investigation I I I I I I I I I I I I I I I I I I</pre>								- 2 -		
	-O	Soil o	gin is "probable" unless otherwise stated. ⁽¹⁾ Consistency/Relative density shad	ing is for vis	sual refer	rence only -	no correla	ation between o	cohesive	and gr	anular m	aterials	is implied.
PLA	NT	:н	and Tools		c	PERAT	OR: 1	Kramer/He	elbia				LOGGED: Kramer

METHOD: Shovel to 0.6m then hand auger to 1.2m



CLIENT:

School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.1 AHD COORDINATE E:384041.4 N: 6355631.2 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 301 PROJECT No: 213618.02 DATE: 04/10/22 SHEET: 1 of 1





SURFACE LEVEL: 2.1 AHD COORDINATE E:384012.6 N: 6355654.8 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 302 PROJECT No: 213618.02 DATE: 04/10/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAM	PLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
se groundwateer observed	-0	0.0	FILL/ (ML) SILT, trace sand; brown; silt fraction low plasticity; sand fraction fine to medium grain; trace rootlets, glass, slag, ash, ceramic, fine to coarse sub-angular to sub-rounded gravel		FILL		W to <pl< td=""><td></td><td>D E D E</td><td></td><td>-0.05</td><td>-PID-</td><td><1</td></pl<>		D E D E		-0.05	-PID-	<1
No fr	-	0.4 -	FILL/ (SP) Silty SAND; brown; fine to medium; with fine to medium sub-angular to sub-rounded gravels (Cnr), slag gravels, trace ceramic, coal reject, and rootlets		FILL		D to M		D E		- 0.5	-PID-	<1
		0.8 -	FILL/ (SP) SAND, with silt; grey; fine to medium; fine to medium sub-angular to sub-rounded gravels, trace ceramic, metal, wire and organics		FILL	NA	М	-	D E		- 1.0 -	-PID-	<1
	-	1.2	FILL/ (CL) Silty CLAY, with sand; grey; clay fraction low plasticity; sand fraction fine to medium grain; trace brick, rootlets		FILL	> > *	W to <pl< td=""><td></td><td>D E</td><td>/</td><td>-1.15-</td><td>-PID-</td><td><1</td></pl<>		D E	/	-1.15-	-PID-	<1
	-	1.4 -	FILL/ (SP) Silty SAND; grey, dark grey; fine to medium; with fine to medium ash gravels, fine to medium sub-angular to sub-rounded gravels (crushed natural rock), trace organics and coal		FILL		M to W	-	D E	/	-1.3-	-PID-	<1
	_	16	FILL/ (SP) SAND; grey; fine to medium; trace fine to medium sub-angular to sub-rounded		FILL		w		D E	/	- 1.5	-PID-	<1
	-	1.7 -	\gravels (crushed natural rock) // FILL/ (ML) SILT; grey brown; low plasticity; trace \rootlets //		FILL	> > > >	W to <pl< td=""><td></td><td>D</td><td></td><td>-1.65-</td><td>-PID-</td><td><1</td></pl<>		D		-1.65-	-PID-	<1
	-	1.9 -	FILL/ (SP) SAND, with silt; grey; fine to medium; organics, fine to medium sub-angular to sub-rounded gravels (crushed natural rock)),		FILL		w	-	D E		-1.8-	-PID-	<1
	-0	2-	FILL/ (ML) Clayey SILT, trace gravel; grey; silt fraction low plasticity; gravel fraction fine to medium, sub-angular to sub-rounded; with		FILL		W to <pl< td=""><td>=</td><td>D E</td><td></td><td>-2.0-</td><td>-PID-</td><td><1</td></pl<>	=	D E		-2.0-	-PID-	<1
	-	-	2.0m: several bones up to 100mm length ¹ Borehole discontinued at 2.10m depth Limit of machine								- - - - -		
NOTE	S: ^(#) S	oil origi	n is "probable" unless otherwise stated. ⁽⁷⁾ Consistency/Relative density shad	ing is for vi	isual refer	ence only ·	- no correlat	ion between co	hesive	and gra	inular ma	aterials i	s implied.
PLA	NT: HO	5.5 D: 3	T Excavator with 300mm Auger 00mm solid flight auger to 2 1m		C	PERA	TOR: T	utt Bryant					LOGGED: Kramer

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using a differential GPS, typical accuracy ±0.1m.



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 3.5 AHD COORDINATE E:383964.9 N: 6355622.3 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

LOCATION ID: 303 PROJECT No: 213618.02 DATE: 04/10/22 SHEET: 1 of 1

CONDITIONS ENCOUNTERED SAMPLE **TESTING AND REMARKS** DENSITY." GROUNDWATER CONSIS. TEST TYPE Ē MOISTURE REMARKS DEPTH (m) INTERVAL GRAPHIC **ORIGIN**^(#) DEPTH (RESULTS DESCRIPTION түре RL (m) AND REMARKS OF STRATA No free groundwater observed 0.0 FILL/ (SP) Silty SAND; brown; fine to medium; $|\cdot|$ -PID-DE 0.05 <1 trace rootlets, bricks, fine to medium sub-angular to sub-rounded gravels (crushed natural rock) $\cdot |\cdot| \cdot |$ FILL Μ 0.3 -PID-<1 $\cdot |\cdot| \cdot |$ $\cdot | \cdot | \cdot$ -PID-<1 0.5 0.5FILL(?)/ (SP) SAND, with silt; grey; fine to ć ossibl FILL medium; trace rootlets M to W 0.65 FILL(?)/ (CH) CLAY; pale brown mottled grey; high plasticity; trace rootlets, fine rounded gravels (crushed natural rock) NA W to <PL ossibl FILL -PID-1.0 <1 1.1 (SP) SAND, with silt; grey; fine to medium; trace rootlets ALV Μ -PID-<1 12 1.3 (SP) SAND; pale grey; fine to medium ALV Μ -PID-<1 15 2 1.7 Borehole discontinued at 1.70m depth Limit of investigation 2 EXPORTED 20/04/23 12:28. TEMPLATE ID: DP_101.02.00_S0ILL0G NOTES: ⁽⁹⁾Soil origin is "probable" unless otherwise stated. ⁽¹⁾Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied. PLANT: 5.5T Excavator with 300mm Auger **OPERATOR:** Tutt Bryant LOGGED: Kramer CASING: METHOD: 300mm solid flight auger to 1.7m

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using a differential GPS, typical accuracy ±0.1m.



CLIENT: PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South

School Infrastructure NSW

TEST PIT LOG

SURFACE LEVEL: 2.4 AHD COORDINATE E:384044.3 N: 6355587.7 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 304 PROJECT No: 213618.02 DATE: 28/11/22 SHEET: 1 of 1



METHOD: 450mm bucket to 1.0m

EXPORTED 20/04/23 12:28. TEMPLATE ID: DP_101.02.00_S0ILL0G



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

 SURFACE LEVEL:
 2.3 AHD

 COORDINATE
 E:384023.0 N: 6355612.5

 DATUM/GRID:
 MGA94 Zone 56

LOCATION ID: 305 PROJECT No: 213618.02 DATE: 28/11/22 SHEET: 1 of 1

		CONDITIONS ENCOUNTERED		SAN	SAMPLE			TESTING AND REMARKS					
GROUNDWATER	RL (m) DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	
dwater observed at 2.8m	- 0.0 -	FILL/ (SP) Silty SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); trace rootlets, roots FILL/ (SP) Silty Gravelly SAND; dark grey brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); trace rootlets, fine to coarse slag		FILL		D		E	Z	~0.0 7 ~0.05~	-PID-	<1	
				FILL		М		D E		-0.3-	-PID-	<1	
n, free ground	- 0.65	gravels, ceramic, glass, ash FILL/ (SP) Silty SAND; brown; fine to medium; trace brick, glass, metal, ceramic 0.6m; fibro fragment observed (305F)		FILL		М		D		-0.6-	-PID-	<1	
Seepage at 1.2r	- - - - -	FILL/ SAND; fine to medium FILL/ SAND; fine to medium FILL/ SAND; intermixed pale brown grey; fine to medium; trace metal sheets / rods, trace glass, plastic FILL/ SILT; dark grey; low plasticity; with organics		FILL			D			PID	<1		
	1.2 - 										PID		
	- - - -				NA	vv		E				<1	
	1.8 - - - 2-			FILL		W >PL		D				<1	
	2.1 - -0 - - 	FILL/ (SP) Silty SAND; grey; fine to medium						D E		-2.5-	PID	<1	
				FILL		M to W		D		- 3.0 -	PID	<1	
	- 3.15	Test pit discontinued at 3.15m depth Limit of machine			1			<u> </u>		· · ·			
	: ^(#) Soil orig	in is "probable" unless otherwise stated. " ^C Consistency/Relative density shad T Excavator with 450mm bucket	ing is for vi	isual refer	ence only -	no correla	ition between o	cohesive	and gra	anular m	aterials i	s implied.	

METHOD: 450mm bucket to 3.15m

EXPORTED 20/04/23 12:28. TEMPLATE ID: DP_101.02.00_S0ILL0G



PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.3 AHD COORDINATE E:384000.5 N: 6355629.8 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 306 DATE: 28/11/22 SHEET: 1 of 1

								SAM			TESTING AND REMARKS		
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
	2	0.0	FILL/ (SP) Silty SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); trace rootlets, roots, ceramic		FILL		D		D E	X	-0.05 -0.05 -	-PID-	<1
	-	0.3 -	FILL/ (SP) Silty Gravelly SAND; dark grey brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); trace rootlets, fine to coarse slag gravels, ceramic, glass, ash		FILL		М		D		- 0.4 -	-PID-	<1
	-	-	SAND; pale brown; fine to medium				м		D E		-0.6-	-PID-	<1
	-	0.7 -	SAND, trace gravel; intermixed pale brown grey; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); with rusted metal, trace glass, sandstone cobbles				М		D E		- 1.0 -	-PID-	<1
		1.1 -	FILL/ Clayey SAND; dark grey; trace metal / gravels, fine to medium sub-angular to sub-rounded (crushed natural rock)		FILL	NA	м		D E		- 1.2-	-PID-	<1
vater observed	-	1.3 -	FILL/ Sandy Clayey GRAVEL; dark grey; gravel fraction fine to medium, sub-angular to sub-rounded, (crushed natural rock); sand fraction fine to medium		FILL		w		D E		- 1.4 -	-PID-	<1
Free ground	-		FILL/ Clayey SAND; grey; fine to medium; trace rootlets		FILL		w		D		- 1.6 -	-PID-	<1
- LL UG	-	2-							D E		-2.0-	-PID-	<1
00-00.20.10	-0	-	FILL/ (CL) Silty CLAY; dark grey; low plasticity; trace rootlets		FILL		W >PL		D E		-2.2-	-PID-	<1
	-	2.3 - - - - -	Test pit discontinued at 2.30m depth Pit collapse	<u>r 1/ 1/</u>	****	9					· · ·		
	 S: ^(#) S	Soil orig	in is "probable" unless otherwise stated. ^(*) Consistency/Relative density shac	ling is for vis	sual refer		ly - no correla	tion between	cohesive	e and gr	anular m	aterials i	

METHOD: 450mm bucket to 2.3m

CLIENT:

School Infrastructure NSW



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.5 AHD COORDINATE E:383985.5 N: 6355642.7 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 307 PROJECT No: 213618.02 DATE: 28/11/22 SHEET: 1 of 1

											SAMPLE			TESTING AND REMARKS	
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	
	-	0.0 - -	FILL/ (SP) Silty SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); trace rootlets, gravel	· · · · · ·	FILL			D		E	~	-0.05-	-PID-	<1	
	5-	0.25 - -	FILL/ Silty SAND, with gravel; grey brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded (crushed natural rock); trace glass, brick, terracotta, slag, ceramic, coal 0.3-0.5m: abundant fibro fragments (307F) present		FILL		-			D E D E		- 0.3- - 0.5-	PID	<1	
	-	- 0.7	FILL/ (SP) SAND; pale brown; fine to medium 0.8m: pale brown grey—		FILL		-	D to M		D		- 0.8 -	-PID-	<1	
	-	0.9 - 1 - -	FILL/ (SP) Clayey SAND, trace gravel; intermixed brown grey; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); trace ceramic, brick, glass, rusted metal		FILL					D		1.0	PID	- <1	
		1.4 -	FILL/ SAND, with silt; grey; fine to medium; trace organics	<	FILL	N	A			D		- 1.5 -	-PID-	- <1	
	-	1.6 - - -	FILL/ (CL) Silty CLAY; dark grey; low plasticity; trace rootlets		FILL		,	W >PL		D E		- 1.8	-PID-	· <1	
90.120 _ 00.20.	-	2.0-	FILL/ Clayey SAND; brown grey; fine to medium; with abundant organics 2.0-2.3m: strong decaying organic odor		FILL		-	w		D		- 2 -	-PID-	<1	
epage	-0	2.3 -	2.3 (SP) SAND; grey pale grey; fine to medium		ALV	× ,		w		D		- 2.5 -	PID	- <1	
(IEU 20/04/23 12:23.	-	- 2.7 - -	Test pit discontinued at 2.70m depth Limit of investigation												
	S: #9	Soil orig	in is "probable" unless otherwise stated. ^(*) Consistency/Relative density shad	ing is for v	visual refe		only -	no correlat	ion betweer	i cohesive	e and gr	anular m	aterials	is implied.	

METHOD: 450mm bucket to 2.7m



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 3.5 AHD COORDINATE E:383956.6 N: 6355601.0 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 308 DATE: 28/11/22 SHEET: 1 of 1

CONDITIONS ENCOUNTERED								SAMPLE				TESTING AND REMARKS		
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	
er observed	-	0.0	FILL/ (SP) Silty SAND; brown; fine to medium; trace rootlets	· · · · · · · · · · · · · · · · · · ·	FILL		D		E	7	-0.05	-PID-	<1	
Vo free groundwat	- (0.2 -	FILL/ (GP) Sandy GRAVEL; grey; fine to medium, sub-angular to sub-rounded, (crushed natural rock); with slag, coal, ash (SP) SAND; pale grey; fine to medium; trace				D		D		-0.25-	PID	<1	
	-ro -	-			ALV		М		D		- 0.5 -	PID	<1	
	-	0.7 -	Test pit discontinued at 0.70m depth Limit of investigation		<u>}</u>									
	-	- 1 -									- 1 -	-		
	-	-									n ,			
	-	-												
	-2	-												
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NOTES) 3: ^(#) S	oil origi	n is "probable" unless otherwise stated. $^{(!)}$ Consistency/Relative density sha	ading is for vi	sual refer	ence only -	no correla	ation between	cohesive	e and gr	anular m	aterials i	s implied.	
PLAI	NT:	5.5	T Excavator with 450mm bucket		C	PFRAT		Futt Brvar	nt				OGGED: Kramer/Krebs	

EXPORTED 20/04/23 12:30. TEMPLATE ID: DP_101.02.00_SOILLOG


CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 3.4 AHD COORDINATE E:383971.8 N: 6355591.2 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 309 DATE: 28/11/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
er observed		0.0	FILL/ (SP) Silty SAND; brown; fine to medium; trace rootlets		FILL		D		E	X	~0.0 ~ -0.05-	-PID-	<1
e groundwat		0.2 -	FILL/ (GP) Sandy GRAVEL; grey; fine to medium, sub-angular to sub-rounded, (crushed natural rock); with slag, coal, ash		FILL	NA	D		D		-0.25-	-PID-	<1
No fre	-ო	-	(SP) SAND; pale grey; fine to medium; trace rootlets		ALV		М		D E		- 0.5 -	-PID-	<1
		0.7 -	Test pit discontinued at 0.70m depth Limit of investigation										
-	-	1-									- 1 -		
-		-											
		-											
	-0	_											
		-											
		-											
-	-	2-									- 2 -		
		-											
-		-											
-		_											
		-											
		-											
NOTES	: ^(#) S	oil origi	n is "probable" unless otherwise stated. $^{\prime\prime} \mbox{Consistency}/\mbox{Relative density sha}$	ding is for vis	sual refer	ence only -	no correla	ation between o	cohesive	e and gr	anular ma	aterials i	is implied.
PLAN	IT:	5.5	T Excavator with 450mm bucket		Ō	PERAT	OR: 1	utt Brvan	t				LOGGED: Kramer/Krebs

