

Schematic Design Report

Cronulla High School

80820341



Prepared for
Schools Infrastructure NSW

15 July 2022

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1 Design criteria and standards

The design criteria are the minimum design standards and engineering guidelines to be used on the project. Guides, supplements and references

1.1 Design reference documents and engineering standards

The following design reference documents and engineering standards will be used in the development of the design:

- > Education Facilities Standards and Guidelines (EFSG)
- > Sutherland Shire Council Standards
- > Concept Design Report by Cardno dated 25 July 2021
- > Geotechnical investigation by Martens, report ref P2108205JR2V01 dated June 2021
- > Permeability Assessment Report by Martens, dated 10 June 2021
- > Relevant Australian and New Zealand Standards, including (but not necessarily limited to):
 - AS 2890.1 Off-Street Car Parking.
 - AS 1428 Design for Access and Mobility
 - AS/NZS 1170.0 – Structural Design Actions: General principles
 - AS/NZS 1170.1 – Structural Design Actions: Permanent, imposed and other actions
 - AS/NZS 1170.2 – Structural Design Actions: Wind actions
 - AS/NZS 1170.4 - Structural Design Actions: Earthquake actions
 - AS2159 – Piling Design and Installation
 - AS3600 – Concrete Structures
 - AS4100 – Steel Structures
 - AS3700 – Masonry Structures
 - AS1720 – Timber Structures
- > National Construction Code 2019 (NCC)
- > Asset Standards Authority (ASA) standards

1.2 Guides, plans and supplements

The following guides, plans and supplements will be used in the development of the detailed design:

- > Sutherland Shire Council Guideline- Stormwater Management – Sutherland Shire Environmental Specification 2009”

1.3 Order of precedence

The following order of precedence shall apply in the event of any inconsistency, ambiguity or discrepancy between the brief, Reference Documents and other standards:

- > Authority Standards
- > Australian/New Zealand Standards

2 General

2.1 Site Description

The Cronulla High School site is located on the corner of Captain Cook Drive and Bate Bay Road in Cronulla. The site encompasses an area of nearly 6ha. Cronulla Caringbah Sharks Junior Rugby League Football Club (JRLFC) is located across the eastern half of this site, whereas the school ground is located at the south western half of this site. The topography within this school is nearly flat and is mildly sloping from the south to the north towards the Woolloomare Bay with contour lines running from 5.5 to 5m. There is an embankment at the north of the main school area and the ground level drops to approximately RL2.3. A site location map is provided below in Figure 1.

Cronulla High School is located within the Sutherland Shire Local Government Area.

There are existing kerb outlets observed on Bates Bay Road and existing stormwater pipes running along Eulera Road.



Figure 1: Location map of Cronulla High School

2.2 Project Description

SINSW brief is to construct new building/s to accommodate 10 x New Permanent Teaching Spaces and an upgrade to existing staff and administration areas. Internal refurbishment to existing buildings to re-purpose some of the spaces where staff and administration spaces are removed will also be required.

The existing buildings are predominantly two-storey suspended slab on concrete columns, double brick walls with metal roof sheeting. Buildings B(h) F(f) and C(e) which are single storey double brick with metal roof sheeting.

An outline of the proposed development is provided below in Figure 2.



Figure 2: Proposed Development Plan (Source –Fulton Trotter Architects)

This report comprising of a stormwater management report, including a soil and water management plan has been prepared to support the REF application.

2.3 Civil Engineering

2.3.1 Existing information

2.3.1.1 List of Existing Information

The following existing information has been used:

- > Site Survey Plan by Total Survey Solutions
- > A site inspection carried out to identify locations of stormwater pits and stormwater discharge points.
- > Permeability assessment report by Martens.

2.3.1.2 Stormwater Disposal Provisions

The proposed Stormwater Management System has been designed in accordance with the following documents:

- > The Sutherland Shire Council guideline “Stormwater Management – Sutherland Shire Environmental Specification 2009”
- > Sutherland Shire Council Development Control Plan: 2015, Chapter 37 “Stormwater and Groundwater Management”
- > “Specifications for Civil Works Carried Out in Conjunction with Subdivisions and Developments” dated 2013.

2.3.1.3 Flooding and Overland Flow Provisions

The school site affected by minor flooding on the lower northern portion of the site for the 1% AEP event. We note that no buildings are proposed in this zone.

2.3.2 Stormwater

2.3.2.1 Stormwater collection

The stormwater drainage pipe system has been designed to collect all storm events in accordance with the AS 3500.3: 2021 and Council Guidelines as mentioned in Section 2.2.1.3.

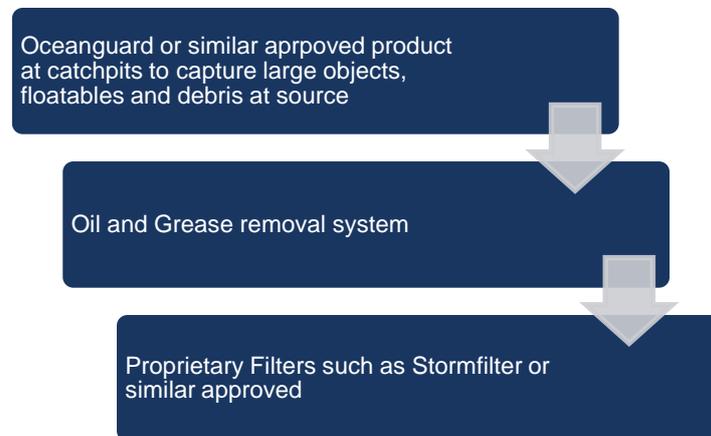
Roof water from the new development (i.e. Building L and Building M) along with runoff from the proposed new carparks are collected, quantity attenuated via an OSD and discharged to the existing public stormwater mains. The OSD will be designed to allow for stormwater discharge of runoffs of all the storms up to 100 years ARI. The controlled discharge will then be conveyed to a Council stormwater connection to an existing Kerb Inlet Pit (KIP) at Elouera Rd near the corner with Bate Bay Rd.

There is no increase in the impervious areas at the location of Building M, hence the roof and surface stormwater in the vicinity of Building M is discharged into the existing stormwater system.

A stormwater layout drawing for the proposed development is provided in Appendix A.

2.3.2.2 Stormwater quality

Water Sensitive Urban Design (WSUD) has been adopted in accordance with Sutherland Shire Council Stormwater Management 2009 requirements when designing the stormwater quality management system for this site. A treatment train system adopted for quality control is shown below



The table summarises pollutant removal targets from different sources to meet minimum Council and ESD requirements.

Pollutant	Performance Targets		
	Council's DCP % removal	ESD Requirements % removal	Achieved % removal
Gross Pollutants	90	90	90
Total Suspended Solids	70	80	80
Total Phosphorus	20	30	30
Total Nitrogen	35	30	35
Oil and Grease	No visible oils and grease in waterways	N/A	Yes

2.3.2.3 Soil and water management

Sedimentation and erosion measures for these proposed additions have been designed in accordance with the NSW Blue Book – Guideline for Managing Urban Stormwater: Soils and Construction – Volume 1 contain any sediment laden runoff from the site and minimise any impacts to the receiving environment. The proposed additions including both the buildings and car parks will be surrounded by silt fences and silt socks will be installed around existing catchpits. Designated entrance and exit with sediment protection measures will be provided for the construction vehicles.

A Soil and Water Management Plan (SWMP) is available in Appendix B.

2.4 Structural Engineering

2.4.1 Existing Information

2.4.1.1 List of existing information.

The following existing information has been used:

- > A site inspection was carried to assess the structural configuration and condition of existing buildings.
- > Geotechnical investigation report by Martens

2.4.1.2 Review of existing information

The buildings on site are generally in a good condition.

The geotechnical investigation report indicates that the area is underlain by alluvial deposits comprising of marine sands. The sand is expected to be of variable relative density and generally very loose to loose at shallow depths increasing to median dense to dense at depths of about 4m to 6m below ground level.

2.4.2 Schematic design

2.4.2.1 Design loads

Design loads have been derived as follows:

2.4.2.1.1 Live loads

- > Classrooms: 3kPa
- > Corridors, hallways and stairs: 4.0kPa

2.4.2.1.2 Wind loads

- > Importance level: 3
- > Annual probability of exceedance: 1:1000
- > V_{1000} : 46m/s
- > Terrain Category: 3

2.4.2.1.3 Earthquake loads

- > Importance Level: 3
- > Annual probability of exceedance: 1:1000
- > Probability Factor k_p : 1.3
- > Hazard Factor Z : 0.09
- > Site Sub-Soil Class: De
- > Earthquake Design Category: II
- > Structural System: Ordinary moment resisting frames
- > Structural Ductility Factor μ : 2
- > Structural Performance Factor S_p : 0.77
- > S_p/μ : 0.38; μ/S_p 2.6

2.4.3 Structural concept

2.4.3.1 *New buildings – Blocks L and M*

Structural schematic design for new buildings L and M are attached in Appendix C

The ground floor is designed as a waffle raft slab in accordance with the principles of AS2870 for a Class P site. Screwed piles founded in the dense sands are provided at every third rib. All columns are also provided with screwed piles to the dense sand.

The first floor is designed as a one-way post-tensioned slab on beams. Lateral stability is provided with frame action.

The roof is steel framed and designed as a portal frame in the transverse direction and fully braced with diagonal bracing to the first-floor slab in the longitudinal direction.

Reinforced brick walls are used where required to meet Australian Standards for robustness and strength for vertical loads and horizontal wind loads.

2.4.3.2 *Alterations to existing buildings.*

Except for demolition of a column on the first-floor level of Building D, the alterations to existing buildings generally include demolition of non-load bearing walls. These works do not affect the structural integrity of the existing buildings.