EXPORTED 20/04/23 12:30. TEMPLATE ID: DP_101.02.00_SOILLOG



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

 SURFACE LEVEL:
 3.3 AHD

 COORDINATE
 E:383991.5 N: 6355574.4

 DATUM/GRID:
 MGA94 Zone 56

LOCATION ID: 310 PROJECT No: 213618.02 DATE: 28/11/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAM	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
er observed	-	0.0	FILL/ (SP) Silty SAND; brown; fine to medium; trace rootlets	· · · · ·	FILL		D		D E	R	-0.05 -0.05	-PID-	<1
No free groundwate	00 -	0.2 - - -	(SP) SAND; pale grey; fine to medium; trace rootlets		ALV	NA	м		D E		-0.25-	-PID-	<1
	-	-							DE	/-	-0.5-	-PID-	<1
	-	0.6 -	Test pit discontinued at 0.60m depth Limit of investigation										
	-	1-									- 1 -		
	5-	-											
	-	-											
0011100_00.701.101_10	-	2-									- 2 -		
EU 20/04/23 12:31. IEMPLAIE 10	-	-											
	- 	Soil orig	in is "nrnhahle" unless otherwise stated ⁽¹ /Consistency/Relative density sh	ading is for vie	ual refer	ence only -	no correla	tion between	cohesive	and or	anular m	aterials i	is implied
PLA	NT	: 5.5	T Excavator with 450mm bucket		C	PERAT	OR: 1	Tutt Bryar	nt	- and gr			LOGGED: Kramer/Krebs

METHOD: 450mm bucket to 0.6m



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

TEST PIT LOG

SURFACE LEVEL: 3.1 AHD COORDINATE E:384002.8 N: 6355589.4 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 311 PROJECT No: 213618.02 DATE: 28/11/22 SHEET: 1 of 1



METHOD: 450mm bucket to 0.8m

EXPORTED 20/04/23 12:31. TEMPLATE ID: DP_101.02.00_S0ILL0G



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South SURFACE LEVEL: 3.8 AHD COORDINATE E:383891.4 N: 6355717.2 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 312 DATE: 29/11/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
e groundwater observed	-	0.0 0.05 0.35	FILL/ (SP) Silty SAND; brown; fine to medium; trace rootlets FILL/ (SP) SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded (crushed natural rock); trace brick, terracotta, rootlets, concrete, plastic		FILL		D		D E D E		-0.25-	-PID-	<1
No fre	- - - -	-	(SP) SAND; pale brown; fine to medium		ALV	NA	М		D		- 0.5	-PID-	<1
		2-	Test pit discontinued at 1.10m depth Limit of investigation						Ē				
	NT	: 5.5	T Excavator with 450mm bucket	ing is for vi	suai reter	PERAT	OR:	Futt Bryan	t	and gra	anular ma	aterials i	s Implied.

METHOD: 450mm bucket to 1.1m

EXPORTED 20/04/23 12:32. TEMPLATE ID: DP_101.02.00_SOILLOG



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.9 AHD COORDINATE E:383843.2 N: 6355654.9 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 313 PROJECT No: 213618.02 DATE: 29/11/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED			-		SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
er observed	-	0.0 0.1 -	FILL/ (SP) SAND, with silt, with gravel; brown; sand fraction fine to medium; gravel fraction fine sub-angular to sub-rounded (crushed natural rock): trace plastic, roots		FILL		D		D E	Z	-0.0 - -0.05-	-PID-	<1
e groundwat	-	- 0.3 -	FILL/ (SP) SAND, trace gravel; pale brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock); cobles		FILL		D		D E		-0.25-	-PID-	<1
No fre	-	-	FILL/ (SP) Gravelly SAND; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded (crushed natural rock); trace rootlets, (possible pavement gravels)	0 : 0 0	FILL		М		D E		-0.5-	PID	<1
	-	0.0 -	FILL/ (SP) Gravelly SAND; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded (crushed natural rock); trace brick, concrete, terracotta, cobbles, plastic, slate, metal	0000						-			
	-4	1-		0000	FILL	NA	D		D E		- 1.0	PID	<1
	-	1.3 -		.O · ·									
	- 	1.37	FILL/ (SP) Gravelly SAND; pale brown; sand fraction fine to medium; gravel fraction fine to medium sub- angular to sub-rounded; (pavement gravels)	0 0 0 0	FILL		М		D E D E		-1.35-	-PID-	<1 <1
		1.65 1.7 -	ASPHALTIC CONCRETE;						D		-1.67-	-PID-	<1
	-	-	(SP) SAND; pale brown; fine to medium							-			
	-m -	2-			ALV		М		D E		-2.0-	-PID-	<1
		2.2 -	Test pit discontinued at 2.20m depth Limit of investigation	<u>[</u>]							-		
	-	-											
	-	-								-			
		-											
	-	-								-			
	-	-											
	-0												
NOTES	: "s	ioil orig	in is "probable" unless otherwise stated. ^{[7} Consistency/Relative density shad T Excavator with 450mm bucket	ing is for vi	sual refer C	ence only -	no correla	ation between o	cohesive t	and gra	anular ma	aterials i	s implied.

METHOD: 450mm bucket to 2.2m

EXPORTED 20/04/23 12:32. TEMPLATE ID: DP_101.02.00_SOILLOG



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

 SURFACE LEVEL:
 3.9 AHD

 COORDINATE
 E:383857.4 N: 6355649.0

 DATUM/GRID:
 MGA94 Zone 56

LOCATION ID: 314 PROJECT No: 213618.02 DATE: 29/11/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED			-		SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
erved		0.0	FILL/ (SP) SAND, with silt; brown; fine to medium: trace rootlets		FILL		D		D E	\prec	-0.05	-PID-	<1
ndwater obse	-	0.1 - 0.2 - 0.25 -	FILL/ (SP) Gravelly SAND, with silt; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed		FILL				D E D		-0.15-	-PID-	<1 <1
e grou	-		natural rock); trace rootlets, brick ASPHALTIC CONCRETE;	0		$\left\{ \right.$							
No free	-	0.5 -	FILL/ (SP) Gravelly SAND; pale brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded; (pavement bravels)		FILL		M		D E		-0.4-	-PID-	<1
	-	0.7	FILL/ (SP) Gravelly SAND; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural	0	FILL	NA	Μ		E		-0.55-	-PID-	<1
			(SP) SAND; pale brown; fine to medium										
	-ო	-			ALV		М						
	-	1-							D		- 1.0 -	-PID-	<1
	-	-	Test pit discontinued at 1.10m depth Limit of investigation										
	-	-											
	-	2-									- 2 -		
	-	-											
	-	-											
	-	-											
	-	-											
	-	-											
	L	-											
	ŀ	-											
	S: #S	oil orig	n is "probable" unless otherwise stated. ⁽¹ Consistency/Relative density shac	ling is for vi	isual refer			ation between o	cohesive	and gr	anular m	aterials i	s implied.

METHOD: 450mm bucket to 1.1m

EXPORTED 20/04/23 12:33. TEMPLATE ID: DP_101.02.00_SOILLOG



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.9 AHD COORDINATE E:383836.2 N: 6355627.1 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 315 PROJECT No: 213618.02 DATE: 29/11/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAM	IPLE				TESTING AND REMARKS
erved GROUNDWATER	RL (m)	0.0 0.0	DESCRIPTION OF STRATA FILL/ Silty SAND; brown; fine to medium	GRAPHIC	(#) NIGIN Fill		MOISTURE	REMARKS	m d TYPE		DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
No free groundwater obs	-		FILL/ (SP) SAND, trace gravel; pale brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded (crushed natural rock); trace sandstone cobbles / boulder		FILL				 		-0.25	-PID- -PID-	<1
	- 4	0.6 -	FILL/ (SP) SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded (crushed natural rock); trace metal, plastic, brick, asphalt, ceramic, terracotta		FILL				D	-	- 1.0-	-PID-	<1
	-	1.15 - 1.25 - -	FILL/ Gravelly SAND; pale red; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed natural rock) FILL/ SAND, with gravel; dark brown; sand fraction fine to medium; gravel fraction fine to medium (crushed natural rock); trace concrete	0	FILL	NA	D to M		 		- 1.2	-PID-	<1
		- - - 2- - - - - - - - - - - - - - - -	SAND; grey pale grey; fine to medium 2.1m: brown, moist—				М		 		- 2.0	-PID-	<1
NOTES			Test pit discontinued at 2.40m depth Limit of investigation	ing is for v	isual refer	ence only -	- no correla	tion between c	cohesive	and gra	nular ma	aterials i	s implied.
PLA	NT:	5.5	T Excavator with 450mm bucket		C	PERA	TOR: 1	utt Brvan	t			1	LOGGED: Kramer/Krebs

METHOD: 450mm bucket to 2.4m

EXPORTED 20/04/23 12:33. TEMPLATE ID: DP_101.02.00_SOILLOG



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 Parkway Avenue, Hamilton South

SURFACE LEVEL: 5.1 AHD **COORDINATE E:**383848.7 N: 6355618.3 **DATUM/GRID:** MGA94 Zone 56 LOCATION ID: 316 PROJECT No: 213618.02 DATE: 29/11/22 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED			-		SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	ΤΕST ΤΥΡΕ	RESULTS AND REMARKS
erved		0.0	FILL/ Silty SAND; brown; fine to medium	$ \cdot \cdot \cdot $	FILL		D		D E	\prec	-0.0 - -0.05-	-PID-	<1
No free groundwater obse	2-	0.1 -	FILL/ (SP) SAND, trace gravel; pale brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded (crushed natural rock); sandstone cobbles / boulder		FILL						-0.25-	-PID- -PID-	<1
	_	- 0.75 - - 1 -	FILL/ (SP) SAND, with silt; dark brown; fine to medium; trace concrete, brick						D		- 1.0	-PID-	<1
	-4	- - 1.35			FILL	NA	D		E		· · ·		
	-	- 1.45	FILL/ Gravelly SAND; pale red; sand fraction fine to medium; gravel fraction fine to medium	0	FILL		D to M		D E	/	- 1.4 -	-PID-	<1
	-		sub-angular to sub-rounded (crushed natural rock) FILL/ (SP) SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded (crushed \natural rock)		FILL		D to M		<u>D</u>		- 1.5 -	-PID-	<1
	-		(SP) SAND; pale grey; fine to medium		ALV		М		D		- 2.0 -	-PID-	<1
	-ო	-											
	-	2.2 - - -	Test pit discontinued at 2.20m depth Limit of investigation		<u> </u>	<u> </u>					· · ·		
NOTES	- 	- - Soil orig	in is "probable" unless otherwise stated. ^{*/} Consistency/Relative density shad	ing is for vi	isual refer	ence only	no correla	ation between (cohesive	and gra	anular ma	aterials i	s implied.
гLAI	NT:	: 5.5	I Excavator with 450mm bucket		C	PERA	IUR: 1	i utt Bryan	τ				LOGGED: Kramer/Krebs

METHOD: 450mm bucket to 2.2m

EXPORTED 20/04/23 12:34. TEMPLATE ID: DP_101.02.00_SOILLOG



SURFACE LEVEL: 2.1 AHD COORDINATE E:384063.5 N: 6355616.0 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 401 PROJECT No: 213618.02 DATE: 09/03/23 SHEET: 1 of 2

				CONDITIONS ENCOUNTERED					SAMPLE		TESTING AND REMARKS
GROUNDWATER	2 RL (m)	0.	ODEPTH (m)	DESCRIPTION OF STRATA FILL/ Silty SAND, trace gravel; dark brown; sand fraction fine to medium; gravel fraction fine to	GRAPHIC	ORIGIN ^{#)}		MOISTURE	REMARKS SSE INTERVAL	0.0	1 TEST TYPE RESULTS AND REMARKS REMARKS REMARKS REMARKS REMARKS REMARKS REMARKS REMARKS REMARKS
	-	0	-	coarse angular to sub-angular; trace rootlets, trace hard plastic		FILL	NA	Μ	PFAS ASS D E	0.5	PID- <1
		0.	1	FILL/ SAND, with silt, trace gravel; brown; gravel fraction fine to coarse angular to sub-angular; trace ceramic shards, trace glass shards		FILL	NA	М	PFAS D D PFAS ASS	1.0 + I	PID-1 000000000000000000000000000000000000
ed at 1.75m	0		2					w	D PFAS ASS D F	- 2.0 I	PID- <1
ndwater observ	-	2.	8	Sandy SILT, trace gravel; dark brown; sand fraction fine; gravel fraction fine sub-angular to sub-rounded		ALV	s	>PL	PFAS ASS D PFAS	2.4	PID- 3
Free grou		:	3	fraction fine; gravel fraction fine sub-rounded; trace shell		-			ASS D PFAS ASS	3.0 +1	PID- <1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
			4-						D E PFAS ASS D	4.0	PID- <1
	-		-			-			PFAS	-	
		1	5						ASS D PFAS	5.0	PID- <1 (2000) PID- <1 (2000) (2000) Class 18 PVC
	- +-		6			ALV	NA	W	ASS D PFAS	6.0	PID- <1
			7 -			-			ASS D E PFAS	-7.0	PID- <1
	- 9	1	8			•			ASS D E PFAS	- 8.0	PID- <1
		1	9			- - - -				- 9.0 - F	PID- <1
						-					
NOTE	 S: ^(#) S	ioil o	origi	n is "probable" unless otherwise stated. $^{\circ \circ}$ Consistency/Relative density shad	ing is for v	isual refer	ence only -	no correl	ation between cohesive and grar] nular mate	erials is implied.
PLA MET	NT: HO	D:	Geo S	oprobe 6712DT folid flight auger to 13.0m, then hollow flight auger to use the second	to	C C	PERAT	OR : 3	Stratacore e		LOGGED: Helbig

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using differential GPS, typical accuracy ±0.1m.



CLIENT:

School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

Douglas Partners Geotechnics | Environment | Groundwater

CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South SURFACE LEVEL: 2.1 AHD COORDINATE E:384063.5 N: 6355616.0 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 401 PROJECT No: 213618.02 DATE: 09/03/23 SHEET: 2 of 2



13.0m for the well installation



SURFACE LEVEL: 2.2 AHD COORDINATE E:384012.3 N: 6355647.0 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 402 DATE: 13/03/23 SHEET: 1 of 2

			CONDITIONS ENCOUNTERED	-				SAMPLE			TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS TYPE INTERVAL	DEPTH (m)	ΤΕST ΤΥΡΕ	RESULTS AND REMARKS BACKFILL WELL PIPE
	-2	-0.0	FILL/ Silty SAND, trace gravel; dark brown; sand fraction fine to medium; gravel fraction fine to medium angular to sub-angular; trace coal reject, trace ceramic shards, trace glass shards	· · · · · · · · ·	FILL	NA	М		-0.0 -0.1 -	-PID-	Had Kill Concrete Backfill Concrete Ses 18 PUC
	_	0.4	FILL/ Clayey SAND, with gravel; brown orange brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded; trace fine slag gravels, trace ceramic shards		FILL	NA	М	D E PFAS	- 0.5 -	-PID-	ntonitie mm Diameter Cts
	-	1- 1-	FILL/ Silty SAND, with clay, with gravel; dark brown; gravel fraction fine to coarse angular, sub-angular to sub-rounded; trace coal reject, trace ceramic shards, trace glass shards	· · · · · · · · ·				D E PFAS	- 1.0 -	-PID-	
ed at 1.6m 🛓	-		1.5-2.1m: trace metal wire and fragments—		FILL	NA	M to W	D E PFAS	- 1.5 -	-PID-	
oundwater observe	-	2- 2.1	FILL(?)/ Sandy SILT, trace gravel; dark brown;					D	-2.0-	-PID-	<1 0 2 0 0 0 0 0 0
Free gr	-		silt fraction low plasticity; sand fraction fine; gravel fraction fine to medium sub-angular to sub-rounded		FILL possibly ALV	y S	>PL	D E PFAS	- 2.5 -	-PID-	
		3-	SAND, with silt, trace gravel; grey brown; sand fraction fine to medium; gravel fraction fine to medium rounded to sub-rounded; trace shell					D	- 3.0 -	-PID-	2/5mm Gravel/Sand C
	-				• • • • •			D E	- 3.5 -	-PID-	
	?	4-			ALV	NA	w	D E PFAS	- 4.0 -	-PID-	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
	-	-							-4.5-	-PID-	3 3 0 0 0 0 0 0 0 0 0 0 0
NOTE:	S: (#)S	ioil orig	jin is "probable" unless otherwise stated. ^(*) Consistency/Relative density shad	ling is for v	isual refer		- no correla	tion between cohesive and g	anular ma	aterials	is implied.
MET 8 0m	HO for	D:	Solid flight auger to 8.0m, then hollow flight auger to 1.0 flight	D	Ċ	CASING	B: None)			g

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using differential GPS, typical accuracy ±0.1m.



CLIENT:

School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.2 AHD COORDINATE E:384012.3 N: 6355647.0 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 402 PROJECT No: 213618.02 DATE: 13/03/23 SHEET: 2 of 2



8.0m for well installation



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.8 AHD COORDINATE E:383945.0 N: 6355704.0 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 403 DATE: 13/03/23 SHEET: 1 of 2

			CONDITIONS ENCOUNTERED		1			SAMPLE			TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS TYPE INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS BACKFILL WIGLI DIDE
	-	-0.0 - - 0.3 -	TOPSOIL/FILL(?)/ SAND, with silt, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded		TOP possibly FILL	/ NA	М	D E PFAS	- 0.1	-PID-	
	2	-	FILL/ Silty SAND, trace gravel; dark brown; sand fraction fine to medium; gravel fraction fine to coarse angular to sub-angular; trace asphalt, brick pieces, glass shards, coal reject, ash and slag gravels	· · · · · · · · ·				D E PFAS	0.5-	-PID-	
	-	- 1- -		· · · · · ·				D E PFAS	- 1.0 -	-PID-	Sentonite
	-	-		· · · · · · · · ·	FILL	NA	M to W	D E PFAS	- 1.5 -	-PID-	<1 °0 0 0 0 0 0 0
oserved at 2.0m 🛛	-	2-	2.1m: metal sheeting present	 					2.0-	PID-	
groundwater of	-	- - 2.6 -	Sandy SILT trace gravel: dark brown: cand					D E PFAS	- 2.5 -	-PID-	- <1 0 0 0 0
Free	-0	3-	fraction fine; gravel fraction fine rounded to sub-rounded		possibly ALV	s	>PL	D	- 3.0 -	- - - - - - - -	Gravel/Sand O O O O
	-	3.4 - -	SAND, with silt; brown; fine to medium		•			D E PFAS	- 3.5 -	PID	
	- 	4 -			ALV	NA	W	D	4.0-	- - - - - - -	
	-2	4.8	SAND; brown yellow; fine to medium		ALV	NA	w		-	-	
	S: #5		in is "probable" unless otherwise stated. ^(*) Consistency/Relative density shad	ding is for v	risual refer		- no correla	ation between cohesive and g	ranular m	aterials	is implied.

7.5m for well installation



BOREHOLE LOG

SURFACE LEVEL: 2.8 AHD COORDINATE E:383945.0 N: 6355704.0 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 403 PROJECT No: 213618.02 DATE: 13/03/23 SHEET: 2 of 2



7.5m for well installation



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 Parkway Avenue, Hamilton South

SURFACE LEVEL: 3.9 AHD COORDINATE E:383944.2 N: 6355579.0 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 404 PROJECT No: 213618.02 DATE: 13/03/23 SHEET: 1 of 3





BOREHOLE LOG

SURFACE LEVEL: 3.9 AHD COORDINATE E:383944.2 N: 6355579.0 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 404 PROJECT No: 213618.02 DATE: 13/03/23 SHEET: 2 of 3





BOREHOLE LOG

SURFACE LEVEL: 3.9 AHD COORDINATE E:383944.2 N: 6355579.0 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 404 PROJECT No: 213618.02 DATE: 13/03/23 SHEET: 3 of 3



7.5m for well installation



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.0 AHD COORDINATE E:383885.8 N: 6355638.0 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 405 PROJECT No: 213618.02 DATE: 10/03/23 SHEET: 1 of 2

			CONDITIONS ENCOUNTERED	1	I			SAMPLE		-		TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS BACKFILL WELL PIPE
		0.0	CONCRETE; brown	Q Q		NA	М					
			FILL/ Silty SAND; dark brown; fine to medium; trace rootlets		FILL	NA	М	ASS		-0.2-	-PID-	
	-	0.3 -	FILL/ Silty SAND, trace gravel; pale brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded; trace brick, trace tiles		FILL	NA	М	E PFAS ASS D E		- 0.5 -	-PID-	C C Bentonii Iameter Class 1
	-	0.75		<u> </u>		>		PFAS				
		1-	FILL/ SAND, trace sitt; pale grey		FILL	NA	М	ASS D E PFAS	1	- 0.9-	PID-	- <1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
	-	1.3 -	SAND; pale grey; fine to medium			>					-	
	_	-					М	ASS D E PFAS	1	- 1.5 -	-PID-	
<u>v</u>	-	- 2-								- 2 -	-	
bserved at 2.0n	-										-	
e groundwater c	-	-						ASS D E PFAS	7	-2.5-	-PID-	e <u>l</u> (Sand ₅ ° 0 hine Sotted∰
E.		3-								- 3 -	-	∆275mm_6rav
	-				ALV	NA				- ·	-	0 0 0 0 0 0 0 0 0 0 0 0 0 0
	-	-					W	ASS D E PFAS	1	- 3.5 -	-PID-	- <1
	-0	- - 4 -						ASS		-4.0-	-PID-	
	_				•			D E PFAS			-	
	-	-									-PID-	<1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
	-									- ·	-	
NOTE:	s: ‴s NT:	ioil orig	in is "probable" unless otherwise stated. ⁽⁷ Consistency/Relative density shac	ling is for vi	isual refer	ence only -	no correla	ation between cohesiv Stratacore	e and gr	anular m	aterials	is implied.

METHOD: Concrete core to 0.115m, hand auger to 1.0m, push tube to 5.0m, solid flight auger to 9.0m, then hollow flight auger to 9.0m for well installation

CASING: None



BOREHOLE LOG

SURFACE LEVEL: 4.0 AHD COORDINATE E:383885.8 N: 6355638.0 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

LOCATION ID: 405 PROJECT No: 213618.02 DATE: 10/03/23 SHEET: 2 of 2



METHOD: Concrete core to 0.115m, hand auger to 1.0m, push tube to 5.0m, solid flight auger to 9.0m, then hollow flight auger to 9.0m for well installation

CASING: None



BOREHOLE LOG

SURFACE LEVEL: 4.0 AHD COORDINATE E:383777.0 N: 6355597.0 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

LOCATION ID: 406 PROJECT No: 213618.02 DATE: 10/03/23 SHEET: 1 of 3



METHOD: Concrete core to 0.05m, hand auger to 0.9, solid flight auger to 8.0m, then hollow flight auger to 8.0m for well installation

CASING:



BOREHOLE LOG

SURFACE LEVEL: 4.0 AHD COORDINATE E:383777.0 N: 6355597.0 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

LOCATION ID: 406 PROJECT No: 213618.02 DATE: 10/03/23 SHEET: 2 of 3



auger to 8.0m, then hollow flight auger to 8.0m for well installation

CASING:



BOREHOLE LOG

SURFACE LEVEL: 4.0 AHD COORDINATE E:383777.0 N: 6355597.0 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

LOCATION ID: 406 PROJECT No: 213618.02 DATE: 10/03/23 SHEET: 3 of 3



METHOD: Concrete core to 0.05m, hand auger to 0.9, solid flight auger to 8.0m, then hollow flight auger to 8.0m for well installation

±0.1m.

CASING:



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.3 AHD COORDINATE E:383998.4 N: 6355647.8 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 501 DATE: 18/03/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED	1				SAN	IPLE				TESTING AND REMARKS
GROUNDWATER		0.0 0.0	DESCRIPTION OF STRATA FILL/ (SP) Silty SAND; brown; fine to medium; trace rootlets, ceramic				■ MOISTURE	REMARKS	D D PFAS	INTERVAL	DEPTH (m)		RESULTS AND REMARKS
	-0	0.2	FILL/ (SP) Silty SAND; dark grey brown; fine to medium; trace brick, ceramic, glass, slag, ash, metal wire		FILL	NA	м		D E PFAS B	1	- 0.2 -0.25-	-PID-	- <1
	-	0.45	FILL/ (SP) SAND; intermixed brown orange grey; fine to medium; trace ceramic, steel, ash		FILL	NA	М		D PFAS		-0.45- -0.5-	-PID-	<1
er observed at 1.55m A	-	1.05	FILL/ (SP) Clayey SAND, with silt; intermixed grey brown; fine to medium; trace glass, ceramic, metal, slag 1.5m: from 1.5m, wet—		FILL	NA	М		D E PFAS]_< 	-1.05- -1.09-	-PID-	- <1
e//44/25 11:50. IEMPLAIE 1U: UP_101.02_001LLUU Seepage at 1.5m, free groundwater		1.9 · 2 - · ·	Test pit discontinued at 1.90m depth Pit collapse								- 2 -		
		Soil orig	in is "probable" unless otherwise stated. ^{Tr} Consistency/Relative density shad	ing is for vi	isual refer	ence only -	no correla	ation between	cohesive	and gr	anular ma	aterials	is implied.

METHOD: 300mm bucket to 1.9m



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.2 AHD COORDINATE E:384018.4 N: 6355632.0 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 502 DATE: 18/03/23 SHEET: 1 of 1

CONDITIONS	ENCOUNTERED	SAMPLE	TESTING AND REMARKS			
E E 000 0 E 0	fine to medium;	RI ORIGIN ^(®) PA CONSIS. ^(™) MOISTURE	REMARKS Series INTERVAL	(m) HL430 C0.0 ³ −0.05 PID- <1	ESULTS AND EMARKS	
-∾ 0.2 FILL/ (SP) Silty SAND; brown; trace bricks, slag, ceramic, gla	ine to medium; i i i i ss i i i i · i i i i i i · i i i i i i · i i i i i i · i i i i i i · i i i i i i · i i i i i i · i i i i i	FILL NA M	D PFAS B D PFAS	-0.25-PID- <1		
0.75 FILL/ (SP) Silty SAND; dark gr medium; trace brick, ceramic, y metal wire FILL/ (SP) SAND; intermixed b brown; fine to medium; trace a plastic, glass	ey brown; fine to glass, slag, ash, rown grey dark sh, slag, metal,	FILL NA M	B D E PFAS	0.6 0.7		
- 1-		FILL NA M	D E PFAS B	- 1.0 - PID- <1		
FILL/ (SP) Clayey SAND, with grey; sand fraction fine to med fraction fine to medium sub-an sub-rounded 1.4rr	silt, trace gravel; ium; gravel gular to 1: from 1.4m, wet depth	FILL NA M	B D E PFAS	- 1.5 PID- <1		
Pit collapse			-	- 2 -		
			-			
			-	· · ·		
NOTES: ^(#) Soil origin is "probable" unless otherwise stated. ^(*) Consi	istency/Relative density shading is for visua	al reference only - no corre	lation between cohesive and grar	anular materials is implied.	romor	

METHOD: 500mm bucket to 1.6m



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.1 AHD COORDINATE E:384048.8 N: 6355605.4 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 503 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1



METHOD: 500mm bucket to 1.7m



TEST PIT LOG

SURFACE LEVEL: 2.4 AHD COORDINATE E:384039.4 N: 6355593.0 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 504 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1



METHOD: 500mm bucket to 1.1m

EXPORTED 20/04/23 11:36. TEMPLATE ID: DP_101.02.00_S0ILL0G



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 2.3 AHD COORDINATE E:384007.5 N: 6355616.0 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 505 DATE: 18/03/23 SHEET: 1 of 1

		CONDITIONS ENCOUNTERED SAMPLE										TESTING AND REMARKS	
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
	-	0.0	FILL/ (SP) SAND, with silt; brown; fine to medium; trace ceramic		FILL	NA	м		D E PFAS B	1	-0.05	-PID-	<1
	-2	0.20	FILL/ (SP) Silty SAND; dark grey, grey; fine to medium; trace slag, ceramic, brick, metal wire		FILL	NA	М		B D E PFAS	1	-0.4-	-PID-	<1
	-	- 0.5	FILL/ (SP) SAND, with silt, trace gravel; intermixed grey pale grey brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded; trace brick, metal, ceramic, glass, terracotta, shells						D E PFAS	1	-0.5-	-PID-	<1
	-	- 1- -							B D E PFAS	1	- 1.0	-PID-	<1
	-	-			FILL	NA	М		D E PFAS	1	- 1.5	-PID-	<1
1	-								B D PFAS		- 2.0	-PID-	<1
/∠3 11:3/. וכוורואוב 12. ער_1			(SP) SAND; pale grey; fine to medium; trace rootlets		ALV	NA	w		D E PFAS	1	-2.4-	-PID-	<1
APUKIEU 20/04/		2.8 -	Test pit discontinued at 2.80m depth Pit collapse		<u>}</u>								
	S: (#)	Soil orig	in is "probable" unless otherwise stated. ^(*) Consistency/Relative density sha	ding is for vi	isual refer			ation between o	cohesive	and gra	anular ma	aterials i	is implied.

METHOD: 500mm bucket to 2.8m



BOREHOLE LOG

SURFACE LEVEL: 2.6 AHD COORDINATE E:384008.5 N: 6355601.3 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/--- LOCATION ID: 506 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1



REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using differential GPS, typical accuracy ±0.1m.



EXPORTED 20/04/23 11:37. TEMPLATE ID: DP_101.02.00_S0ILL0G

CLIENT:School Infrastructure NSWPROJECT:Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 3.4 AHD COORDINATE E:383983.4 N: 6355584.7 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 507 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

	CONDITIONS ENCOUNTERED 5							SAMPLE				TESTING AND REMARKS		
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	
idwater observed	-	0.0	FILL/ (SP) Silty SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded; (crushed natural rock), trace rootlets		FILL	NA	D		D E PFAS B	1	-0.05	-PID-	<1	
o free grour	- ~	0.35	FILL/ (GP) GRAVEL, with sand; grey; gravel fraction fine to medium; sand fraction fine to medium; slag gravels		FILL	NA	D		D E PFAS	7t	- 0.3 - -0.35-	-PID-	<1	
Ż	-	-	(SP) SAND; pale grey; fine to medium; trace rootlets		ALV	NA	D		D E PFAS	1	- 0.5 -	PID	<1	
	-	0.8 -	Test pit discontinued at 0.80m depth Limit of investigation	<u> </u>										
	-	1-									- 1 -			
	-	-												
	-0	-												
	-	-												
	-	_												
	_	-												
01LL06	-	2-									- 2 -			
91.02.00_SC	-	-												
1D: DP_10		-												
. TEMPLATE	-	-												
4/23 11:37	-	-												
0RTED 20/0	-	-												
	S: (#)	Soil orig	in is "probable" unless otherwise stated. $^{(7)}$ Consistency/Relative density sha	ding is for vi	sual refe	rence only -	no correla	tion between	cohesive	and gra	anular m	aterials	is implied.	
PLA	NT	: 5T	Excavator with 500mm Bucket		C	OPERAT	TOR: F	oster					LOGGED: Kramer	

METHOD: 500mm bucket to 0.8m



School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

CLIENT:

SURFACE LEVEL: 3.5 AHD COORDINATE E:383978.9 N: 6355605.5 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 508 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

	CONDITIONS ENCOUNTERED											TESTING AND REMARKS		
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	
dwater observed	-	0.0	FILL/ (SP) Silty SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded; (crushed natural rock), trace rootlets		FILL	NA	D		D E PFAS	1	-0.05-	PID	<1	
free groun	-	0.4	FILL/ (GP) GRAVEL, with sand; grey; fine to medium; slag gravels		FILL	NA	D		D E PFAS	1	-0.3-	-PID-	<1	
No	-ro	0.4	FILL/ (SP) SAND, with silt; brown; fine to medium		FILL	NA	М		D E PFAS	1	- 0.5 -	PID	<1	
	-	0.7	(SP) SAND; pale grey; fine to medium; trace rootlets		ALV	NA	М							
	-	-	Test pit discontinued at 1.00m depth Limit of investigation						D E PFAS	1	- 1.0 -			
		-												
	-	- - 2 -									- 2 -			
	-	-												
	-	-												
NOTES	: ^(#) S	- oil orig	in is "probable" unless otherwise stated. ^(*) Consistency/Relative densitv sha	ding is for vi	isual refer	ence only -	no correla	ation between	cohesive	and gr	anular m	aterials i	s implied.	
PLA	NT:	5T	Excavator with 500mm Bucket		C	PERA1	OR: F	Foster		J.			LOGGED: Kramer	

METHOD: 500mm bucket to 0.4m, shovel to 1.0m

EXPORTED 20/04/23 11:37. TEMPLATE ID: DP_101.02.00_SOILLOG

REMARKS: Service hit at 0.4m (redundant network cable). Location coordinates are in MGA94 Zone 56. Coordinates obtained using differential GPS, typical accuracy ±0.1m.