For Building D, a new steel beam has been designed to provide support to existing roof beams impacted by the demolition of the first-floor column.

2.4.3.3 *Building I*

Building I is a demountable building that is proposed to be re-located. We understand that the building was constructed in 2010.

An inspection of the exposed sub-floor framing indicates the following:

- > Building I is approximately 27m long x 9m wide with a 2m wide awning and ramp entry.
- > The building appears to have been joined together from 9, 3m wide sections.
- > The footings appear to be stacked concrete blocks with stacked block piers placed at 3m x 3m grids to the underside of the floor framing.
- > The floor framing comprises of 200C purlin sections acting as joists at about 550mm centres and spanning onto 200C purlins sections acting as bearers.
- > The 200C purlins joists are welded to the 200C bearers with welds at the flanges only.
- > There are various areas of corrosion to the 200C purlins.

The following works are required:

- > All corroded areas of the sub-floor framing shall be removed and replaced.
- > The flange welded connections are not structurally adequate. The connections shall be rectified using GP brackets and M12 purlin bolts.
- > Screwed pile footings shall be provided at the proposed location.
- > Steel bridging beams are to be provided to span across an existing stormwater line.

APPENDIX

A

STORMWATER DISPOSAL PLAN



now



CIVIL WORKS

NSW DEPARTMENT OF EDUCATION

CRONULLA HIGH SCHOOL CAPTAIN COOK DRIVE, CRONULLA NSW

COVER SHEET, LOCALITY PLAN AND SCHEDULE OF DRAWINGS

SCHEDULE OF DRAWINGS	
DRAWING No.	DESCRIPTION
CIVIL WORKS	
80821341-CI-0001	CIVIL COVER SHEET, LOCALITY PLAN AND SCHEDULE OF DRAWINGS
80821341-CI-0002	CIVIL CONSTRUCTION NOTES
80821341-CI-0101	CIVIL SITE AND STORMWATER DRAINAGE PLAN SHEET 1
80821341-CI-0102	CIVIL SITE AND STORMWATER DRAINAGE PLAN SHEET 2
80821341-CI-0103	CIVIL OSD TANK SECTIONS AND DETAILS
80821341-CI-0105	CIVIL SEDIMENTATION AND EROSION CONTROL PLAN SHEET 1
80821341-CI-0106	CIVIL SEDIMENTATION AND EROSION CONTROL PLAN SHEET 2
80821341-CI-0109	CIVIL CAR PARK PAVEMENT PLAN AND DETAILS
80821341-CI-0110	CIVIL SEDIMENTATION AND EROSION CONTROL DETAILS
80821341-CI-0111	CIVIL STORMWATER STANDARD DETAILS SHEET 1
80821341-CI-0112	CIVIL STORMWATER STANDARD DETAILS SHEET 2

INDICATIVE AREA
 OF WORKS



LOCALITY PLAN

DATE PLOTTED: 4 July 2022 6:00 PM BY: GORAN MILENKOVIC

XREFS: CAD File: \\cardno\opg\p\ba\au\sw\Directory\Structure\Projects\80821341-CI-0001.dwg

Rev.	Date	Description	Des.	Verif.	Appd.
1	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP

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Datum	AHD	Scale	NTS
Size	A1	Drawing Number	80821341-CI-0001
Revision	1		

CIVIL CONSTRUCTION NOTES

GENERAL NOTES

- G1. THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER CONSULTANTS DRAWINGS AND SPECIFICATIONS AND WITH SUCH OTHER WRITTEN INSTRUCTIONS THAT MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ANY DISCREPANCIES IN THESE DOCUMENTS SHALL BE REFERRED TO THE SUPERINTENDENT FOR A DECISION BEFORE PROCEEDING WITH THE WORK.
- G2. THE CONTRACTOR SHALL CHECK AND BE RESPONSIBLE FOR THE CORRECTNESS OF ALL DIMENSIONS AND ANY DISCREPANCY SHALL BE REPORTED IMMEDIATELY TO THE SUPERINTENDENT. DIMENSIONS SHALL NOT BE OBTAINED BY SCALING FROM THE DRAWINGS.
- G3. STABILITY OF THE BUILDING DURING CONSTRUCTION AND EXCAVATION IN THE VICINITY OF ADJACENT BUILDINGS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. NO PART OF THE STRUCTURE SHALL BE OVER STRESSED. APPROVAL OF ALL PROPOSALS MUST BE GRANTED BY THE ARCHITECT PRIOR TO THE COMMENCEMENT OF WORK.
- G4. THE CONTRACTOR SHALL NOTIFY THE ENGINEER FORTY EIGHT (48) HOURS BEFORE THE REINFORCEMENT IS COMPLETED. THE CONTRACTOR SHALL ALLOW TWO (2) HOURS AFTER THE COMPLETION OF THE REINFORCEMENT FOR THE ENGINEER'S INSPECTION. CONCRETE SHALL NOT BE ORDERED UNTIL THE REINFORCEMENT IS APPROVED BY THE ENGINEER.
- G5. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT SAA CODES, THE BY-LAWS AND ORDINANCES OF THE RELEVANT BUILDING AUTHORITY AND THE SPECIFICATION.
- G6. NO CHANGES SHALL BE MADE WITHOUT THE WRITTEN CONSENT OF THE ENGINEER.
- G7. U.N.O. DENOTES UNLESS NOTED OTHERWISE ON THE DRAWINGS.

SITE PREPARATION

ROAD WORKS

- SP1. REMOVE TOP SOIL, ROOT AFFECTED SOIL, FILL AND OTHER DELETERIOUS MATERIAL TO EXPOSE NATURAL SUBGRADE.
- SP2. THE EXPOSED SUBGRADE SHOULD THEN BE PROOF ROLLED WITH AT LEAST EIGHT (8) PASSES OF A 10 TONNE MIN. DEAD WEIGHT ROLLER. ANY SOFT OR HEAVING AREAS SHOULD BE REMOVED TO A MAXIMUM DEPTH OF 300mm AND REPLACED WITH CLEAN WELL GRADED MATERIAL SUCH AS RIPPED OR CRUSHED SANDSTONE COMPACTED TO AT LEAST 100% OF STANDARD MAXIMUM DRY DENSITY (SMDD) AT ±2% OPTIMUM MOISTURE CONTENT IN ACCORDANCE WITH AS1289.
- SP3. COMPACTED FILL SHOULD BE PLACED IN LAYERS NOT EXCEEDING 150mm THICK AND COMPACTED TO AT LEAST 100% SMDD. FILL SHALL CONSIST OF CLEAN WELL GRADED MATERIAL SUCH AS RIPPED OR CRUSHED SANDSTONE WITH A MIN. CBR OF 15%.
- SP4. DENSITY TESTING SHALL BE CARRIED OUT TO LEVEL 2 CERTIFICATION IN ACCORDANCE WITH AS3798.
- SP5. SUB-BASE COURSE - TO BE DGS20 OR DGS40 OR RIPPED OR CRUSHED SANDSTONE WITH A CBR GREATER THAN 40%, MAXIMUM PARTICLE SIZE OF 60mm, WELL GRADED WITH A PLASTIC INDEX LESS THAN 10. COMPACT TO AN AVERAGE OF NOT LESS THAN 100% SMDD WITH A MINIMUM VALUE OF 98% SMDD.
- SP6. ALL KERBS TO BE FORMED BY KERB MACHINE AND NOT BY HAND. EXTERNAL FOOTPATHS/PAVEMENTS
- SP7. REMOVE TOP SOIL, ROOT AFFECTED SOIL, FILL AND OTHER DELETERIOUS MATERIAL TO EXPOSE NATURAL SUBGRADE.
- SP8. THE EXPOSED SUBGRADE SHOULD BE PROOF ROLLED. ANY SOFT OR HEAVING AREAS SHOULD BE REPLACED WITH CLEAN WELL GRADED MATERIAL. FILL IF REQUIRED, SHOULD BE CLEAN AND WELL GRADED. COMPACT TO 100% SMDD.

STORMWATER CONSTRUCTION NOTES

- SW1. ESTABLISH EXACT LOCATION AND INVERT OF EXISTING SERVICES PRIOR TO COMMENCING WORKS.
- SW2. ALLOW TO PAY ALL LOCAL AUTHORITY FEES AS REQUIRED FOR PERIODIC INSPECTIONS/APPROVALS.
- SW3. ALL WORK TO BE IN ACCORDANCE WITH THE LOCAL COUNCIL STORMWATER POLICY.
- SW4. MINOR FLOW ARI = 10 YEARS
MAJOR FLOW ARI = 100 YEARS