SURFACE LEVEL: 3.5 AHD COORDINATE E:383963.3 N: 6355633.8 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 509 PROJECT No: 213618.02 DATE: 19/03/23 SHEET: 1 of 1



METHOD: 100mm hand auger to 1.7m

CLIENT:

School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South



SURFACE LEVEL: 3.5 AHD COORDINATE E:383947.5 N: 6355604.6 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 510 DATE: 18/03/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED	1				SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
ater observed	-	0.0	FILL/ (SP) Silty SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded; (crushed natural rock), trace rootlets		FILL	NA	D		D E PFAS B	1	-0.05	-PID-	<1
groundv		0.3 -	FILL/ (GP) GRAVEL, with sand; grey; fine to medium; slag gravels	0.	FILL	NA	D		D E PEAS	<u>/</u> t	-0.25- -0.3-	-PID-	<1
No free g		-	(SP) SAND; pale grey; fine to medium; trace rootlets		ALV	NA	D		D	<u>_</u>	- 0.5	PID	<1
	-	- 0.8	Test pit discontinued at 0.80m depth Limit of investigation	-l. *									
		1-									- 1 -		
	-	-											
	-	-											
	-	-											
	5	-											
	-	-											
		2-									- 2 -		
00-1107_00_00.101_40	-	-											
	-	-											
8. IEMPL		-											
/23 11:3	-	-											
10 20/04 [,]	-	-											
EXPORTE		-											
PLA	.s: "'s	5011 orig	In Is "probable" unless otherwise stated. 'Consistency/Relative density share Excavator with 500mm Bucket	ing is for vi	isual refer	ence only -	no correla	oster	ohesive	and gr	anular ma	aterials	IS IMPlied.

METHOD: 500mm bucket to 0.8m

CLIENT:

School Infrastructure NSW

PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South



TEST PIT LOG

SURFACE LEVEL: 3.8 AHD COORDINATE E:383929.7 N: 6355629.8 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 511 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1



METHOD: 300mm solid flight auger to 0.6m

EXPORTED 20/04/23 11:38. TEMPLATE ID: DP_101.02.00_S0ILL0G



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 3.9 AHD COORDINATE E:383924.8 N: 6355661.7 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 512 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

	CONDITIONS ENCOUNTERED							SAMPLE				TESTING AND REMARKS		
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	
erved		0.0	TOPSOIL/FILL/ Silty SAND; brown dark brown; fine to medium; abundant organics		TOP and	NA	М			\mathcal{A}	⊂0.0⊅ −0.05−	-PID-	<1	
roundwater obse	-	0.1	FILL/ (SP) Gravelly SAND; grey; sand fraction fine to, medium plasticity; gravel fraction fine to coarse sub-angular to angular; trace ash gravels and coal reject	0	FILL	NA	М	PF	AS D E AS	1	-0.15-	-PID-	<1	
No free g	-	-	(SP) SAND, with silt; brown; fine to medium		ALV	NA	М	PF	D E AS	1	- 0.4	-PID-	3	
			(SP) SAND; grey; fine to medium		ALV	NA	М	PF	D E AS	1	- 0.8 -	-PID-	<1	
DKTED 20/04/23 11:38. TEMPLATE ID: DP_101.02.00_SOILLOG		1.0-	Test pit discontinued at 1.00m depth Limit of investigation											
	ES: (#)	Soil orig	in is "probable" unless otherwise stated. ^(*) Consistency/Relative density share	ling is for vi	sual refer	ence only -	no correla	ation between cohe	esive a	and gra	anular ma	aterials i	s implied.	
PL		• Ko	helco SK175R 1 7T Excavator with 300mm Auger		C	PFRAT		oreman					LOGGED. Helbig	

METHOD: 300mm solid flight auger to 1.0m



School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

CLIENT:

TEST PIT LOG

SURFACE LEVEL: 3.7 AHD COORDINATE E:383913.6 N: 6355650.4 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 513 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1



METHOD: 300mm solid flight auger to 0.7m



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South

TEST PIT LOG

 SURFACE LEVEL:
 3.9 AHD

 COORDINATE
 E:383869.9 N: 6355635.8

 DATUM/GRID:
 MGA94 Zone 56

LOCATION ID: 514 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAM	NPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
oundwater observed	-	0.0	FILL/ (SP) SAND, with silt; brown; fine to medium; trace ash gravels, coal reject, concrete pieces, brick fragments, imported fine to coarse sub-angular to angular gravels		FILL	NA	М		D E PFAS B	7	-0.1-	-PID-	<1
No free gr	_	0.5	(SP) SAND, trace gravel; brown dark brown; sand fraction fine to medium; gravel fraction fine sub-rounded		ALV	NA	м		D E PFAS	1	-0.3-	-PID-	<1
		- 0.5	(SP) SAND, trace gravel; yellow brown; sand fraction fine to medium; gravel fraction fine to medium rounded		ALV	NA	М		D E PFAS	1	- 0.8-	-PID-	<1
	_	1.0-	Test pit discontinued at 1 00m depth								- 1 -		
											· · · · · · · · · · · · · · · · · · ·		
	-	-									· · ·		
NOTES		- - oil origi	in is "probable" unless otherwise stated. ^(*) Consistency/Relative density share	ding is for vis	sual refer	ence only -	no correla	tion between	cohesive	and gra	anular ma	aterials i	s implied.

METHOD: 300mm solid flight auger to 1.0m

EXPORTED 20/04/23 11:39. TEMPLATE ID: DP_101.02.00_SOILLOG


TEST PIT LOG

 SURFACE LEVEL:
 3.9 AHD

 COORDINATE
 E:383863.9 N: 6355645.3

 DATUM/GRID:
 MGA94 Zone 56

LOCATION ID: 515 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*) DENSITY. ^(*)	MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
No free groundwater observed		0.0	FILL/ (SP) SAND; brown; fine to medium; trace rootlets to 0.1m, trace ash gravel, coal reject, concrete pieces, brick fragments, imported fine to coarse sub-angular to angular gravel, with trace ceramic tile fragments and asphalt		FILL	NA	Μ		D E PFAS B D		- 0.5 -	PID - - - PID	<1
	- - - -	0.6 -	(SP) SAND, trace gravel; yellow brown; sand fraction fine to medium; gravel fraction fine to medium rounded		ALV	NA	Μ		D E PFAS		- 0.9 -	- PID	<1
NOTES		2	Test pit discontinued at 1.10m depth Limit of investigation	ing is for via	sual refer	ence only -	no correla	ation between (cohesive	and grade	- 2 -	- - - - - - - - - - - - - - - - - - -	is implied.
PLA	NT	: Kol	pelco SK175R 1.7T Excavator with 300mm Auger		C	PERAT	OR:	oreman					LOGGED: Helbig

METHOD: 300mm solid flight auger to 1.1m

EXPORTED 20/04/23 11:39. TEMPLATE ID: DP_101.02.00_SOILLOG



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.1 AHD COORDINATE E:383855.8 N: 6355644.1 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 516 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
No free groundwater observed	- 4	0.0	FILL/ (SP) SAND, with silt; brown; fine to medium; trace rootlets to 0.1m, trace ash gravel, coal reject, concrete pieces, brick fragments, imported fine to coarse sub-angular to angular gravel, with trace asphalt and roof tile fragments		FILL	NA	М		D PFAS B D		- 0.5 -	-PID-	<1
		-	0.6m: 20mm asphalt layer—						PFAS				
	0	.65 -	FILL/ (SP) Gravelly SAND; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded; trace asphalt	000	FILL	NA	М		D E PFAS B	1	-0.7-	-PID-	<1
		1-	(SP) SAND; pale yellow pale brown; fine to medium							-	- 1 -		
	-ന	-			ALV	NA	М		D PFAS [_]	1	- 1.2 -	-PID-	<1
	-	_	Test pit discontinued at 1.40m depth Limit of investigation							-	 		
		2-								-	- 2 -		
		-											
NULES	. 50	n origi	This probable unless otherwise stated. Consistency/Relative density shad	ing is for vi	sual refer	ence only -	10 correl	auon between o	JUNESIVE	ano gra	anular ma	aterials	is implied.
PLAN	IT:	Kob	belco SK175R 1.7T Excavator with 300mm Auger		C	PERAT	OR: I	oreman					LOGGED: Helbig

METHOD: 300mm solid flight auger to 1.4m

EXPORTED 20/04/23 11:39. TEMPLATE ID: DP_101.02.00_SOILLOG



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.7 AHD COORDINATE E:383851.6 N: 6355653.3 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 517 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
observed GROUNDWATER	RL (m)	0 DEPTH (m)	DESCRIPTION OF STRATA FILL/ (SP) SAND, with silt, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded	GRAPHIC		GCONSIS. ^(*) ■ DENSITY. ^(*)	■ MOISTURE	REMARKS	D D E PFAS		(m) DELTH (m) T-0.0→	а тезт түре	RESULTS AND REMARKS
No free groundwater	-	0.2 -	FILL/ (SP) SAND, trace gravel; yellow pale brown; sand fraction fine to medium; gravel fraction fine to coarse angular to sub-angular; one fibro sheeting fragment at approximately 0.4m		FILL	NA	М		B D D PFAS	1	- 0.2 - 0.4	-PID-	<1
	- 4	0.6 -	FILL/ (SP) SAND, with gravel; brown dark brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded; trace brick fragments and whole bricks, roof tiles, steel plates, concrete fragments, metal reinforcing bars, ceramic fragments, asphalt, ash		FILL	NA	М		B D E PFAS	1	-0.6-	-PID-	<1
	-		FILL/ (SP) SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded; trace asphalt		FILL	NA	М		B D E PFAS	1	- 1.2-	-PID-	<1
	3		Test pit discontinued at 1.40m depth Pit collapse								- 2 -		
NOTES	- 5: ^(#) So	- oil orig	in is "probable" unless otherwise stated. ^{*/} Consistency/Relative density shad	ing is for vi	isual refer	ence only -	no correla	ation between 6	cohesive	and gra	anular ma	aterials i	s implied.

METHOD: 300mm GP Bucket to 1.4m

EXPORTED 01/05/23 10:46. TEMPLATE ID: DP_101.02.00_SOILLOG



CLIENT:School Infrastructure NSWPROJECT:Newcastle High School UpgradeLOCATION:Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.9 AHD COORDINATE E:383841.2 N: 6355653.1 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 518 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED	1				SAMP	E			TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*) DENSITY. ^(*)	MOISTURE	REMARKS	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
erved		0.0	FILL/ (SP) SAND; brown; fine to medium; with rootlets	[·[·[·]·	FILL	NA	М			0.01	Γ_PID-	<1
free groundwater obse	-	0.1 -	FILL/ (SP) SAND, trace gravel; yellow pale brown; sand fraction fine to medium; gravel fraction fine to coarse angular to sub-angular		FILL	NA	М			0.3	PID-	1
No	- 4	-	FILL/ (SP) SAND, with gravel; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded; whole bricks, trace concrete slabs and pieces, tiles, ceramic, fibro sheeting, metal sheeting, asphalt, slag, ash gravels		FILL	NA	М		D AS 3	0.5	PID	1
	-	1								1.0	PID	<1
	-	1.3 -	(SP) SAND, trace; brown; sand fraction fine to medium; fraction fine to medium sub-rounded gravels; silt		ALV	NA	М	PF	AS	1.3	PID	<1
	- - - - -	- - 2 -	Pit collapse							- 2		
		-								-		
	⊦ 3: ^(#) Si	oil orig	n is "probable" unless otherwise stated. "Consistency/Relative density shad	ling is for vi	isual refer			ation between cohe	esive and	l granular r	naterials	

METHOD: 300mm GP Bucket to 1.5m

EXPORTED 20/04/23 11:40. TEMPLATE ID: DP_101.02.00_SOILLOG



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

 LOCATION:
 Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.6 AHD COORDINATE E:383835.1 N: 6355642.4 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 519 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1



METHOD: 300mm GP Bucket to 1.4m

EXPORTED 20/04/23 11:41. TEMPLATE ID: DP_101.02.00_S0ILL0G



TEST PIT LOG

SURFACE LEVEL: 4.8 AHD COORDINATE E:383835.2 N: 6355628.6 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 520 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1



METHOD: 300mm GP Bucket to 0.9m



TEST PIT LOG

SURFACE LEVEL: 6.0 AHD COORDINATE E:383846.8 N: 6355620.7 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 521 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1



METHOD: 300mm GP Bucket to 0.8m



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

 SURFACE LEVEL:
 3.9 AHD

 COORDINATE
 E:383856.3 N: 6355625.1

 DATUM/GRID:
 MGA94 Zone 56

LOCATION ID: 522 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
bserved		0.0 0.1 -	FILL/ (SP) Gravelly SAND; brown; sand fraction fine to medium; gravel fraction fine to medium	0	FILL	NA	М		D E PFAS	Æ	-0.05 -0.05- -0.1-	-210-	~ 2
oundwater ol	-	0.18 _	(SP) SAND, trace gravel; pale brown pale orange; sand fraction fine to medium; gravel fraction fine to medium sub-rounded			NA	M		B D E PFAS	1	-0.15- -0.2-	-PID-	1
No free gro	-	-	FILL/ (SP) SAND, with gravel; brown coffee brown; sand fraction fine to medium; gravel fraction fine to medium sub-rounded; possibly with indurated sand		FILL	NA	М		B D E PFAS		- 0.5 -	-PID-	<1
	-	0.6 -	(SP) SAND, trace gravel; yellow brown; sand fraction fine to medium; gravel fraction fine to medium rounded							L	-0.6-		
	-e	1-			ALV	NA	Μ		D E PFAS		- 1.0 -	-PID-	<1
	-	12-											
	_	1. Z	Test pit discontinued at 1.20m depth Limit of investigation										
	_												
	-	-											
	-												
	-0												
	-	2-									- 2 -		
	-												
	-												
	_												
	-	-											
	-												
	-	-											
	-												
			75										
	s: ‴s	Soil orig	in is "probable" unless otherwise stated. "Consistency/Relative density shad	ling is for vis	sual refer	ence only -	no correla	ation between	cohesive	and gra	anular m	aterials	is implied. LOGGED: Helbia

METHOD: 300mm solid flight auger to 1.2m

EXPORTED 20/04/23 11:42. TEMPLATE ID: DP_101.02.00_SOILLOG



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

 SURFACE LEVEL:
 2.9 AHD

 COORDINATE
 E:383845.7 N: 6355688.5

 DATUM/GRID:
 MGA94 Zone 56

LOCATION ID: 523 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

	,		CONDITIONS ENCOUNTERED					SAMPL	E			TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
ee groundwater observed	-	0.0	FILL/ (SP) SAND, with silt, with gravel; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded; trace roots and rootlets		FILL	NA	М	D E PFA	S	0.2-	PID-	<1
No fi	-	0.56	FILL/ (SP) Gravelly SAND; brown orange; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded	0 0 0	FILL	NA	М	D E PFA	- S	0.4-	-PID-	<1
		v 1.	(SP) SAND; pale yellow; fine to medium		ALV	NA	М	D E PFA	S	0.8-	- PID	<1
	-	1.25								-	-	
	-		Liest pit discontinued at 1.25m depth Limit of investigation								-	
		-								- 2	-	
U 20/04/23 II:43. IEMPLAIE IU: U	-									-	-	
	= =S: (#	Soil ori	ain is "probable" unless otherwise stated. ⁽⁷ Consistency/Relative density shar	ling is for vi	isual refe	rence only -	no correl:	ation between cohes	ve and	granular m	aterials	is implied.
PLA	NT	r: Ha	nd Auger		() DPERA	OR:	Helbig				LOGGED: Helbig



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

 SURFACE LEVEL:
 7.3 AHD

 COORDINATE
 E:383879.9 N: 6355732.8

 DATUM/GRID:
 MGA94 Zone 56

LOCATION ID: 524 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

		CONDITIONS ENCOUNTERED					SAN	/IPLE				TESTING AND REMARKS
ved GROUNDWATER RL (m)	0 DEPTH (m)	DESCRIPTION OF STRATA FILL/ (SP) Silty SAND, trace gravel; brown dark	GRAPHIC		E CONSIS. ^(*) E DENSITY. ^(*)	■ MOISTURE	REMARKS	по ТҮРЕ		⊂0.0 [→]		RESULTS AND REMARKS
No free groundwater obser).1 - - -	brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular; trace soft plastics, trace roots and rootlets FILL/ (SP) SAND, with gravel; brown grey; sand fraction fine to medium; gravel fraction fine to coarse angular to sub-angular; trace coal reject		FILL	NA	М		PFAS B D E PFAS		- 0.2 - - 0.5 -	PID-	<1
	1-	(SP) SAND; pale brown; fine to medium		ALV	NA	М		D		— 0.6 — 	-PID-	<1
	-	Test pit discontinued at 1.10m depth Limit of investigation										
- - - -	2 -									- 2 -		
NOTES: ^(#) Soil	- - - - - - - - - - - - - - - - - - -	n is "probable" unless otherwise stated. ^{*/} Consistency/Relative density shad	ling is for vis	sual refer	ence only -	no correla	ation between	cohesive	e and gra	anular ma	aterials	is implied.

METHOD: 300mm solid flight auger to 1.1m

EXPORTED 20/04/23 11:43. TEMPLATE ID: DP_101.02.00_SOILLOG



CLIENT: School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South SURFACE LEVEL: 6.9 AHD COORDINATE E:383886.1 N: 6355741.2 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 525 DATE: 18/03/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
e groundwater observed	-	0.0	FILL/ (SP) Silty SAND, trace gravel; brown dark brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular; trace soft plastics, trace roots and rootlets		FILL	NA	М		D E PFAS B	<u>7</u>	-0.1-	-PID-	<1
No fre	-	0.4 -	sub-angular gravels, possibly bedding layer FILL/ (SP) SAND; brown; fine to medium		FILL	NA	М		D	_	-0.45-	-PID-	<1
	9	0.5 -	FILL/ (SP) SAND, with gravel; brown coffee brown; sand fraction fine to medium; gravel fraction fine to medium sub-rounded; possibly with indurated sand		FILL	NA	М		PFAS B D E PFAS		- 0.6 - - 0.8 -	- PID	<1
	-	1 - -	(SP) SAND; pale brown; fine to medium		ALV	NA	М		D E PFAS	1	- 1 -	PID	<1
		- - - - - - - - - - - - - - - - - - -	Test pit discontinued at 1.40m depth Limit of investigation							-	- 2 -		
	3: ‴S NT:	ioil orig	in is "probable" unless otherwise stated. "Consistency/Relative density share	ling is for vis	sual refer	ence only -	no correla	ation between o	cohesive	and gra	anular m	aterials i	is implied.

METHOD: 300mm solid flight auger to 1.4m

EXPORTED 20/04/23 11:44. TEMPLATE ID: DP_101.02.00_SOILLOG



BOREHOLE LOG

School Infrastructure NSW PROJECT: Newcastle High School Upgrade LOCATION: Parkway Avenue, Hamilton South

CLIENT:

SURFACE LEVEL: 3.9 AHD COORDINATE E:383920.1 N: 6355546.7 PROJECT No: 213618.02 DATUM/GRID: MGA94 Zone 56 **DIP/AZIMUTH:** 90°/---

LOCATION ID: 601 DATE: 18/03/23 SHEET: 1 of 1

	_		CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
erved		0.04			****	NA	NA						
dwater obse	-	0.15	FILD (SP) Gravely SAND; pale brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded; (pavement gravels)			NA	D 		D E PFAS	1	-0.15-	-PID-	<1
tree groun	-	0.35	FILL/ (SP) SAND, with gravel; grey; sand fraction fine to medium; gravel fraction fine to medium sub-angular to sub-rounded; (crushed natural						D E PFAS	1	-0.3-	-PID-	<1
ž	-	-	(SP) SAND; pale grey; fine to medium; trace rootlets		ALV	NA	М		D E PFAS	<u>_</u>	- 0.5 -	-PID-	<1
	m	0.9 ·	Borehole discontinued at 0.90m depth Limit of investigation		<u> </u>						- 1 -		
1 20/04/22 03:10. IEM-LALE IN: Dr_101.02.00_301LLU0		2-									- 2 -		
	S: (#)	Soil orig	in is "probable" unless otherwise stated. ⁽¹⁾ Consistency/Relative density shac	ling is for vi	sual refer	ence only -	no correla	tion between	cohesive	and gr	anular ma	aterials	is implied.
PLA	NT	: 5T	Excavator with 500mm Bucket	-	0	OPERAT	OR: F	oster					LOGGED: Kramer



BOREHOLE LOG

SURFACE LEVEL: 3.9 AHD COORDINATE E:383925.6 N: 6355567.7 DATUM/GRID: MGA94 Zone 56 DIP/AZIMUTH: 90°/---

LOCATION ID: 602 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	/IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
served	-	0.04	ASPHALTIC CONCRETE; black	$\left(\begin{array}{c} 0 \end{array} \right)$	Eur	NA	NA				-0.01-		
ter obs	'	0.15	fraction fine to medium; gravel fraction fine to			NA	0		E		-0.1-	-910-	
undwa	-	-	gravels) (SP) SAND: polo grav: fino to modium						D		-0.25-	-PID-	<1
ee gro	L	-	(SF) SAND, pale grey, fille to medium		ALV	NA	D						
No fi	_	-											
	-	-							D E	/	-0.5-	-PID-	<1
		0.6 -	Borehole discontinued at 0.60m depth	<u></u>									
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	-0	-											
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NOTE	5: ^(#) S	oil orig	in is "probable" unless otherwise stated. "Consistency/Relative density share	ding is for vi	sual refer	ence only -	no correla	ation between	cohesive	e and gra	anular m	aterials	is implied.
PLA	NT: HO	5T D:4	Excavator with 450mm Auger		C C	PERA	f or:	oster					LOGGED: Kramer

REMARKS: Location coordinates are in MGA94 Zone 56. Coordinates obtained using differential GPS, typical accuracy ±0.1m.



EXPORTED 20/04/23 11:45. TEMPLATE ID: DP_101.02.00_SOILLOG

TEST PIT LOG

SURFACE LEVEL: 4.0 AHD COORDINATE E:383912.2 N: 6355599.3 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 603 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1



METHOD: 500mm bucket to 1.8m



TEST PIT LOG

SURFACE LEVEL: 3.8 AHD COORDINATE E:383908.3 N: 6355606.3 DATUM/GRID: MGA94 Zone 56 LOCATION ID: 604 PROJECT No: 213618.02 DATE: 18/03/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED	1				SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
groundwater observed	-	0.0	FILL/ (SP) SAND, trace gravel; brown grey; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded; (crushed natural rock), rootlets, sandstone boulders, roots		FILL	NA	D		D E PFAS B D E PFAS	1	-0.25-	PID	<1
No free	-	0.35 - -	(SP) SAND; pale grey; fine to medium; trace rootlets 0.65m: brown, trace fine rounded gravels		ALV	NA	М		D E PFAS		-0.35-	PID	<1
	-ო -	-											
	-	0.9 -	Test pit discontinued at 0.90m depth Limit of investigation	<u> </u>					, ,	-	- 1 -		
	_	-								-			
	_	-								-			
	-	-								-			
	-2	-								-			
	_	-								-			
	_	2-								-	- 2 -	-	
	-	-								-		-	
	-	-											
	-	-											
		-								-			
NOTES	- S: ^(#) S	- Soil orig	in is "probable" unless otherwise stated. ^{(*} Consistency/Relative density shad	ling is for vi	sual refer	ence only -	no correla	ation between	cohesive	and gra	anular m	aterials	is implied.
PLA	NT	: 5T	Excavator with 500mm Bucket		C	PERAT	f or : F	Foster					LOGGED: Kramer

METHOD: 500mm bucket to 0.9m

EXPORTED 20/04/23 11:46. TEMPLATE ID: DP_101.02.00_SOILLOG



 CLIENT:
 School Infrastructure NSW

 PROJECT:
 Newcastle High School Upgrade

LOCATION: Parkway Avenue, Hamilton South

SURFACE LEVEL: 4.3 AHD COORDINATE E:383871.5 N: 6355691.9 DATUM/GRID: MGA94 Zone 56

LOCATION ID: 605 PROJECT No: 213618.02 DATE: 19/03/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
groundwater observed	- 4	0.0	FILL/ (SP) SAND, with silt, with gravel; brown; sand fraction fine to medium; gravel fraction fine to coarse sub-angular to sub-rounded; trace roots and rootlets		FILL	NA	М		D E PFAS	₹{	-0.2-	PID	<1
No free	-	0.4 -	FILL/ (SP) SAND, trace gravel; brown; sand fraction fine to medium; gravel fraction fine to medium sub-angular; gravels comprise ash and imported igneous gravels (SP) SAND: nale grav nale brown; fine to		FILL	NA	М		D E PFAS	₹	- 0.4 - - 0.6 -	PID	<1
	-	- - 1	medium		ALV	NA	М		D E PFAS	7		- PID	<1
	m	1.2 -	Test pit discontinued at 1.20m depth Limit of investigation		1							-	
	_	-											
	-	2-									- 2 -		
	5-	-									· ·		
	-	-										-	
	 s: ^{##} s NT:	oil orig Hai	n is "probable" unless otherwise stated. ^[7] Consistency/Relative density shad	ling is for vi	isual refer	ence only -	no correl	ation between o	cohesive	and gra	anular m	aterials i	s implied. LOGGED: Helbig

METHOD: 100mm hand auger to 1.2m

EXPORTED 20/04/23 11:46. TEMPLATE ID: DP_101.02.00_SOILLOG



CLIENT: SCHOOL INFRASTRUCTURE NSW

PROJECT: NEWCASTLE HIGH SCHOOL UPGRADE

LOCATION: 160-200 PARKWAY AVENUE, HAMILTON SOUTH

REDUCED LEVEL:2.6

COORDINATES: 384035.8E 6355583.4N AHD

CPT101 Page 1 of 2 DATE 13/07/2022 PROJECT No: 213618.01

Douglas Partners Geotechnics | Environment | Groundwster



REMARKS: TEST DISCONTINUED DUE TO SUDDEN BEND ON HARD MATERIAL GROUNDWATER LEVEL OBSERVED AT 0.7M AFTER WITHDRAWAL OF RODS

Water depth after test: 0.70m depth (measured)

 File:
 P:\213618.01 - HAMILTON SOUTH, Newcastle High Drilling\4.0 Field Work\CPT Logs\CPT101.CP5

 Cone ID:
 170705
 Type:
 I-CFXY-10

CLIENT: SCHOOL INFRASTRUCTURE NSW

PROJECT: NEWCASTLE HIGH SCHOOL UPGRADE

LOCATION: 160-200 PARKWAY AVENUE, HAMILTON SOUTH

REDUCED LEVEL:2.6

COORDINATES: 384035.8E 6355583.4N AHD

 CPT101

 Page 2 of 2

 DATE
 13/07/2022

 PROJECT No: 213618.01



REMARKS: TEST DISCONTINUED DUE TO SUDDEN BEND ON HARD MATERIAL GROUNDWATER LEVEL OBSERVED AT 0.7M AFTER WITHDRAWAL OF RODS

Water depth after test: 0.70m depth (measured)

 File:
 P:\213618.01 - HAMILTON SOUTH, Newcastle High Drilling\4.0 Field Work\CPT Logs\CPT101.CP5

 Cone ID:
 170705
 Type:
 I-CFXY-10



CLIENT: SCHOOL INFRASTRUCTURE NSW

PROJECT: NEWCASTLE HIGHSCHOOL UPGRADE

LOCATION: 160-200 PARKWAY AVENUE, HAMILTON SOUTH

REDUCED LEVEL:2.3

COORDINATES: 384014.1E 6355610.4N AHD

 CPT102

 Page 1 of 2

 DATE
 14/07/2022

 PROJECT No: 213618.01

Douglas Partners
 Geotechnics | Environment | Groundwster



REMARKS: TEST DISCONTINUED DUE TO EXCESSIVE ROD BOWING IN INFERRED WEATHERED ROCK GROUNDWATER LEVEL OBSERVED AT 0.5M AFTER WITHDRAWAL OF RODS

Water depth after test: 0.50m depth (measured)

 File:
 P:\213618.01 - HAMILTON SOUTH, Newcastle High Drilling\4.0 Field Work\CPT Logs\CPT102.CP5

 Cone ID:
 170705
 Type:
 I-CFXY-10

CLIENT: SCHOOL INFRASTRUCTURE NSW

PROJECT: NEWCASTLE HIGHSCHOOL UPGRADE

LOCATION: 160-200 PARKWAY AVENUE, HAMILTON SOUTH

REDUCED LEVEL:2.3

COORDINATES: 384014.1E 6355610.4N AHD

 CPT102

 Page 2 of 2

 DATE
 14/07/2022

 PROJECT No: 213618.01

Cone Resi g _c (MPa)	istance	Sleeve Friction f _s (kPa)		Friction Ratio R _f (%)
pth			Soil Behaviour Type	
$\begin{array}{c} (m) & 0.0 & 1.0 \\ 20 & 1.0 \\ 21 & 0.0 \\ 21 & 0.0 \\ 21 & 0.0 \\ 22 & 0.0 \\ 23 & 0.0 \\ 24 & 0.0 \\ 25 & 0.0 \\ 26 & 0.0 \\ 27 & 0.0 \\ 28 & 0.0 \\ 29 & 0.0 \\ 29 & 0.0 \\ 31 & 0.0$			CLAY: Stiff to Very Stiff CLAY with some SILTY CLAY / CLAYEY SILT: Very Stiff to Hard	
32 End at 31.9	94m q _c = 19.7			31.94
34 -				
35 -				
36 -				
7-				
в-				

REMARKS: TEST DISCONTINUED DUE TO EXCESSIVE ROD BOWING IN INFERRED WEATHERED ROCK GROUNDWATER LEVEL OBSERVED AT 0.5M AFTER WITHDRAWAL OF RODS

Water depth after test: 0.50m depth (measured)

File: P:\213618.01 - HAMILTON SOUTH, Newcastle High Drilling\4.0 Field Work\CPT Logs\CPT102.CP5
Cone ID: 170705
Type: I-CFXY-10



CLIENT: SCHOOL INFRASTRUCTURE NSW

PROJECT: NEWCASTLE HIGHSCHOOL UPGRADE

LOCATION: 160-200 PARKWAY AVENUE, HAMILTON SOUTH

REDUCED LEVEL:3.3

COORDINATES: 383991.2E 6355578.0N AHD

 CPT103

 Page 1 of 2

 DATE
 13/07/2022

 PROJECT No: 213618.01

		Cone Resistance g _o (MPa)	Sleeve Friction fo (kPa)	Friction Ratio R _f (%)
Depth			0 100 200 300 400 500 Soil Behaviour Type	0 2 4 6 8 10 Depti
(m) [⁰	C	0.0 1.0 2.0 3.0 4.0 5.0	GRAVELLY SAND with some SAND:	Γ ^(m)
			Medium Dense (FILL?)	
1-	¥		SAND: Loose to Medium Dense	
		No. Contraction of the second se		
2-				
3-				- 3
		The second		
4 -				- 4
5 -				5
6-			Clayey layer (<0.3 m thick)	
0				
7 -			CLAY: Firm becoming Stiff 6.91	7
				3
8-				- 8
			SAND. Medium Dance to Dance to Dance 8.80	
9-			SAND. Medium Dense to Dense	9
10 -				- 10
11 -				- 11
12 -				12
13 -				- 13
			CLAY: Stiff 13.22	
14 -				- 14
15 -				- 15
10				
16 -		No. State	CLAY: Very Stiff	
17 -				17
18 -				18
19 -				19
20				

REMARKS: TEST DISCONTINUED DUE TO SUDDEN BEND ON HARD MATERIAL GROUNDWATER LEVEL OBSERVED AT 1.0M AFTER WITHDRAWAL OF RODS

Water depth after test: 1.00m depth (measured)

File: P:\213618.01 - HAMILTON SOUTH, Newcastle High Drilling\4.0 Field Work\CPT Logs\CPT103.CP5
Cone ID: 170705
Type: I-CFXY-10



CLIENT: SCHOOL INFRASTRUCTURE NSW

PROJECT: NEWCASTLE HIGHSCHOOL UPGRADE

LOCATION: 160-200 PARKWAY AVENUE, HAMILTON SOUTH

REDUCED LEVEL:3.3

COORDINATES: 383991.2E 6355578.0N AHD

 CPT103

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 DATE
 13/07/2022

 PROJECT No: 213618.01

Cone Resistance	Sleeve Friction		Friction Ratio
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	0 100 200 300 400 500		
0.0 1.0 2.0 3.0 4.0 5.0			
		CLAY: Very Stiff	2
		20.5 m to 21.5 m, Hard	
<u> </u>			
End at 22.74m q _c = 4.9			22.74