PIPEWORK

- SW5. ALL WORKS TO COMPLY TO AS3500
- SW6. ALL NEW PIPES 300 DIA. & LARGER SHALL BE REINFORCED CONCRETE CLASS 2 WITH RUBBER RING JOINTS. WHERE ANGLED THE MINIMUM RADIUS SHALL BE 152m. 150/225mm DIA. STORMWATER PIPES SHALL BE SEWER CLASS UPVC. MINIMUM PIPE SIZE FOR AN UNDERGROUND PIPE TO BE 150mm. MINIMUM PIPE GRADIENTS 1% U.N.O.
- SW7. ALL DRAINAGE TRENCHES SHALL BE IN SOUND EXCAVATED MATERIAL. IF SOFT SPOTS EXIST, REMOVE AND BACKFILL WITH COMPACTED ROAD BASE (DGS40) WITH A MINIMUM CBR OF 25 COMPACT TO 98% STANDARD MAXIMUM DRY DENSITY TO AS 1289 E1.1.
- SW8. ALL PIPES SHALL BE BEDDED ON 100mm SAND BED AND BACKFILLED WITH SAND TO 150mm ABOVE BARREL OF PIPE. THE REMAINDER OF THE TRENCH WILL BE BACKFILLED IN 150mm COMPACTED LAYERS IN GRANULAR FILL NON DISPERSIVE (EMERSON CLASS 5 OR 6) MATERIAL - NO TOP SOIL, GRASS, ROOTS, OR DELETERIOUS MATERIAL. COMPACT TO 98% STANDARD MAXIMUM DRY DENSITY AT ±2% OMC.
- SW9. PROVIDE A 100mm DIA. UPVC. SLOTTED DRAINAGE PIPE 3000 LONG WRAPPED IN FILTER FABRIC SOCK IN ALL TRENCHES ADJACENT TO INLET PIPES TO PITS & CONNECTED TO PIT.
- SW10. OTHER SUB SOIL DRAINAGE PIPES SHALL BE 100mm DIA. UPVC SLOTTED BEDDED AND BACKFILLED WITH 20mm GAUGE BLUE METAL. CLEAN OUTS SHALL BE EXTENDED TO THE SURFACE AND PROVIDED WITH A SCREWED COVER PLATE FLUSH WITH THE FINISHED SURFACE LEVEL.
- SW11. ALL CONCRETE PITS CONSTRUCTED SHALL BE BEDDED AS PER PIPE SPECIFICATION. PIT BASES SHALL BE SMOOTH CONTOURED WITH MASS CONCRETE BENCHING PROVIDE STEP IRONS AS PER PIT SCHEDULE. PROVIDE HEAVY, MEDIUM OR LIGHT DUTY GALVANISED GRATE COVERS AS SPECIFIED IN SCHEDULE. ALL DRAINAGE WORKS ARE TO BE COMPLETED TO THE SATISFACTION OF THE SUPERVISING CIVIL ENGINEER. ALL WORKS TO COMPLY TO AS3500. HEADWALLS SHALL BE PRECAST CONCRETE BY 'BCP PRECAST' OR EQUAL. ALL PITS SHALL BE PRECAST CONCRETE TYPE DPT FOR DEEP PITS BY ICON INDUSTRIES OR EQUAL. PRECAST DRAINAGE PITS DEEPER THAN 1800mm SHALL HAVE 150mm MIN. WALL THICKNESS. 20mm DIA. GALV. MS. STEP IRONS SHALL BE INSTALLED IN PITS 1200mm AND DEEPER.
- SW12. UNLESS NOTED OTHERWISE ON THE PLANS, PROVIDE THE FOLLOWING MIN. COVER TO PIPE:
- UNDER LANDSCAPE & PAVEMENT - 300mm
- UNDER ROAD (TRAFFIC) - 600mm

CONCRETE

- C1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE CURRENT SAA CODE AS3600, WITH AMENDMENTS, EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.

CONCRETE QUALITY:

ELEMENT	SLUMP (MAX)	MAX AGG SIZE	CEMENT TYPE	ADMIX.	CONCRETE GRADE
CONCRETE DRIVEWAYS	60	20mm	A	N/A	40 MPa
CONCRETE KERBS, RETAINING WALLS	80	20mm	A	N/A	32 MPa
FOOTPATHS	80	20mm	A	N/A	25 MPa
PIERS	80	20mm	A	N/A	25 MPa

SUSPENDED CONCRETE FLOORS -
MIN. CEMENT CONTENT = 300 kg/m³
MAX. PERMISSIBLE DRYING SHRINKAGE = 750 MICROSTRAIN AT 56 DAYS.

- C3. CLEAR CONCRETE COVER IN mm TO REINFORCEMENT U.N.O. SHALL BE AS FOLLOWS:-

STRUCTURAL ELEMENT	REINFORCEMENT COVER			
	INTERNAL		EXTERNAL	
	TOP	BTM.	TOP	BTM.
FOOTINGS & PIERS	-	-	50	50
DRAINAGE PITS	-	-	50	50
CONCRETE DRIVEWAY	-	-	40	40

NOTES:

- 1. CONCRETE POURED OVER A MEMBRANE ON THE GROUND IS INCLUDED AS INTERNAL.
- 2. CONCRETE EXPOSED TO CORROSIVE VAPOURS, CORROSIVE GROUND WATER, SEA WATER OR SPRAY IS TO HAVE REINFORCEMENT COVER AS NOTED ON THE DRAWINGS.
- 3. CONCRETE REQUIRING A FIRE RESISTANCE RATING SHALL HAVE REINFORCEMENT COVER AS NOTED ON THE DRAWINGS.
- 4. EXTERNAL ABOVE GROUND ELEMENTS ARE CLASSIFIED IN NEAR COASTAL ENVIRONMENT. CONDUITS, PIPES, ETC., SHALL NOT BE PLACED IN THE CONCRETE COVER TO REINFORCEMENT AND NO HOLES OR CHASES OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ALLOWED WITHOUT THE PRIOR APPROVAL OF THE SUPERINTENDENT.
- C5. CONCRETE SIZES DO NOT INCLUDE THE THICKNESS OF APPLIED FINISHES.
- C6. THE DEPTH OF BEAMS IS GIVEN FIRST AND INCLUDES THE SLAB THICKNESS.
- C7. CONSTRUCTION JOINTS, WHERE NOT SHOWN, SHALL BE LOCATED TO THE APPROVAL OF THE SUPERINTENDENT.
- C8. FORMWORK SHALL REMAIN IN POSITION FOR THE TIME SPECIFIED. WHERE SLABS AND BEAMS ARE TO SUPPORT MASONRY OVER, FORMWORK AND PROPS MUST BE REMOVED PRIOR TO THE CONSTRUCTION OF MASONRY.
- C9. ALL CONCRETE SHALL BE MECHANICALLY VIBRATED. THE VIBRATOR SHALL NOT BE USED TO SPREAD CONCRETE.
- C10. CONCRETE SHALL BE CURED IN ACCORDANCE WITH AS3600 WITH A PRODUCT COMPATIBLE WITH THE APPLIED FINISHES. CURING COMPOUNDS SHALL COMPLY WITH AS3799. PVA BASED CURING COMPOUNDS ARE NOT ACCEPTABLE.
- C11. REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY, IT IS NOT NECESSARILY SHOWN IN TRUE PROJECTION.
- C12. WELDING OF REINFORCEMENT SHALL NOT BE PERMITTED UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.
- C13. SPLICES IN THE MAIN REINFORCEMENT SHALL BE MADE ONLY IN THE POSITIONS SHOWN. SPLICES IN THE DISTRIBUTION REINFORCEMENT MAY BE POSITIONED AS NECESSARY WITH SPLICES OF SUFFICIENT LENGTH TO DEVELOP THE FULL STRENGTH OF THE BARS. MINIMUM LAPS TO FABRIC SHALL BE TO OVER LAP TWO CROSS WIRES PLUS 50mm U.N.O. REINFORCEMENT SHALL BE SECURELY TIED AT ALL LAPS AND INTERSECTIONS WITH 1.25mm BLACK ANNEALED WIRE. THE WRITTEN APPROVAL OF THE SUPERINTENDENT SHALL BE OBTAINED FOR OTHER SPLICES WHERE THE LAP LENGTH IS NOT SHOWN. IT SHALL DEVELOP THE FULL STRENGTH OF THE REINFORCEMENT.
- C14. ALL UNSUPPORTED BARS SHALL BE TIED IN A TRANSVERSE DIRECTION WITH N12-300 U.N.O.
- C15. REINFORCEMENT SHALL BE SUPPORTED ON APPROVED PLASTIC OR PLASTIC TIPPED WIRE STOOLS AT NOT MORE THAN 600mm CENTRES BOTHWAYS IN SLABS AND AT 1000mm CENTRES IN BEAMS.

C16.  DENOTES MAIN WIRES OF RECTANGULAR FABRIC TO AS 4671.

 DENOTES SQUARE FABRIC TO AS 4671

SL..... DENOTES GRADE 500 DEFORMED WIRE REINFORCING SQUARE FABRIC OF DUCTILITY CLASS L TO AS 4671.

RL..... DENOTES GRADE 500 DEFORMED WIRE REINFORCING RECTANGULAR FABRIC OF DUCTILITY CLASS L TO AS 4671.

R DENOTES GRADE 250 ROUND BARS OF DUCTILITY CLASS N TO AS 4671.

N DENOTES GRADE 500 DEFORMED BARS OF DUCTILITY CLASS N TO AS 4671.

S DENOTES GRADE 250 DEFORMED BARS OF DUCTILITY CLASS N TO AS 4671.

- C17. FABRIC SHALL BE SUPPLIED IN FLAT SHEETS, ROLLS WILL NOT BE ACCEPTED

TYPICAL REINFORCEMENT NOTATION:-

- 5N24-200 INDICATES
5 DENOTES NUMBER OF BARS REQUIRED
N DENOTES GRADE OF REINFORCEMENT
24 DENOTES BAR DIAMETER IN MILLIMETRES
200... DENOTES BAR SPACING IN MILLIMETRES

TYPICAL ABBREVIATIONS:-

- B DENOTES BARS IN BOTTOM LAYER
- T DENOTES BARS IN TOP LAYER
- ALT... DENOTES BARS ALTERNATING
- NF DENOTES BARS IN NEAR FACE
- FF DENOTES BARS IN FAR FACE
- EF DENOTES BARS IN EACH FACE

- C19. FOR SLAB FALLS, CHAMFERS, REGLETS, DRIP GROOVES, ETC., REFER TO THE ARCHITECT'S DRAWINGS.