REMARKS: TEST DISCONTINUED DUE TO SUDDEN BEND ON HARD MATERIAL GROUNDWATER LEVEL OBSERVED AT 1.0M AFTER WITHDRAWAL OF RODS

Water depth after test: 1.00m depth (measured)

File: P:\213618.01 - HAMILTON SOUTH, Newcastle High Drilling\4.0 Field Work\CPT Logs\CPT103.CP5
Cone ID: 170705
Type: I-CFXY-10



CLIENT: SCHOOL INFRASTRUCTURE NSW

PROJECT: NEWCASTLE HIGH SCHOOL UPGRADE

LOCATION: 160-200 PARKWAY AVENUE, HAMILTON SOUTH

REDUCED LEVEL:4.1

COORDINATES: 383825.6E 6355634.9N AHD

 CPT104

 Page 1 of 2

 DATE
 13/07/2022

 PROJECT No: 213618.01

Douglas Partners
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REMARKS: TEST DISCONTINUED DUE TO EXCESSIVE ROD BOWING IN INFERRED WEATHERED ROCK GROUNDWATER LEVEL OBSERVED AT 1.6M AFTER WITHDRAWAL OF RODS

Water depth after test: 1.60m depth (measured)

 File:
 P:\213618.01 - HAMILTON SOUTH, Newcastle High Drilling\4.0 Field Work\CPT Logs\CPT104.CP5

 Cone ID:
 170705
 Type:
 I-CFXY-10

CLIENT: SCHOOL INFRASTRUCTURE NSW

PROJECT: NEWCASTLE HIGH SCHOOL UPGRADE

LOCATION: 160-200 PARKWAY AVENUE, HAMILTON SOUTH

REDUCED LEVEL:4.1

COORDINATES: 383825.6E 6355634.9N AHD

 CPT104

 Page 2 of 2

 DATE
 13/07/2022

 PROJECT No: 213618.01



REMARKS: TEST DISCONTINUED DUE TO EXCESSIVE ROD BOWING IN INFERRED WEATHERED ROCK GROUNDWATER LEVEL OBSERVED AT 1.6M AFTER WITHDRAWAL OF RODS

Water depth after test: 1.60m depth (measured)

 File:
 P:\213618.01 - HAMILTON SOUTH, Newcastle High Drilling\4.0 Field Work\CPT Logs\CPT104.CP5

 Cone ID:
 170705
 Type:
 I-CFXY-10



CLIENT: SCHOOL INFRASTRUCTURE NSW

PROJECT: NEWCASTLE HIGH SCHOOL UPGRADE

160-200 PARKWAY AVENUE, HAMILTON SOUTH LOCATION:

CPT105 Page 1 of 2 DATE 14/07/2022

Douglas Partners Geotechnics | Environment | Groundwster

PROJECT No: 213618.01



REDUCED LEVEL:4.1

REMARKS: TEST DISCONTINUED DUE TO EXCESSIVE ROD BOWING IN INFERRED WEATHERED ROCK GROUNDWATER LEVEL OBSERVED AT 1.7M AFTER WITHDRAWAL OF RODS

Water depth after test: 1.70m depth (measured)

File: P:\213618.01 - HAMILTON SOUTH, Newcastle High Drilling\4.0 Field Work\CPT Logs\CPT105.CP5 Cone ID: 170705 Type: I-CFXY-10

CLIENT: SCHOOL INFRASTRUCTURE NSW

PROJECT: NEWCASTLE HIGH SCHOOL UPGRADE

LOCATION: 160-200 PARKWAY AVENUE, HAMILTON SOUTH

REDUCED LEVEL:4.1

COORDINATES: 383849.0E 6355627.1N AHD

 CPT105

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 DATE
 14/07/2022

 PROJECT No: 213618.01



REMARKS: TEST DISCONTINUED DUE TO EXCESSIVE ROD BOWING IN INFERRED WEATHERED ROCK GROUNDWATER LEVEL OBSERVED AT 1.7M AFTER WITHDRAWAL OF RODS

Water depth after test: 1.70m depth (measured)

 File:
 P:\213618.01 - HAMILTON SOUTH, Newcastle High Drilling\4.0 Field Work\CPT Logs\CPT105.CP5

 Cone ID:
 170705
 Type:
 I-CFXY-10



CLIENT: SCHOOL INFRASTRUCTURE NSW

PROJECT: NEWCASTLE HIGHSCHOOL UPGRADE

LOCATION: 160-200 PARKWAY AVENUE, HAMILTON SOUTH

REDUCED LEVEL:4.0

COORDINATES: 383803.1E 6355604.1N AHD

 CPT106

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 DATE
 13/07/2022

 PROJECT No: 213618.01



REMARKS: TEST DISCONTINUED DUE TO EXCESSIVE BENDING IN VERY DENSE SANDS. ASPHALT 30MM THICK. GROUNDWATER LEVEL OBSERVED AT 1.5M AFTER WITHDRAWAL OF RODS

Water depth after test: 1.50m depth (measured)

 File:
 P:\213618.01 - HAMILTON SOUTH, Newcastle High Drilling\4.0 Field Work\CPT Logs\CPT106.CP5

 Cone ID:
 170705
 Type:
 I-CFXY-10



CLIENT: SCHOOL INFRASTRUCTURE NSW

PROJECT: NEWCASTLE HIGH SCHOOL UPGRADE

LOCATION: 160-200 PARKWAY AVENUE, HAMILTON SOUTH

REDUCED LEVEL: 3.9

COORDINATES: 383822.5E 6355565.9N AHD

 CPT107

 Page 1 of 1

 DATE
 13/07/2022

 PROJECT No: 213618.01



REMARKS: TEST DISCONTINUED DUE TO EXCESSIVE BENDING IN VERY DENSE SANDS GROUNDWATER LEVEL OBSERVED AT 1.4M AFTER WITHDRAWAL OF RODS

Water depth after test: 1.40m depth (measured)

 File:
 P:\213618.01 - HAMILTON SOUTH, Newcastle High Drilling\4.0 Field Work\CPT Logs\CPT107.CP5

 Cone ID:
 170705
 Type:
 I-CFXY-10



Appendix B

Table B1: Results of Groundwater and Surface Water Analysis (Douglas, 2023b)

Table B2: Summary of Groundwater Levels and Field Parameters during Purging and Sampling (Douglas, 2023b)

Table B1: Results of Groundwater and Surface Water				alysis (DP, 2023b)	401	402	403	Ground	lwater	404	405	406	D/SW	Surfac	e water	514/2
				Sample Location	401 401	402 402	403	403 403		404 404	405	406	SW1	SW1 SW1	SW1	SW2 SW2	
				ANZG (2018) Freshwater		23/03/2023	23/03/2023	23/03/2023	23/03/2023	23/03/2023	23/03/2023	23/03/2023	23/03/2023	30/03/2023	30/03/2023	30/03/2023	30/03/2023
Group	Analyte	Units	POL	Slightly to Moderately Disturbed Systems DGVs	Freshwater 99%												
Metals (Disastured)	Aluminium (Filtered)	mg/L	0.01	0.055		-	-	-	-	-	-	-	-	-	-	-	-
(Dissolved)	Beryllium (Filtered)	mg/L mg/L	0.0005	0.013		<0.0005	<0.0005	<0.002	<0.002	-	<0.001	<0.0005	<0.0005	<0.001	<0.0005	<0.001	<0.001
	Cadmium (Filtered) Chromium (III+VII) (Filtered)	mg/L mg/L	0.0001	0.0002		<0.0001	<0.0001	<0.001	<0.001	<0.0001	<0.001	<0.001	<0.001	<0.001	<0.001	<0.0001	<0.001
	Cobalt (Filtered)	mg/L mg/l	0.001	0.0014		<0.001	<0.001	<0.001	<0.001	-	<0.001	<0.001	<0.001	<0.001	<0.001	<0.001	<0.001
	Lead (Filtered) Manganese (Filtered)	mg/L mg/l	0.001	0.0034		<0.001	<0.001	<0.001	<0.001	<0.001	<0.001	<0.001	<0.001	<0.001	<0.001	<0.001	0.001
	Mercury (Filtered) Nickel (Filtered)	mg/L mg/L	0.00005 / 0.0001	0.00006		<0.00005	<0.00005	<0.0005	<0.00005	<0.0001 0.010	<0.0005	<0.00005	<0.00005	<0.0005	<0.0005 <0.001	<0.0005	<0.00005 <0.001
	Selenium (Filtered) Silver (Filtered)	mg/L mg/L	0.001 0.001	0.005		<0.001	<0.001	<0.001	<0.001	-	<0.001	<0.001	<0.001	<0.001	<0.001	<0.001	<0.001
Metals (Total)	Zinc (Filtered) Arsenic	mg/L mg/L	0.001 / 0.005	0.008 0.013		0.01	0.044	0.016	0.017	0.015	0.044	0.064	0.034	0.035 <0.001	0.036	0.035 <0.001	0.03
	Beryllium Boron	mg/L mg/L	0.0005	0.37		-	-	-	-	-	-	-	-	<0.0005 0.04	<0.0005 0.04	<0.0005 0.04	<0.0005 0.06
	Cadmium Chromium (III+VI)	mg/L mg/L	0.0001	0.0002		-	-	-	-	-	-	-	-	<0.0001 <0.001	<0.0001 <0.001	<0.0001 <0.001	<0.0001 <0.001
	Cobalt Copper	mg/L mg/L	0.001 0.001	0.0014		-	-	-	-	-	-	-	-	<0.001 0.002	<0.001 0.002	<0.001 0.002	<0.001 0.004
	Lead Manganese	mg/L mg/L	0.001	0.0034		-	-	-	-	-	-	-	-	0.002	0.002	0.002	0.001 0.057
	Mercury Nickel	mg/L mg/L	0.00005	0.00006		-	-	-	-	-	-	-	-	<0.00005 <0.001	<0.00005 <0.001	<0.0005 <0.001	<0.00005 <0.001
2411	Zinc	mg/L mg/L	0.001	0.005		-	-	-	-	-	-	-	-	<0.001	<0.001	<0.001	<0.001
PAHS	Acenaphthene	mg/L μg/L	0.0002			<0.1	<0.1	<0.0002	<0.0002	<0.1	0.0003	<0.0002	<0.0002	<0.0002	<0.0002	<0.0002	<0.0002
	Anthracene Benz(a)anthracene	μg/L μg/L	0.1	0.01		<0.1	0.1	<0.1	<0.1	<0.1	0.2	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
	Benzo(a) pyrene Benzo(g.b.i)pervlene	μg/L μg/l	0.1/0.05	0.1		0.1	0.7	<0.1	<0.1	<0.05	0.2	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
	Chrysene Dibenz(a,h)anthracene	μg/L μg/L	0.1			0.1	0.5	<0.1	<0.1	<0.1	0.2	<0.1	<0.1	<0.1 <0.1	0.1	<0.1 <0.1	<0.1
	Fluoranthene Fluorene	μg/L μg/L	0.1	1		0.3	0.9	<0.1	0.1	<0.1	0.6	<0.1	<0.1 <0.1	<0.1	0.2	<0.1 <0.1	<0.1 <0.1
	Indeno(1,2,3-c,d)pyrene Naphthalene	μg/L μg/L	0.1	16		<0.1 <0.2	0.3	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1 <0.2	<0.1 <0.2
	Phenanthrene Benzo(a)pyrene TEQ	μg/L mg/L	0.1 0.0005	0.6		0.1 <0.0005	0.4	<0.1 <0.0005	<0.1 <0.0005	<0.1 <0.0005	0.8 <0.0005	<0.1 <0.0005	<0.1 <0.0005	<0.1 <0.0005	<0.1 <0.0005	<0.1 <0.0005	<0.1 <0.0005
	Pyrene PAHs (Sum of positives)	μg/L mg/L	0.1 0.0001			0.3 0.0013	0.8	<0.1 <0.0001	0.1 0.00022	<0.1	0.5	<0.1 <0.0001	<0.1 <0.0001	<0.1 <0.0001	0.2	<0.1 <0.0001	<0.1 <0.0001
PFAS	10:2 Fluorotelomer sulfonic acid (10:2 FTS)	μg/L	0.002 / 0.005			<0.002	<0.002	<0.002	<0.002	<0.005	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002
	4:2 Fluorotelomer sulfonic acid (4:2 FTS)	μg/L	0.002 / 0.005			<0.001	<0.001	<0.001	<0.001	<0.005	<0.001	<0.001	<0.001	<0.001	<0.001	<0.001	<0.001
	N-ethyl-perfluorooctanesulfonamidoacetic acid	μg/L	0.002 / 0.005			<0.02	<0.004	<0.002	<0.002	<0.005	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002
	N-methylperfluorooctane	μg/L	0.002 / 0.005			<0.02	<0.02	<0.002	<0.002	<0.005	<0.002	<0.002	<0.004	<0.002	<0.002	<0.002	<0.002
	sulfonamidoacetic acid N-	μg/L	0.05 / 0.005			<0.05	<0.05	<0.05	<0.05	<0.005	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
	Netnyiperriuorooctanesuifonamidoetnano		0.002 (0.01			-0.01	-0.004	-0.01	-0.01	-0.05	0.005	0.02	0.004	0.007	0.007	0.007	0.007
	Perfluorodecanesulfonic acid (PFDS) Perfluorodecanesulfonic acid (PFDS) Perfluorobectanes sulfanis acid (PFDS)	μg/L μg/L	0.002 / 0.01			<0.002	<0.004	<0.002	<0.002	<0.05	<0.008	<0.002	<0.004	<0.007	<0.007	<0.007	<0.007
	Perfluoropentane sulfonic acid (PEPeS)	ug/l	0.001 / 0.002			<0.001	<0.001	0.001	0.001	<0.010	<0.001	0.001	<0.001	<0.001	<0.001	<0.001	<0.001
	Perfluoropentanoic acid (PFPeA) Sum of PEAS	μg/L μg/l	0.002			<0.002	<0.002	0.002	0.002	<0.010	0.008	0.025	<0.002	0.01	0.01	0.01	0.01
	Sum of PFHxS and PFOS Sum of US EPA PFAS (PFOS + PFOA)*	μg/L μg/L	0.0002 / 0.002			0.012	0.002	0.087	0.088	0.099	0.023	0.065	0.05	0.035	0.037	0.035	0.035
	8:2 Fluorotelomer sulfonic acid (8:2 FTS)	mg/L	0.0000004 / 0.000005			<0.000008	<0.0000004	<0.000004	<0.0000004	<0.005	<0.0000004	<0.0000004	<0.000004	<0.000004	<0.0000004	<0.000004	<0.0000004
	Perfluorodecanoic acid (PFDA) Perfluorohexanoic acid (PFHxA)	μg/L μg/L	0.002			<0.02 <0.0004	<0.002 <0.0004	<0.002 0.004	<0.002 0.004	<0.010 <0.010	<0.002 0.0068	<0.002 0.043	<0.002 0.002	<0.002 0.014	<0.002 0.015	<0.002 0.014	<0.002 0.011
	N-Ethyl perfluorooctane sulfonamide (NEtFOSA)	μg/L	0.1/0.005			<0.1	<0.1	<0.1	<0.1	<0.025	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
	N- ethylperfluorooctanesulfonamidoethanol	μg/L	0.5 / 0.005			<0.5	<0.5	<0.5	<0.5	<0.025	<0.5	<0.5	<0.5	<0.5	<0.5	<0.5	<0.5
	(NEtFOSE) N-Methyl perfluorooctane sulfonamide	μg/L	0.05 / 0.002			<0.05	<0.05	<0.05	<0.05	<0.010	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
	(NMeFOSA) Perfluorobutane sulfonic acid (PFBS)	μg/L	0.0004 / 0.002			0.001	<0.0004	0.002	0.002	<0.010	0.001	0.003	0.002	0.003	0.003	0.003	0.002
	Perfluorooctanesulfonic acid (PFOS) Perfluorododecanoic acid (PFDoDA)	mg/L μg/L	0.000002		0.0000023	0.0000083 <0.05	0.000002 <0.01	0.000064 <0.005	0.000065 <0.005	0.000072 <0.010	0.00002 <0.005	0.00005 <0.005	0.000041 <0.02	0.000025 <0.005	0.000026 <0.005	0.00025 <0.005	0.000024 <0.005
	Perfluoroheptanoic acid (PFHpA) Perfluorohexane sulfonic acid (PFHxS)	μg/L μg/L	0.0004 / 0.002			<0.0004 0.0039	<0.0004	0.002	0.002	<0.010	0.0046	0.012	0.001 0.0089	0.0074	0.0074	0.0074	0.0063
	Perfluorononanoic acid (PFNA) Perfluorooctane sulfonamide (PFOSA) Perfluorotetradecanoic acid (PETEDA)	μg/L μg/L	0.01/0.002			<0.002	<0.001	<0.001	<0.001	<0.010	<0.001	<0.001	<0.001	<0.01	<0.01	<0.01	<0.01
	Perfluorotridecanoic acid (PFTrDA) Perfluoroundecanoic acid (PFUnDA)	μg/L μg/l	0.01 / 0.002			<0.03	<0.01	<0.01	<0.01	<0.023	<0.01	<0.01	<0.01	<0.03	<0.01	<0.01	<0.01
	6:2 Fluorotelomer sulfonic acid (6:2 FTS)	mg/L	0.0000004 / 0.005			<0.000004	<0.0000004	<0.000004	<0.0000004	<0.000010	<0.0000004	<0.0000004	<0.0000004	<0.0000004	<0.0000004	<0.0000004	<0.0000004
TRH	Perfluorooctanoic acid (PFOA) C10-C14	mg/L μg/L	0.0000002		0.019	0.000001 <50	<0.000002 <50	0.000088 <50	0.0000089 <50	0.000013 <50	0.0000073	0.000017 <50	0.0000069	0.000026 <50	0.000025 <50	0.000026 <50	0.000021 <50
	C15-C28 C29-C36	μg/L μg/L	100 100 / 50			<100 <100	<100 <100	<100 <100	<100 <100	<100 <50	<100 <100	<100 <100	<100 <100	<100 <100	<100 <100	<100 <100	<100 <100
	C10-C16 C10-C16 (F2 minus Naphthalene)	μg/L μg/L	50 / 100 50 / 100			<50 <50	<50 <50	<50 <50	<50 <50	<100 <100	<50 <50	<50 <50	<50 <50	<50 <50	<50 <50	<50 <50	<50 <50
	C16-C34 C34-C40	μg/L μg/L	100 100			<100 <100	<100 <100	<100 <100	<100 <100	<100 <100	<100 <100	<100 <100	<100 <100	<100 <100	<100 <100	<100 <100	<100 <100
	C10-C40 (Sum of total) C10-C36	μg/L μg/L	50 / 100 50			<50 <50	<50 <50	<50 <50	<50 <50	<100 <50	<50 <50	<50 <50	<50 <50	<50 <50	<50 <50	<50 <50	<50 <50
voc	1,1,1,2-tetrachloroethane 1,1,1-trichloroethane	μg/L μg/L	1/5	270		<1 <1	<1 <1	<1	<1 <1	<5	<1	<1 <1	<1	<1 <1	<1 <1	<1 <1	<1 <1
	1,1,2,2-tetrachloroethane 1,1,2-trichloroethane	μg/L μg/L	1/5	400 6500		<1	<1 <1	<1	<1 <1	<5	<1 <1	<1	<1	<1 <1 1	<1	<1 <1 <1	<1 <1
	1,1-dichloroethene	μg/L μg/L	1/5	700		<1	<1	<1	<1	<5	4	<1	<1 <1	<1	<1	<1	<1 <1
	1,2,3-trichlorobenzene	μg/L μg/L	1/5	3		<1	4	<1 <1	<1	<5	<1	<1	<1	<1	<1	<1	<1 <1
	1,2,3-trichlorobenzene 1,2,4-trichlorobenzene 1,2,4-trimethylbenzene	μg/L μg/L	1/5	85		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	1,2-dibromo-3-chloropropane 1,2-dibromoethane	на/L μg/L μg/I	1/5			<1 <1	<1 <1	<1 <1	<1	<5 <5	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1
	1,2-dichlorobenzene 1,2-dichloroethane	μg/L μg/l	1/5	160 1900		<1 <1	<1 <1	<1 <1	<1	<5 <5	<1 <1	<1	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1
	1,2-dichloropropane 1,3,5-trimethylbenzene	μg/L μg/L	1/5	900		<1 <1	<1 <1	<1 <1	<1 <1	<5	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1
	1,3-dichlorobenzene 1,3-dichloropropane	μg/L μg/L	1/5	260 1100		<1 <1	<1	<1	<1 <1	<5 <5	<1 <1	<1	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1
	1,4-dichlorobenzene 2,2-dichloropropane	μg/L μg/L	1/5	60		<1 <1	<1 <1	<1 <1	<1 <1	<5 <5	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1
	2-chlorotoluene 4-chlorotoluene	μg/L μg/L	1/5			<1 <1	<1 <1	<1 <1	<1 <1	<5 <5	<1	<1 <1	<1	<1 <1	<1 <1	<1 <1	<1 <1
	Benzene Bromobenzene	μg/L μg/L	1/5	600		<1 <1	<1 <1	<1 <1	<1 <1	<5 <5	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1	<1 <1
	Bromochloromethane	μg/L μg/L	1/5			<1 <1	<1 <1	<1	<1	<5 <5	<1	<1 <1	<1	<1 <1	<1 <1	<1 <1	<1 <1
	Bromomethane	μg/L μg/L	1/5 10/50	240		<1 <10	<1 <10	<1 <10	<1 <10	<5	<1 <10	<1 <10	<1	<1 <10	<1 <10	<1 <10	<1 <10
1	cal bon ten achionide	HR/L	1/3	240		×1	~1	×1	×1	~>	~1	~1	×1	×1	~1	×1	×1