- C20. LAP LENGTHS FOR DEFORMED BARS AS FOLLOWS:

BAR TYPE AND SIZE	VERTICAL BARS	HORIZONTAL BARS WITH MORE THAN 300mm OF CONCRETE BELOW BAR	OTHER LOCATIONS	90° COG LENGTH
N12	500	550	500	200
N16	700	800	700	200
N20	1000	1250	1000	250
N24	1200	1500	1200	300
N28	1400	1750	1400	350

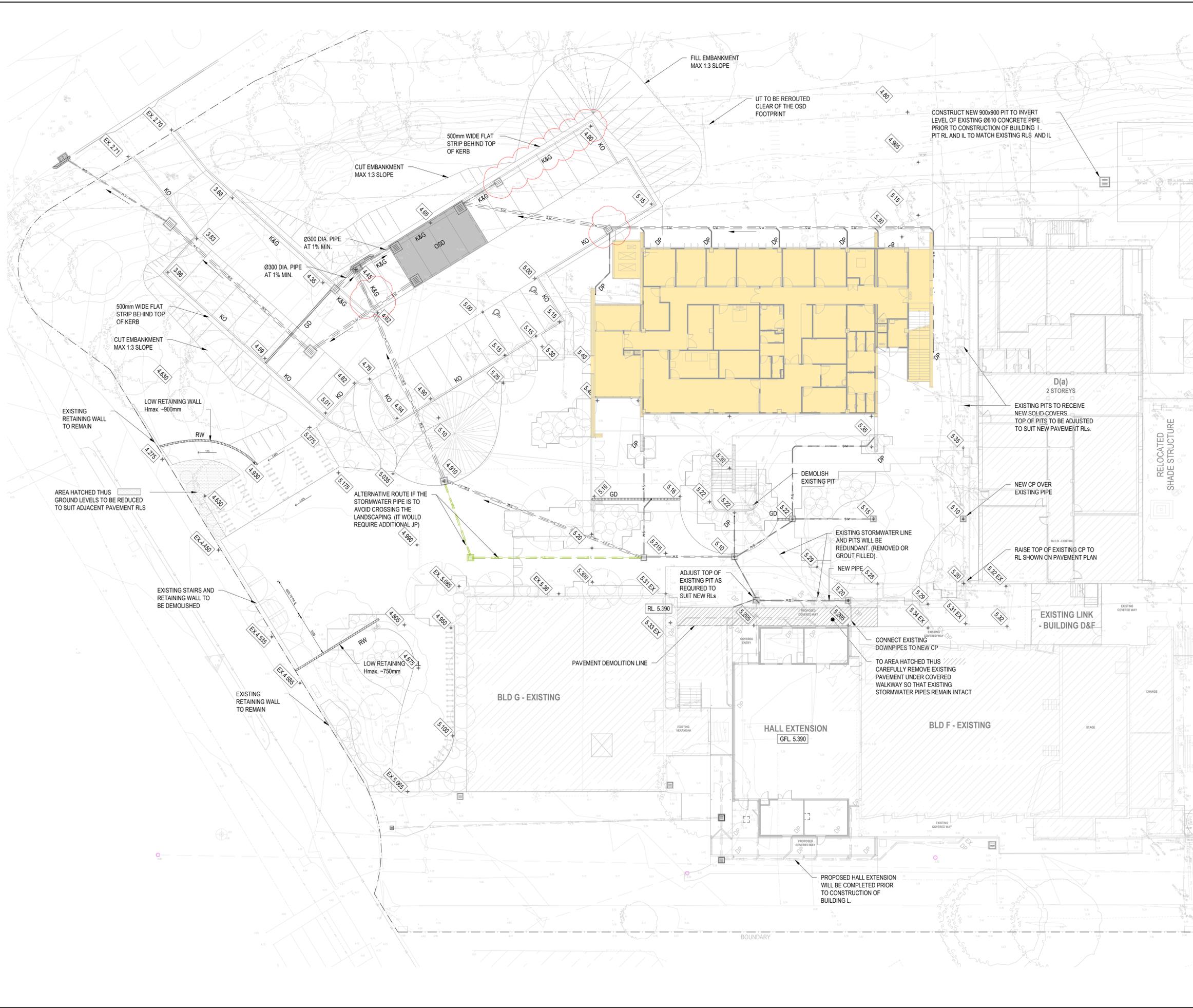
SEDIMENT RUN-OFF CONTROL NOTES

- SR1. THE CONTRACTOR SHALL INSTALL AND MAINTAIN SOIL EROSION AND SEDIMENT CONTROL MEASURES GENERALLY IN ACCORDANCE WITH GUIDELINES OF THE LANDCOM MANAGING URBAN STORMWATER MANUAL AND AS NECESSARY TO PREVENT RUN-OFF FROM SITE OF SEDIMENT RESULTING FROM THE WORKS. SUCH MEASURES SHALL ALSO COMPLY WITH REQUIREMENTS OF COUNCIL, LANDCOM "BLUE BOOK" AND EPA. THIS WORK SHALL BE DONE PRIOR TO ANY EARTHWORKS COMMENCING ON SITE.
- SR2. GRADE FINISHED SURFACE TO SHED WATER EVENLY WITHOUT CHANNELLING (UNTIL PIPED STORMWATER SYSTEM IS CONSTRUCTED). NOMINAL GRADIENTS FROM HIGH POINT OF 0.2%.
- SR3. MAINTAIN THE EROSION CONTROL DEVICES INDICATED ON THE DRAWINGS TO THE SATISFACTION OF THE SITE SUPERINTENDENT AND THE LOCAL AUTHORITIES.
- SR4. WHEN PROPOSED STORMWATER PITS ARE CONSTRUCTED, PREVENT SITE RUNOFF ENTERING UNLESS SILT FENCES ARE ERECTED AROUND PITS AND ON ROAD.
- SR5. STREET PROTECTION WITH SHAKER EXIT GRIDS & STREET PIT INLET PROTECTION TO BE MAINTAINED FOR THE DURATION OF THE CONTRACT.

WARNING

UNLESS NOTIFIED TO THE CONTRARY IN WRITING, THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR ANY BREACHES OF THE PROTECTION OF ENVIRONMENT OPERATIONS ACT 1997. PLEASE NOTE FAILURE TO IMPLEMENT OR MAINTAIN APPROPRIATE EROSION/SEDIMENT CONTROL MEASURES IS A BREACH OF THE ACT. SUCH A BREACH IS LIABLE FOR A ON-THE SPOT FINE AND/OR PENALTY.

1	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
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Checked	MR	Date	MAY 2022		
Designed	VJ	Date	MAY 2022		
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Client NSW DEPARTMENT OF EDUCATION					
Project CRONULLA HIGH SCHOOL CAPTAIN COOK DRIVE, CRONULLA NSW					
Title CIVIL CONSTRUCTION NOTES					
Status SCHEMATIC DESIGN NOT TO BE USED FOR CONSTRUCTION PURPOSES					
Datum	AHD	Scale	N/A	Size	A1
Drawing Number					Revision
80821341-CI-0002					1



ALL PITS TO BE FITTED WITH WITH OCEANGUARD BASKET OR APPROVED EQUIVALENT

THE RL'S IN THE VICINITY OF THE HALL EXTENSION SHOWN AS EXISTING, ARE BASED ON THE ASSUMPTION THAT THE DESIGN RL'S OF THE HALL EXTENSION PROJECT HAVE BEEN ACHIEVED - TO BE CONFIRMED ON SITE.

LEGEND EXISTING:

- EXISTING CONTOURS
- EXISTING TELECOMMUNICATIONS (SURVEY)
- EXISTING ELECTRICITY (SURVEY)
- EXISTING GAS (SURVEY)
- EXISTING WATER MAIN
- EXISTING SEWER
- EXISTING OPTICAL FIBRE
- EXISTING UNKNOWN SERVICE
- EXISTING STORMWATER LINE
- EXISTING FIRE HYDRANT
- EXISTING DOOR / GATE
- EXISTING SURFACE AMENITIES / SERVICES
- EXISTING BOLLARDS
- EXISTING STORMWATER PIT

LEGEND PROPOSED:

- PROPOSED GRATED SURFACE INLET PIT
- PROPOSED SUBSOIL DRAINAGE
- ALTERNATIVE STORMWATER LINE
- PROPOSED STORMWATER LINE
- EXISTING STORMWATER/DOWNSPIPE LINE
- PROPOSED DOWN PIPE
- EXISTING DOWN PIPE

FOR CONTINUATION REFER TO DRAWING CI-0102

Rev.	Date	Description	Des.	Verif.	Appd.
4	14/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
3	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
2	01/07/2022	ISSUED FOR COORDINATION	VJ	VJ	PP
1	09/06/2022	ISSUED FOR COORDINATION	VJ	VJ	PP

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Verified	PP	Date	MAY 2022	
Approved		Date	MAY 2022	

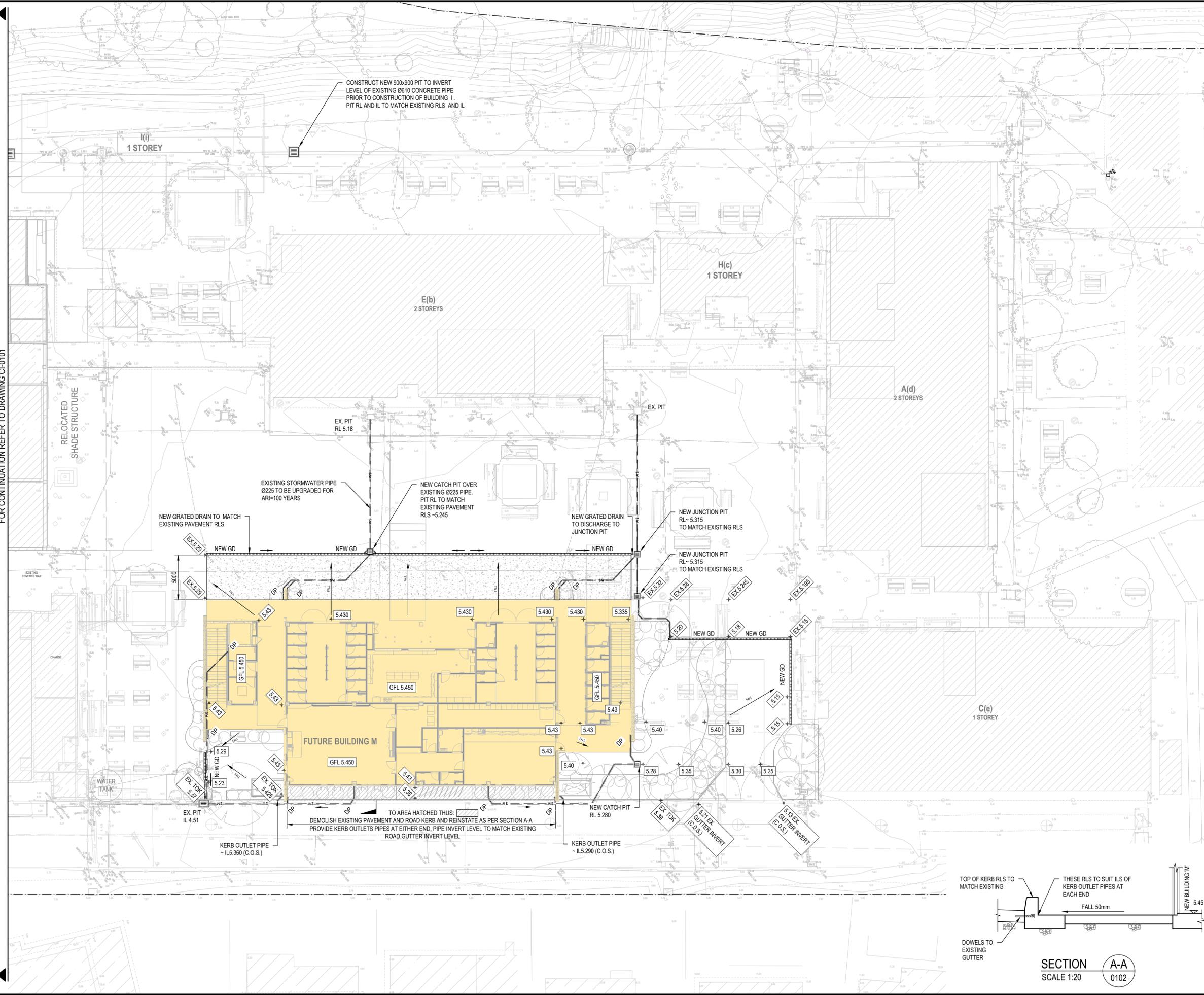
Client: **NSW DEPARTMENT OF EDUCATION**

Project: **CRONULLA HIGH SCHOOL
 CAPTAIN COOK DRIVE, CRONULLA NSW**

Title: **CIVIL
 SITE AND STORMWATER DRAINAGE PLAN
 SHEET 1**

Status: **SCHEMATIC DESIGN
 NOT TO BE USED FOR CONSTRUCTION PURPOSES**

Datum	AHD	Scale	1:200	Size	A1
Drawing Number	80821341-CI-0101			Revision	4



LEGEND EXISTING:

- EXISTING CONTOURS
- UT EXISTING TELECOMMUNICATIONS (SURVEY)
- UE EXISTING ELECTRICITY (SURVEY)
- UG EXISTING GAS (SURVEY)
- UM EXISTING WATER MAIN
- US EXISTING SEWER
- UOF EXISTING OPTICAL FIBRE
- UU EXISTING UNKNOWN SERVICE
- USW EXISTING STORMWATER LINE
- UPH EXISTING FIRE HYDRANT

— EXISTING DOOR / GATE

— EXISTING SURFACE AMENITIES / SERVICES

— EXISTING BOLLARDS

— EXISTING STORMWATER PIT

LEGEND PROPOSED:

- PROPOSED GRATED SURFACE INLET PIT
- SS PROPOSED SUBSOIL DRAINAGE
- SW ALTERNATIVE STORMWATER LINE
- SW PROPOSED STORMWATER LINE
- SW EXISTING STORMWATER/DOWNSPIPE LINE
- DP PROPOSED DOWN PIPE
- DP EXISTING DOWN PIPE

Rev.	Date	Description	Des.	Verif.	Appd.
4	14/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
3	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
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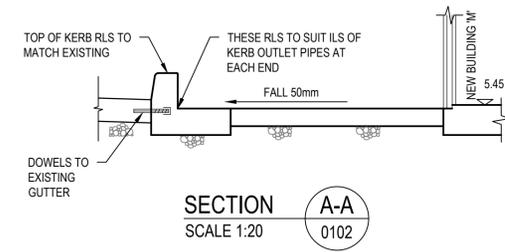
Client: **NSW DEPARTMENT OF EDUCATION**

Project: **CRONULLA HIGH SCHOOL
 CAPTAIN COOK DRIVE, CRONULLA NSW**

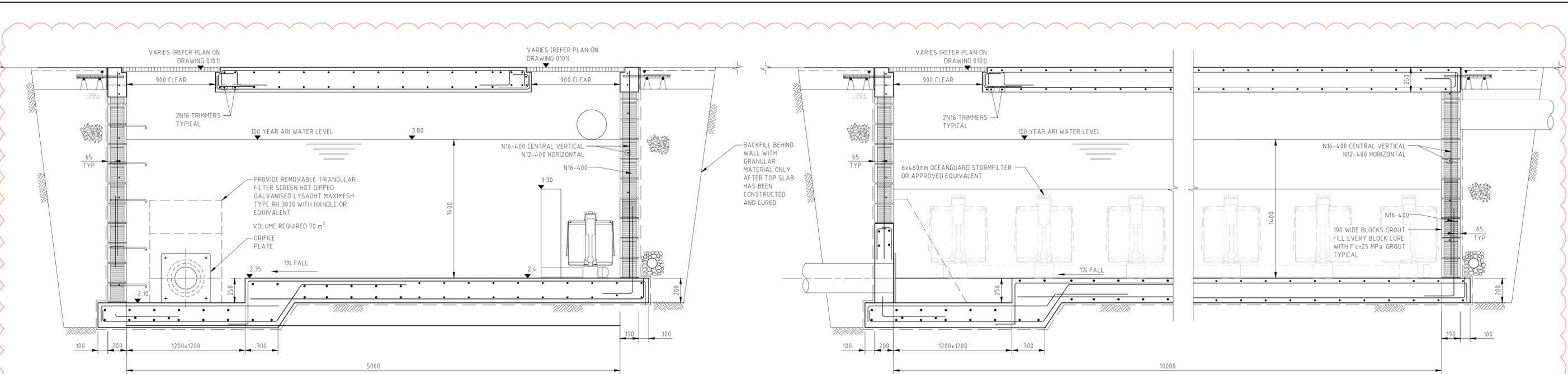
Title: **CIVIL
 SITE AND STORMWATER DRAINAGE PLAN
 SHEET 2**