	Chiorobenzene	µg/L	1/5	22	<1	<1	<1	<1	<>	<1	<1	<1	<1	<1	<1	<1
	Chlorodibromomethane	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	Chloroethane	μg/L	10/50		<10	<10	<10	<10	<50	<10	<10	<10	<10	<10	<10	<10
	Chloroform	μg/L	1/5	370	<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	Chloromethane	μg/L	10/50		<10	<10	<10	<10	<50	<10	<10	<10	<10	<10	<10	<10
	cis-1,2-dichloroethene	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	cis-1,3-dichloropropene	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	Cyclohexane	mg/L	0.001		< 0.001	< 0.001	< 0.001	< 0.001	<5	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
	Dibromomethane	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	Dichlorodifluoromethane	μg/L	10/50		<10	<10	<10	<10	<50	<10	<10	<10	<10	<10	<10	<10
	Hexachlorobutadiene	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	Isopropylbenzene	μg/L	1/5	30	<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	n-butylbenzene	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	n-propylbenzene	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	p-isopropyltoluene	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	sec-butylbenzene	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	Styrene	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	Trichloroethene	μg/L	1/5	330	<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	tert-butylbenzene	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	Tetrachloroethene	μg/L	1/5	70	<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	Ethylbenzene	μg/L	1/5	80	<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	Toluene	μg/L	1/5	180	<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	trans-1,2-dichloroethene	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	trans-1,3-dichloropropene	μg/L	1/5		<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	Trichlorofluoromethane	μg/L	10/50		<10	<10	<10	<10	<50	<10	<10	<10	<10	<10	<10	<10
	Vinyl chloride	μg/L	10/50	100	<10	<10	<10	<10	<50	<10	<10	<10	<10	<10	<10	<10
	Xylene (m & p)	μg/L	2		<2	<2	<2	<2	<2	<2	<2	<2	<2	<2	<2	<2
	Xylene (o)	μg/L	1	350	<1	<1	<1	<1	<1	<1	<1	<1	<1	<1	<1	<1
j-	C6-C9	μg/L	10/20		<10	<10	<10	<10	<20	<10	<10	<10	<10	<10	<10	<10
XN in	C6-C10	μg/L	10/20		<10	<10	<10	<10	<20	<10	<10	<10	<10	<10	<10	<10
	C6-C10 (F1 minus BTEX)	μg/L	10/20		<10	<10	<10	<10	<20	<10	<10	<10	<10	<10	<10	<10
	Naphthalene	μg/L	1/5	16	<1	<1	<1	<1	<5	<1	<1	<1	<1	<1	<1	<1
	Benzene	μg/L	1	600	<1	<1	<1	<1	<1	<1	<1	<1	<1	<1	<1	<1
	Ethylbenzene	μg/L	1/2	80	<1	<1	<1	<1	<2	<1	<1	<1	<1	<1	<1	<1
	Toluene	μg/L	1/2	180	<1	<1	<1	<1	<2	<1	<1	<1	<1	<1	<1	<1
	Xylene (m & p)	μg/L	2		<2	<2	<2	<2	<2	<2	<2	<2	<2	<2	<2	<2
	Xylene (o)	μg/L	1/2	350	<1	<1	<1	<1	<1	<1	<1	<1	<1	<1	<1	<1

T1/400GW - triplicate of 4 D/SW- replicate of SW1



Table B2: Summary of Groundwater Levels and Field Parameters during Purging and Sampling (DP, 2023b)

Well ID	Date	Easting	Northing	Ground Level (AHD)	TOC Level (AHD)	Depth to GW (m TOC) ⁽³⁾	Depth to GW (m bgl) ⁽³⁾	Reduced Water Level (AHD) ⁽³⁾	PID Well Headspace (ppm) ⁽¹⁾	PID GW/SW Headspace (ppm) ⁽¹⁾	Depth of Floating Product (mm) (2)	рН	EC (μS/cm)	ORP (mV)	Turbidity (NTU)	Temp (°C)	DO (mg/L)	Volume Purged (L)	Comments	Sampling Method
	Groundwater Irrigation Bore																			
GW1	20/12/2022	384024	6355584	-	N/A	N/A	-	-	N/A	<1	-	7.0	243	172	3	19.4	8.08	10	Clear, grey	Sample from bore tap
	Groundwater Monitoring Wells																			
401	23/03/2023	384063.5	6355615.8	2.14	2.06	1.14	1.22	0.92	<1	<1	<1	6.8	642	-109	>1000	24.4	0.06	10	Very turbid, brown, no odour, no obvious contamination.	MicroPurge
	11/04/2023					0.96	1.04	1.10	<1	NT	<1	NT	NT	NT	NT	NT	NT	NT	Not sampled	Not sampled
402	23/03/2023	384012.3	6355646.5	2.20	2.26	1.19	1.24	1.07	<1	<1	<1	6.6	606	-6	>1000	21.5	0.79	10	Very turbid, pale brown, no odour, no obvious contamination.	MicroPurge
	11/04/2023					1.03	0.98	1.23	<1	NT	<1	NT	NT	NT	NT	NT	NT	NT	Not sampled	Not sampled
403	23/03/2023	383945.0	6355704.4	2.82	2.74	1.38	1.46	1.36	<1	1	<1	5.7	387	4	>1000	22.1	0.05	24	Very turbid, grey, slight sulfur odour, no obvious contamination.	MicroPurge
	11/04/2023					1.26	1.34	1.48	<1	NT	<1	NT	NT	NT	NT	NT	NT	NT	Not sampled	Not sampled
404	23/03/2023	383944.2	6355578.7	3.87	3.86	2.42	2.44	1.44	3	<1	<1	4.8	264	202	>1000	23.8	0.66	12	Very turbid, brown, no odour, no obvious contamination.	MicroPurge
	11/04/2023					2.26	2.28	1.60	<1	NT	<1	NT	NT	NT	NT	NT	NT	NT	Not sampled	Not sampled
405	23/03/2023	383885.8	6355637.9	4.00	3.97	2.47	2.50	1.50	<1	<1	<1	6.0	383	66	>1000	25.5	0.23	16	Very turbid, brown, no odour, no obvious contamination.	MicroPurge
	11/04/2023					2.34	2.37	1.63	<1	NT	<1	NT	NT	NT	NT	NT	NT	NT	Not sampled	Not sampled
406	23/03/2023	383777.0	6355597.0	3.98	4.05	2.30	2.37	1.75	<1	<1	<1	5.3	267	131	>1000	24.1	0.11	14	Very turbid, pale brown, no odour, no obvious contamination. Sand present in well.	MicroPurge
	11/04/2023					2.15	2.08	1.90	<1	NT	<1	NT	NT	NT	NT	NT	NT	NT	Not sampled	Not sampled
	<u>, </u>						-			Surface Water									-	-
SW1	30/03/2023	383892	6355753	-	-	-	-	-	-	<1	-	7.1	314	-25	1	22.9	6.92	-	Clear, grey, slight sulfur odour, no obvious contamination. Algae present, water flowing easterly.	Swing Sampler
SW2	30/03/2023	384074	6355650	-	-	-	-	-	-	<1	-	7.4	352	-43	6	25.4	9.60	-	Clear, pale yellow/straw, decomposing / organic odour, no obvious contamination. Algae present, water flowing easterly.	Swing Sampler

Appendix C

Figure C1: Summary of Groundwater Levels (2022-2023) (DP, 2023h)

Table C1: Summary of groundwater levels from all assessments

Figure F1 – Groundwater Level vs Rainfall (22 Nov 2023 to 16 Jan 2024) (Douglas, 2024a)





Well ID /	Date	Ground		Depth to GW	Depth to GW	Reduced Water Level	Event Type
Bore							
BH101	20/01/2021	з	Boren	ules _	7	0	
BH102	20/01/2021	3	_	_	56	-26	
BH103	20/01/2021	4	-	-	3.5	0.5	
BH104	20/01/2021	4	-	-	4	0	
BH105	20/01/2021	3	-	-	2.4	0.6	
BH106	20/01/2021	4	-	-	2	2	During drilling
BH107	20/01/2021	4	-	-	2.5	1.5	
BH108	20/01/2021	4	-	-	2.5	1.5	
BH109	20/01/2021	3	-	-	NE	<]	
BH110	20/01/2021	3	-	-	NE	<]	
BH111	20/01/2021	4	-	-	NE	<2	
	- /		DP Bores a	and CPTs	·		
Bore la	8/07/2022	3.2	-	-	NE	<2.1	During drilling.
Bore 5a	8/07/2022	4.1	-	-	NE 07	<1.9	
	14/07/2022	2.0	-	-	0.7	1.9	-
CPT 102	14/07/2022	2.5			10	23	
CPT 104	13/07/2022	41	_	_	1.0	2.5	Groundwater level after rods removed
CPT 105	14/07/2022	4.1	-	-	1.7	2.4	
CPT 106	13/07/2022	4.0	-	-	1.5	2.5	
CPT 107	13/07/2022	3.9	-	-	1.4	2.5	
		Groun	dwater monito	ring wells (401-	406)		
	9/03/2023			-	1.75	0.39	During drilling
	9/03/2023			1.07	1.15	0.99	Well development
	23/03/2023			1.14	1.22	0.92	Groundwater gauging, groundwater sampling
Well 401	11/04/2023	2.14	2.06	0.96	1.04	1.10	Groundwater gauging
	6/06/2023			0.92	0.92	1.14	Groundwater gauging, gas monitoring
	28/11/2023			1.05	1.13	1.01	Groundwater gauging
	16/1/2024			1.26	1.34	0.80	Groundwater gauging
	13/03/2023			-	1.60	0.60	During drilling
	16/03/2023			1.09	1.04	1.17	Well development
	23/03/2023			1.19	1.24	1.07	Groundwater gauging, groundwater sampling
Well 402	11/04/2023	2.20	2.26	1.03	0.98	1.23	Groundwater gauging
	6/06/2023			1.02	1.02	1.24	Groundwater gauging, gas monitoring
	22/11/2023			1.18	1.25	1.08	Groundwater gauging, logger download
	16/1/2024			1.27	1.32	0.98	Groundwater gauging, logger download
	13/03/2023			-	2.00	0.82	During drilling
	16/03/2023			1.41	1.49	1.33	Well development
	23/03/2023			1.38	1.46	1.36	Groundwater gauging, groundwater sampling
Well 403	11/04/2023	2.82	2.74	1.26	1.34	1.48	Groundwater gauging
	6/06/2023			1.25	1.25	1.48	Groundwater gauging, gas monitoring
	28/11/2023			1.38	1.46	1.35	Groundwater gauging
	16/1/2024			1.53	1.61	1.21	Groundwater gauging
	13/03/2023			-	2.80	1.07	During drilling
	16/03/2023			2.34	2.36	1.51	Well development
	23/03/2023			2.42	2.44	1.44	Groundwater gauging, groundwater sampling
Well 404	11/04/2023	3.87	3.86	2.26	2.28	1.60	Groundwater gauging
	6/06/2023			2.20	2.20	1.66	Groundwater gauging, gas monitoring
	22/11/2023			2.45	2.47	1.40	Groundwater gauging, logger installation
	28/11/2023			2.41	2.43	1.45	Groundwater gauging, logger download
	16/1/2024			2.55	2.57	1.30	Groundwater gauging, logger download
	10/03/2023			-	2.00	2.00	
	23/03/2023			2.44	2.47	1.55	Groupdwater gauging, groupdwater sampling
	11/04/2023			2.47	2.30	1.50	Groundwater gauging, groundwater sampling
Well 405	6/06/2023	4.00	3.97	2.36	2.36	1.61	Groundwater gauging, gas monitoring
	22/11/2023	1		2.59	2.56	1.41	Groundwater gauging, logger installation
	28/11/2023	1		2.53	2.50	1.47	Groundwater gauging, logger download
	16/1/2024	1		2.65	2.62	1.32	Groundwater gauging, logger download
	10/03/2023			-	2.00	1.98	During drilling
	10/03/2023			2.27	2.20	1.78	Well development
	23/03/2023			2.30	2.30 2.37 1.75 Grou 2.15 2.08 1.90 Grou		Groundwater gauging, groundwater sampling
Well 406	11/04/2023	3.98	4.05	2.15			Groundwater gauging
	6/06/2023			2.11	2.18	1.94	Groundwater gauging, gas monitoring
	28/11/2023			2.34	2.41	1.70	Groundwater gauging
	16/1/2024			2.50	2.57	1.55	Groundwater gauging
		Mi	nimum			0.80	Development, sampling or gauging event
		Ma	aximum			1.94	Development, sampling or gauging event

Note to Table: AHD – Australian Height Datum m bgl – metres below ground level

GW – groundwater TOC – top of PVC casing



Appendix D

Department of Planning and Environment (Water) – EIS Letter Response dated 9 August 2023

Department of Planning and Environment (Water) – EIS Letter Response dated 15 March 2024
Department of Planning and Environment



Our ref: OUT23/12629

Mr Patrick Andrade Planning and Assessment Group NSW Department of Planning and Environment

Email: patrick.andrade@dpie.nsw.gov.au

9 August 2023

Subject: Newcastle Education Campus (Newcastle City) (SSD-41814831) – Environmental Impact Statement (EIS)

Dear Mr Andrade,

I refer to your request for advice sent on 30 June 2023 to the Department of Planning and Environment (DPE) Water about the above matter.