Status: **SCHEMATIC DESIGN
 NOT TO BE USED FOR CONSTRUCTION PURPOSES**

Datum	AHD	Scale	1:200	Size	A1	
Drawing Number	80821341-CI-0102				Revision	4

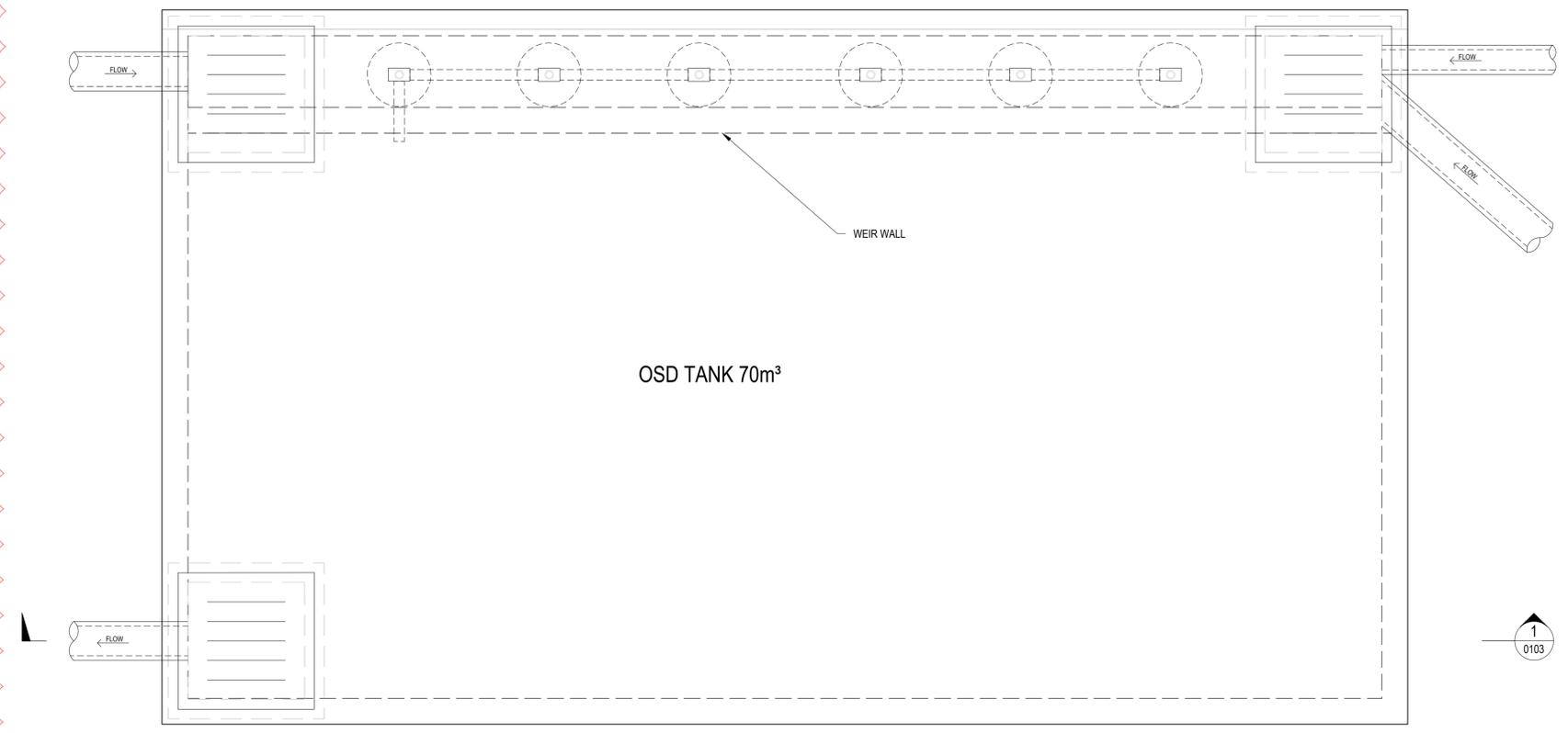


SECTION A-A
 SCALE 1:20
 0102



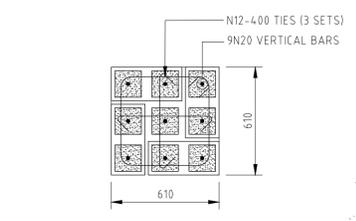
SECTION 1
SCALE 1:20
0103

SECTION 2
SCALE 1:20
0101

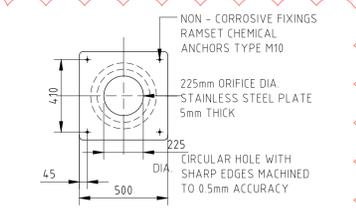


OSD TANK 70m³

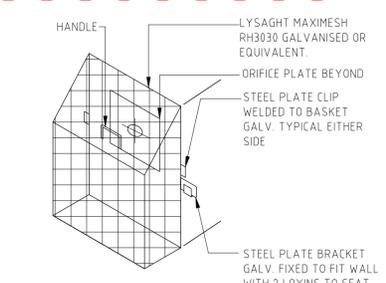
OSD TANK PLAN
SCALE 1:25



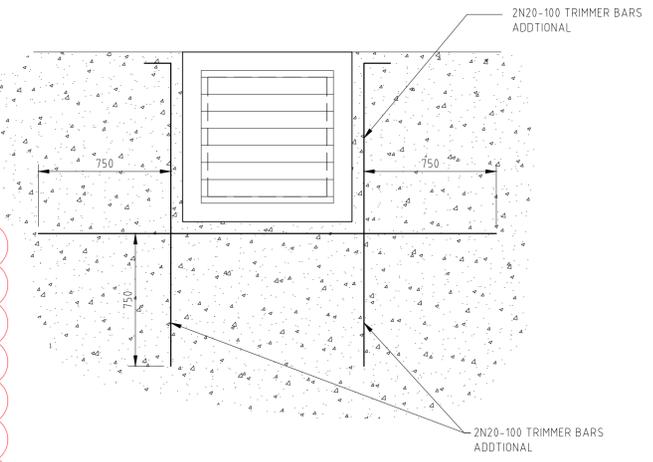
TYPICAL BLOCK PIER
N.T.S.



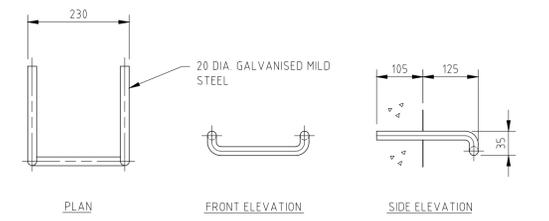
ORIFICE PLATE DETAIL
N.T.S.



TRASH SCREEN DETAIL
N.T.S.



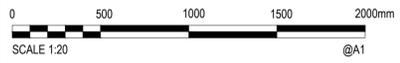
TRIMMER BARS TO OPENINGS
N.T.S.



STEP IRON DETAIL
N.T.S.

XREFs: CAD File: N:\Projects\08\FY21\341_Cronulla_HS-Phase 2 to 6012_OVILDDrawings\Build\8021341-CI-0103.dwg

Rev.	Date	Description	Des.	Verif.	Appd.
4	14/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
3	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
2	01/07/2022	ISSUED FOR COORDINATION	VJ	VJ	PP
1	27/06/2022	ISSUED FOR COORDINATION	VJ	VJ	PP

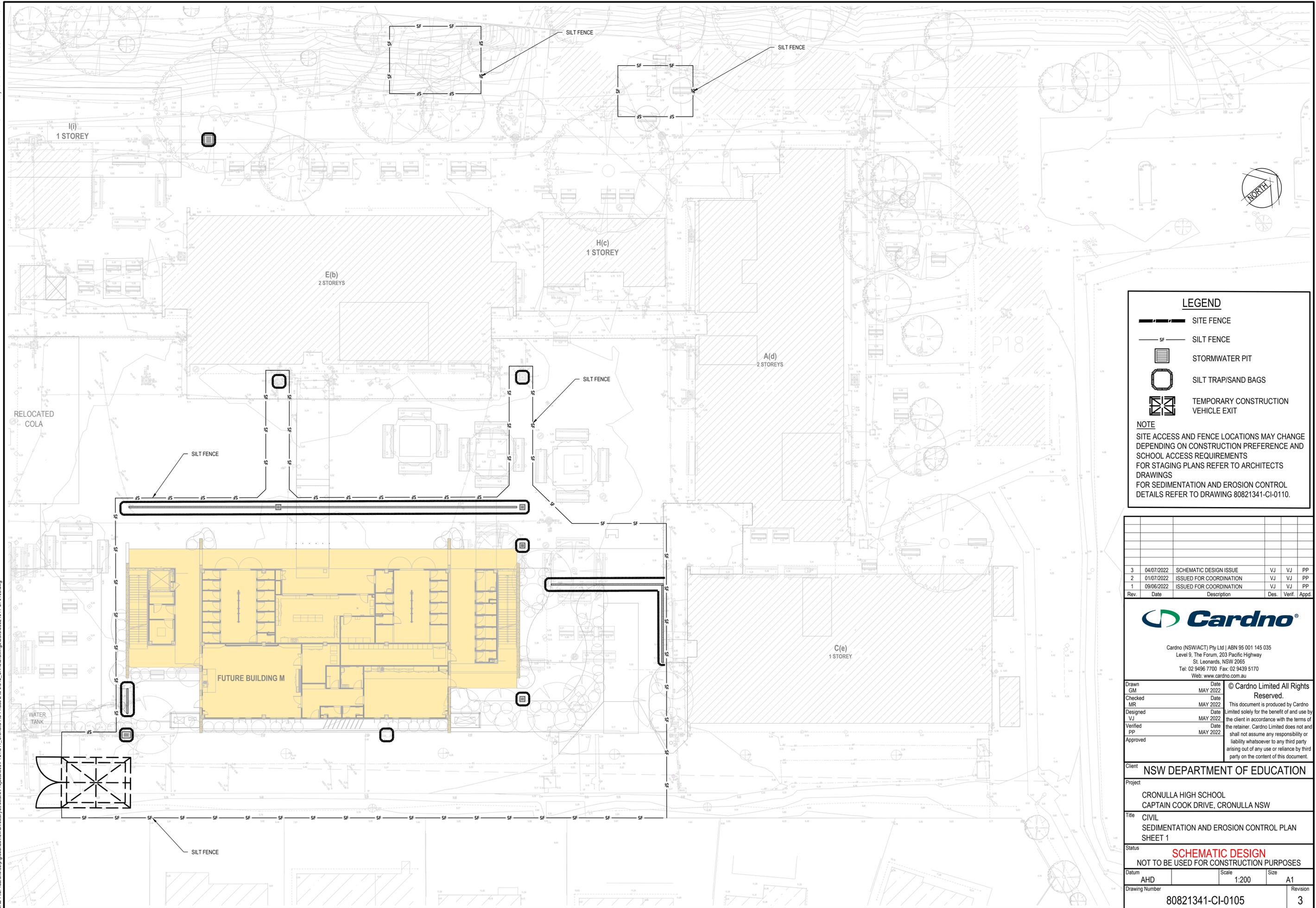


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Drawn GM	Date MAY 2022	Client NSW DEPARTMENT OF EDUCATION
Checked MR	Date MAY 2022	Project CRONULLA HIGH SCHOOL CAPTAIN COOK DRIVE, CRONULLA NSW
Designed VJ	Date MAY 2022	Title CIVIL OSD TANK SECTIONS AN DETAILS
Verified PP	Date MAY 2022	
Approved		

Status SCHEMATIC DESIGN	Scale 1:200	Size A1
Datum AHD	Drawing Number 80821341-CI-0103	Revision 4



LEGEND

- SITE FENCE
- SILT FENCE
- STORMWATER PIT
- SILT TRAP/SAND BAGS
- TEMPORARY CONSTRUCTION VEHICLE EXIT

NOTE
 SITE ACCESS AND FENCE LOCATIONS MAY CHANGE DEPENDING ON CONSTRUCTION PREFERENCE AND SCHOOL ACCESS REQUIREMENTS
 FOR STAGING PLANS REFER TO ARCHITECTS DRAWINGS
 FOR SEDIMENTATION AND EROSION CONTROL DETAILS REFER TO DRAWING 80821341-CI-0110.

Rev.	Date	Description	Des.	Verif.	Appd.
3	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
2	01/07/2022	ISSUED FOR COORDINATION	VJ	VJ	PP
1	09/06/2022	ISSUED FOR COORDINATION	VJ	VJ	PP



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Approved		Date		

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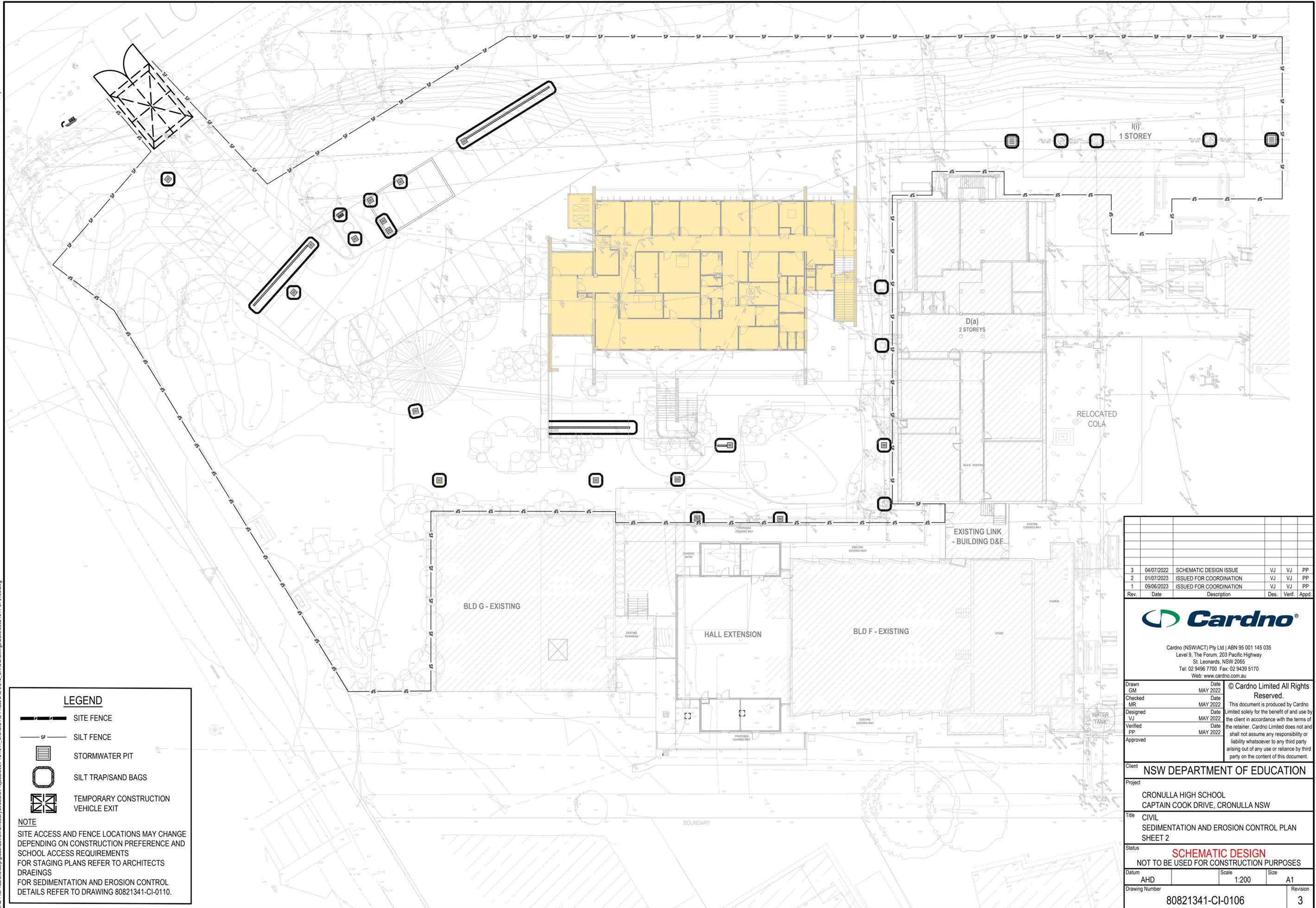
Project **CRONULLA HIGH SCHOOL
 CAPTAIN COOK DRIVE, CRONULLA NSW**

Title **CIVIL
 SEDIMENTATION AND EROSION CONTROL PLAN
 SHEET 1**

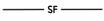
Status **SCHEMATIC DESIGN
 NOT TO BE USED FOR CONSTRUCTION PURPOSES**

Datum	AHD	Scale	1:200	Size	A1
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Drawing Number	80821341-CI-0105	Revision	3
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LEGEND

-  SITE FENCE
-  SILT FENCE
-  STORMWATER PIT
-  SILT TRAP/SAND BAGS
-  TEMPORARY CONSTRUCTION VEHICLE EXIT

NOTE
 SITE ACCESS AND FENCE LOCATIONS MAY CHANGE DEPENDING ON CONSTRUCTION PREFERENCE AND SCHOOL ACCESS REQUIREMENTS
 FOR STAGING PLANS REFER TO ARCHITECTS DRAEINGS
 FOR SEDIMENTATION AND EROSION CONTROL DETAILS REFER TO DRAWING 80821341-CI-0110.

Rev.	Date	Description	Des.	Verif.	Appd.
3	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
2	01/07/2023	ISSUED FOR COORDINATION	VJ	VJ	PP
1	09/06/2023	ISSUED FOR COORDINATION	VJ	VJ	PP



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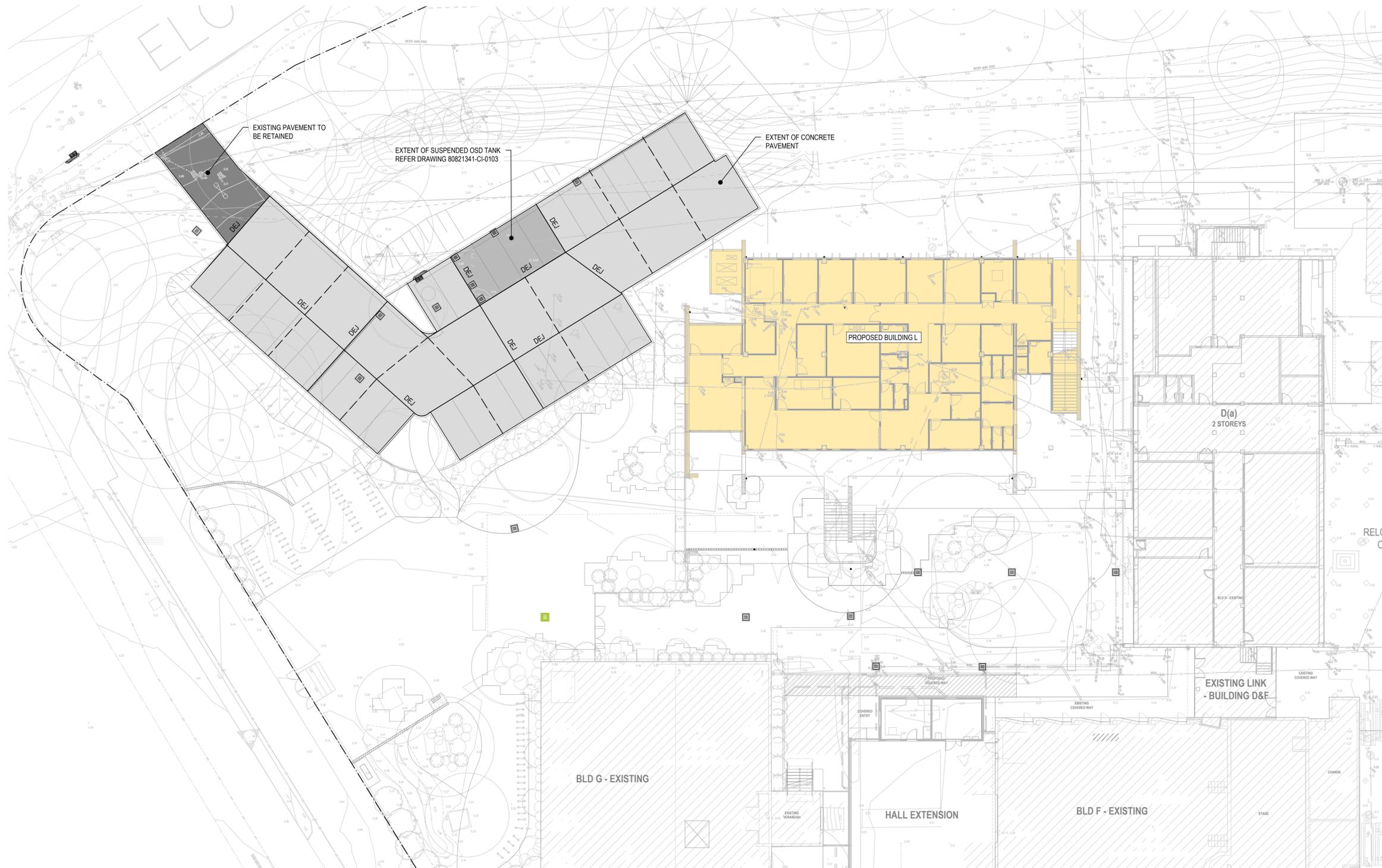
Project **CRONULLA HIGH SCHOOL
 CAPTAIN COOK DRIVE, CRONULLA NSW**