The NSW Department of Education is seeking approval to upgrade the Newcastle Education Campus to provide improved facilities to meet the educational needs of students. The site is located at 25a National Park Street, Newcastle West.

DPE Water has reviewed the Environmental Impact Statement (EIS) and has recommendations regarding water take and licensing and ground water impacts. Please see Attachment A for more detail.

Should you have any further queries in relation to this submission please do not hesitate to contact DPE Water Assessments <u>water.assessments@dpie.nsw.gov.au</u>.

Yours sincerely

Simon Francis Senior Project Officer, Assessments, Knowledge Division Department of Planning and Environment: Water

Attachment A

Detailed advice to DPE Planning & Assessment regarding the Newcastle Education Campus (Newcastle City) (SSD-41814831)

1.0 Water Take and Licencing

1.1 Recommendation – Prior to Approval

The applicant should provide a dewatering management plan that details the maximum depth of excavation for construction and the maximum annual volume of water take due to aquifer interference activities required for the project.

Explanation

The EIS reports groundwater depths ranging between 2 m and 5.5 m below ground level, however the Desktop Review Soil and Water report (Appendix R) indicates groundwater was encountered at depths ranging between 0.5 m and 2.5 m.

Infrastructure works may require excavations >2m and intercept groundwater, and there is uncertainty as to the potential groundwater take. The proponent has committed to providing a dewatering management plan post determination. However, this should be provided prior to approval and should identify the maximum depth of excavations for construction and estimate the maximum annual volume of water take due to aquifer interference activities required for the project. It should also specifically nominate a drained method for the proposed development.

In preparing the dewatering management plan, the applicant should consider the *Guidelines for Groundwater Documentation for SSD/SSI Projects (2022)* and the *Minimum Requirements for Building Site Groundwater Investigations and Reporting (2022)* to ensure documentation is fit for purpose. These documents are available at:

- Guidelines for Groundwater Documentation for SSD/SSI Projects (2022)
 - <u>https://water.nsw.gov.au/ data/assets/pdf file/0020/507611/Guidelines-for-Groundwater-</u> <u>Documentation-for-SSD-SSI-Projects.pdf</u>
- Minimum Requirements for Building Site Groundwater Investigations and Reporting (2022)
 - <u>https://water.dpie.nsw.gov.au/__data/assets/pdf_file/0003/541605/minimum-requirements-</u> <u>for-building-site-groundwater-investigations-and-reporting.pdf</u>

1.2 Recommendation – Post Approval

The proponent must ensure sufficient water entitlement is held in a Water Access Licence (WAL)to account for the maximum predicted take for each water source prior to take occurring, unless an exemption applies.

Explanation

Under the *Water Management Act 2000,* a Water Access Licence (WAL) is required for water take above 3 ML/year. If the water take is less than or equal to 3 ML /year for any aquifer interference activities listed in Clause 7 of Schedule 4 of the *Water Management (General) Regulation 2018,* an exemption may apply. It is noted that this project includes excavation activities required for the construction of a building, road or infrastructure, which is item 2(c) in Clause 7 of the Water Management (General) Regulation 2018.

DPE Water notes that there are requirements for an exemption, such as:

- 1. the water is not taken for consumption or supply;
- 2. the person claiming the exemption keeps a record of the water taken under the exemption and provides this to the Minister within 28 days of the end of the water year; and
- 3. the records are kept for 5 years.

Further information on these requirements and other information on exemptions can be found on: <u>https://water.dpie.nsw.gov.au/licensing-and-trade/licensing/groundwater-wal-exemptions</u>. Please note that an exemption application form and a specific FAQ on 'WAL exemptions – 3ML or less of groundwater' is provided at this website as well as a form to report and record water take under an exemption.

End Attachment A



Department of Climate Change, Energy, the Environment and Water

Our ref: OUT24/3818

Dana Wilson Douglas Partners Email: Dana.Wilson@douglaspartners.com.au

15/03/2024

Subject: New castle Education Campus (SSD-41814831) - Dew atering Management Plan

Dear Ms Wilson

I refer to your request for advice sent on 20 February 2024 to the Department of Climate Change, Energy, the Environment and Water (DCCEEW) Water Group about the above matter.

DCCEEW Water has reviewed the DMP and advises that it is adequately fit for purpose. The DMP effectively describes the ground conditions, nature of the excavation activities, and methods and assumptions behind the estimated inflow volumes (predicted to be well below 3 ML/year).

Should you have any further queries in relation to this submission please do not hesitate to contact DCCEEW Water Assessments <u>water.assessments@dpie.nsw.gov.au</u>.

Yours sincerely

Simon Francis, Senior Project Officer, Water Assessments, Knowledge Division Department of Climate Change, Energy, the Environment and Water

Appendix E

Drawing 1 – Test Location Plan and Site Features (Douglas, 2023b)

Bulk Earthworks Plan (2023) – Stantec Australasia Pty Ltd

Staging Site Plan – EJE Architecture (Ref 13331, A-0030, Rev 0, Phase T, 21 April 2023)

Stantec (2023) Stormwater Drainage Plan (Sheets 1 to 4)

Hydraulic Services Site Plan (Ref 10344317 NEC-HDR-ZZ-XX-DR-H-0002, Issue P11, 21 April 2023)



0 10 20 30 40 m

Drawing adapted from Metromap image dated 11.06.2022. Test locations are approximate only and were located using Differential GPS.



CLIENT:	School Infrastru	cture NSW	TITLE:	Test Location Plan and Site Features
OFFICE:	Newcastle	DRAWN BY: PLH		Newcastle High School Upgrade
SCALE: 1	1,500 @ A3	DATE: 23.May.2023		25a National Park Street, Newcastle West



Site Location

Legend

Current Investigation Test Locations

- ACM Sample
- Surface Water Sample
- 🖶 Test Pit
- + Hand Auger
- Groundwater Monitoring Well
- + Hand Auger

🖶 Test Pit

- + Hand Augers / Hand Pits
- Surface Samples

Previous Test Locations

- Geotechnical Bore (DP, 2022b)
- ↔ Geotechnical CPT (DP, 2022b)
- Geotechnical Marten (2021)
- Investigation Area

Site Boundary

Lot Boundary

 Appoximate Boundary of Mapped Anthropogenic Deposits

PROJECT No: 213618.02 DRAWING No: 1 2 **REVISION**:



		SITE BOUNDARY
l l		BULK EARTHWORKS CONTOUR
)	18.0·	EXISTING CONTOURS
		APPROX GARDEN BED FOOTPRINT
	PAD RL	BULK EARTHWORKS LEVEL
$\langle \rangle$		
\leq		
\langle	COMPARISON BETWEEN T	HE DESIGN SURFACE AND THE
)	2. NOTE THAT ALL VOLUMES AND MAY NOT REFLECT D	DEPICTED ARE SOLID VOLUMES ONLY ETAILED EARTHWORKS.
	 NO ALLOWANCE HAS BEEN NO ALLOWANCE HAS BEEN 	N MADE FOR BULKING FACTORS. N MADE FOR DETAILED EARTHWORKS; id
\langle	RAINWATER TANK, SERVIC FOOTINGS, RETAINING WA	CE TRENCHING, DETAILED EXCAVATION, LLS, PAVEMENT BOXING, BUILDING
2	5. THE CONTRACTOR SHALL TYPICAL PAVEMENT DETA	USE FINAL SURFACE LEVELS AND ILS FOR ACTUAL EARTHWORKS LEVELS
	 BULK EARTHWORKS CUT/F 150mm TOPSOIL HA 	FILL VOLUME CONSIDERATIONS: S BEEN CONSIDERED TO BE REMOVED.
	- 400mm STRUCTURA - 300mm THICKNESS 200mm EOB LANDS	AL SLAB UNDER BUILDING PADS. FOR PAVEMENTS.
	- 500mm FOR GARDE - 1.365mm THICKNES	IN BEDS S FOR THE INFILTRATION SYSTEMS
5	7. THE SURVEY SURFACE AS COMPARISON PURPOSES.	PROVIDED HAS BEEN UTILISED FOR
\leq	8. STANTEC DOES NOT TAKE EXISTING SURVEY.	RESPONSIBILITY FOR ACCURACY OF
	9. BOLK EARTHWORKS DOES CONTAMINDATED MATERI/ WORKS	AL AND ANY REMEDIATION STRATEGY
ECH	10. SHOULD DETAILED EXCAV FOR BULK EARTHWORKS,	ATION AND TRENCHING SPOIL BE USED WE ANTICIPATE APPROX. ADDITIONAL
EM	3,000 cu.m OF CUT TO BE A	
2	CUT AND FILL V	DLUME:
	CUT: 4,342 m ³	
\leq	FILL: 4,607 m ³	$\langle \rangle$
)	NET: 265 m ³ (FI	LL)
$\langle \rangle$		
2	CUT/FILL DEPT	H RANGES
	COLOUR LOWER	UPPER
\leq	-2.75	-2.50
\langle	-2.50	-2.25
	-2.25	-2.00
\leq	-1.75	-1.50
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301350909 Revision Drawing No.

Project No.

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Scale 1:500

NEC-STNC-XX-XX-DR-C-100001

STAGING SITE PLAN - GROUND FLOOR 1400 STUDENTS (STREAM 9) + SUPPORT (8LS) **GROUND FLOOR**

STAGE 1

- Move Building H to new location as shown
- Services Infrastructure Upgrades
- Demolition of Building B and existing Sport Courts
- Remove trees, other planting in Stage 2 area

STAGE 2

- Construct New Learning Hub
- Landscaping walkway and external works associated with New Learning Hub
- New Support drop off zone

STAGE 3

- Construct new Multipurpose Facility
- Landscaping and external works associated with Multipurpose Facility
- Demolish Building P

STAGE 4

- Refurbish Building A & K
- Demolish Building J & existing walkwys to Building K
- Landscaping, site works

STAGE 5

- Demolish Buildings D, E & I
- Construct new Sports Courts
- Campus Green & remaining landscapes, walkways

STAGE 1

Demolition of Buildings, **Sports Court & other** items within Stage 2 **Build Area**



EJE ARCHITECTUREACN 002 912 843 | ABN 82 644 649 849Nominated Architect - Bernard CollinsP +61 2 4929 2353 | F +61 2 4926 3069 | E mail@eje.com.au | W www.eje.com.auA 412 KING STREET, NEWCASTLE, NSW 2300 COMPLETION OF THE QUALITY ASSURANCE CHECKS IS VERIFICATION THAT THE WHERE THE QUALITY ASSURANCE CHECK IS INCOMPLETE THIS DOCUMENT IS PRELIMINARY FOR INFORMATION PURPOSES ONLY, OR SUCH PURPOSES AS STATED IN THE REVISION COLUMN. THE IDEAS, INFORMATION AND CONCEPTS CONTAINED IN THIS DOCUMENT ARE THE

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REV	DATE	COMMENTS
F	12/07/2022	50% SCHEMATIC
G	02/08/2022	SCHEMATIC 75% ISSUE
Н	23/08/2022	SCHEMATIC 90% ISSUE
J	05/09/2022	STAGING PLAN REVISED
K	27/09/2022	PRELIMINARY SCHEMATIC 100% ISSUE
L	12/12/2022	25 % DETAILED DESIGN
М	16/01/2023	50 % DETAILED DESIGN
Ν	28/02/2023	100% DETAILED DESIGN
Р	03/04/2023	TENDER FOR REVIEW
Q	13/04/2023	TENDER ISSUE
0	21/04/2023	TENDER ISSUE





NEWCASTLE **EDUCATION CAMPUS**

SCHOOL INFRASTRUCTURE NSW

160/200 PARKWAY AVE HAMILTON SOUTH NSW 2303

STAGING SITE PLAN

SITE :

Autodesk Docs://Newcastle High School/NEC-EJE-ZZ-ZZ-M3-A-0001.rvt PROJECT No : 13331

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	Title STORMWATER DR SHEET 1	AINAGE PLAN -
CATION CAMPUS		
REET, NEWCASTLE WEST 2302	Project No. 301350909	Scale 1:250
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	Title STORMWATER DR SHEET 1	AINAGE PLAN -
CATION CAMPUS		
REET, NEWCASTLE WEST 2302	Project No. 301350909	Scale 1:250
001.DWG 2022.07.12 Dwn. Dsgn. Chkd. YYYY.MM.DD	Revision Drawing No. N NEC-STNC	C-XX-XX-DR-C-520001



B 75% SCHEMATIC DESIGN ISSUE

A 50% SCHEMATIC DESIGN ISSUE

Issued/Revision

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ISTING KERB INLET PIT
OPOSED DOWNPIPE
FILTRATION SYSTEM
INWATER TANK
AEP FLOOD EXTENT
OPOSED TREE. FER LANDSCAPE ARCHITECT.
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	P8 75% DETAILED DESIGN P9 100% DETAILED DESIGN	JG JG	JG JG	06.02.23 30.02.23	
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NEC-HDR-ZZ-XX-DR-H-0002

21.04.23 ISSUE

P11

Appendix F

Drawing 1 – Test Location Plan (Douglas, 2024a)

Drawing 2 – Groundwater Monitoring Well Location Plan (Douglas, 2024a)



NOTE:

1. Drawing from Metromap Image dated 15.10.2023.

2. Test locations are approximate only and were located using differential GPS



CLIENT:	Hansen Yunck	TITLE:	
OFFICE:	Newcastle	DRAWN BY: PLH	
SCALE:	1:500@A3	DATE: 06.December.2023	

Test Location Plan Newcastle High School Upgrade 25a National Park Street, Newcastle West, NSW

DP.QGIS.A3LandscapeDrawingLayout.Rev3 - P.\213618.04 - HAMILTON SOUTH Newcastle High Borrow Pit\7.0 Drawings\7.2 Out\213618.04.D.001.Rev0.qgz



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CLIENT: Hansen Yuncl	ken Pty Ltd	TITLE:	Groundwater Monitoring Well Loca
OFFICE: Newcastle	DRAWN BY: PLH		Newcastle High School Upgrade
SCALE: 1:1500 @A3	DATE: 18.January.2024		25a National Park Street, Newcastle

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