Title **CIVIL
 SEDIMENTATION AND EROSION CONTROL PLAN
 SHEET 2**

Status **SCHEMATIC DESIGN**
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Datum	AHD	Scale	1:200	Size	A1
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Drawing Number	80821341-CI-0106	Revision	3
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LEGEND:

	EXISTING PAVEMENT TO REMAIN
	PROPOSED CONCRETE PAVEMENT
	SUSPENDED OSD TANK SLAB

Rev.	Date	Description	Des.	Verif.	Appd.
2	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
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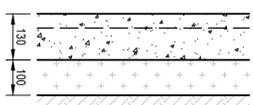
Project: **CRONULLA HIGH SCHOOL
 CAPTAIN COOK DRIVE, CRONULLA NSW**

Title: **CIVIL
 CAR PARK PAVEMENT PLAN AND
 DETAILS**

Status: **SCHEMATIC DESIGN
 NOT TO BE USED FOR CONSTRUCTION PURPOSES**

Datum	AHD	Scale	1:200	Size	A1
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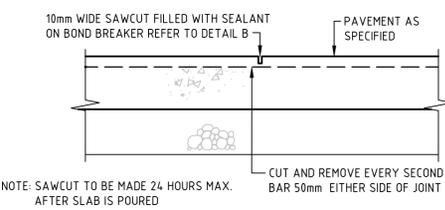
Drawing Number	80821341-CI-0109	Revision	2
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130MM SLAB, REINFORCED WITH SL82 MESH POURED ON 5MM SAND BLINDING OVER VAPOUR PROOF MEMBRANE ON 100MM DGB20 COMPACTED TO 98% SMDD. INCREASE SLAB THICKNESS TO 220MM FOR AT LEAST 15M FROM THE ENDS.
 SUBGRADE TO BE PREPARED FOR A CBR OF 10% IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT

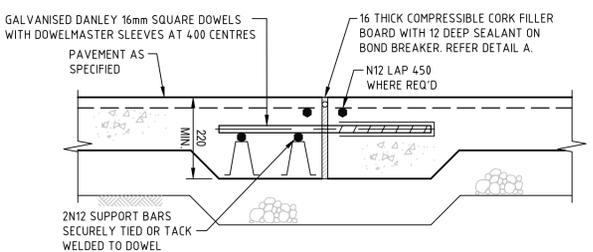
CONCRETE PAVEMENT
 SCALE 1:10

JOINTS AS SHOWN SHALL BE SAWN JOINTS UNLESS NOTED OTHERWISE



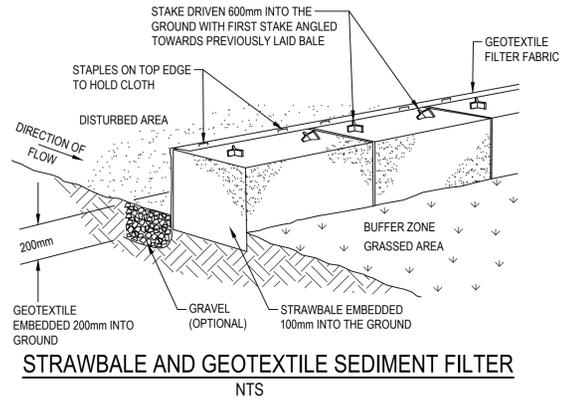
NOTE: SAWCUT TO BE MADE 24 HOURS MAX. AFTER SLAB IS POURED
 CUT AND REMOVE EVERY SECOND BAR 50mm EITHER SIDE OF JOINT

SAWN JOINT (TYPICAL)
 SCALE 1:10

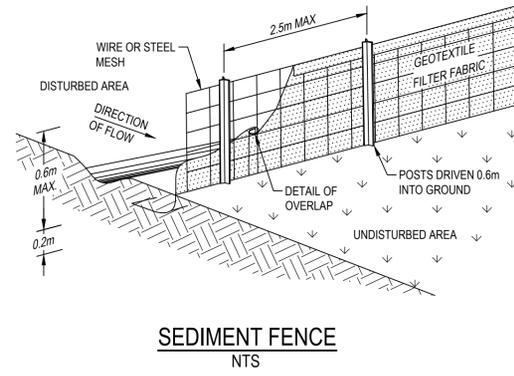


GALVANISED DANLEY 16mm SQUARE DOWELS WITH DOWELMASTER SLEEVES AT 400 CENTRES
 16 THICK COMPRESSIBLE CORK FILLER BOARD WITH 12 DEEP SEALANT ON BOND BREAKER. REFER DETAIL A.
 N12 LAP 450 WHERE REQ'D
 2N12 SUPPORT BARS SECURELY TIED OR TACK WELDED TO DOWEL

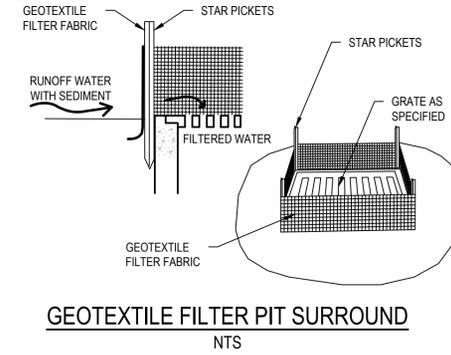
DOWELLED EXPANSION JOINT (DEJ1) (TYPICAL UNO)
 SCALE 1:10



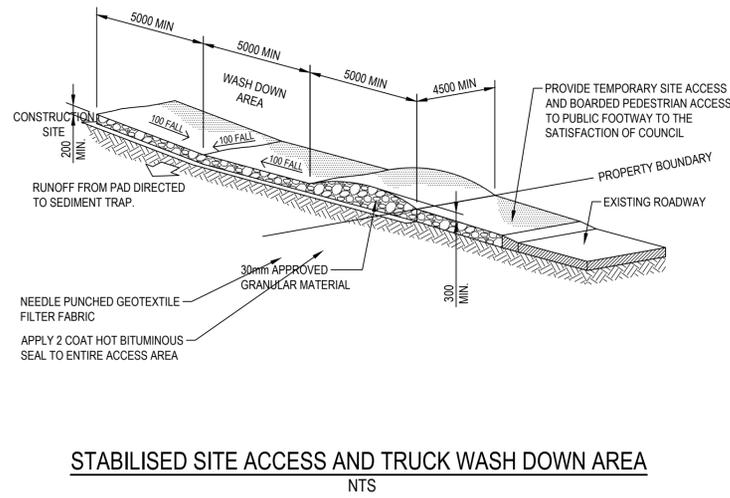
STRAWBALE AND GEOTEXTILE SEDIMENT FILTER
NTS



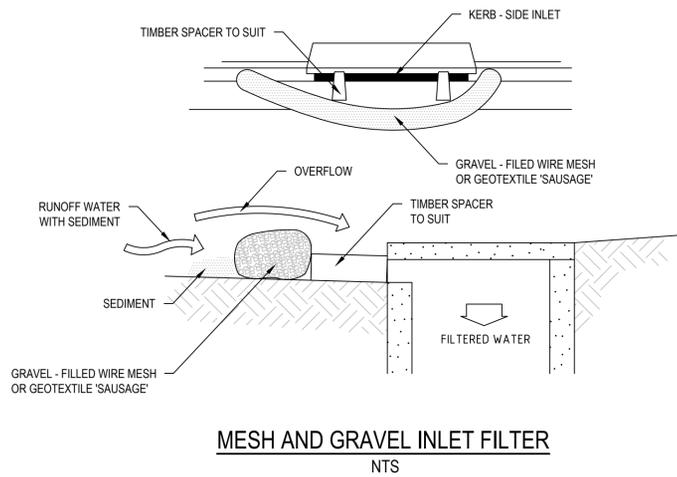
SEDIMENT FENCE
NTS



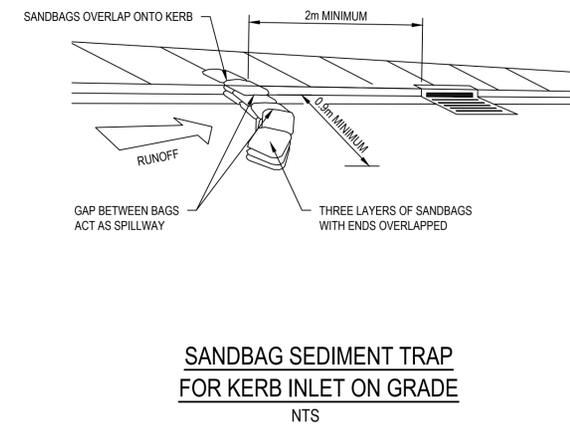
GEOTEXTILE FILTER PIT SURROUND
NTS



STABILISED SITE ACCESS AND TRUCK WASH DOWN AREA
NTS



MESH AND GRAVEL INLET FILTER
NTS



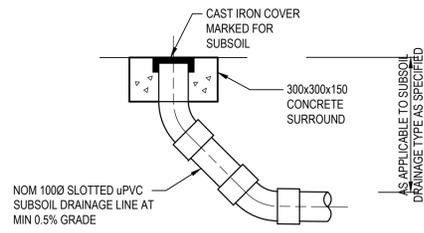
SANDBAG SEDIMENT TRAP FOR KERB INLET ON GRADE
NTS

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2	01/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
1	09/06/2022	ISSUED FOR COORDINATION	VJ	VJ	PP

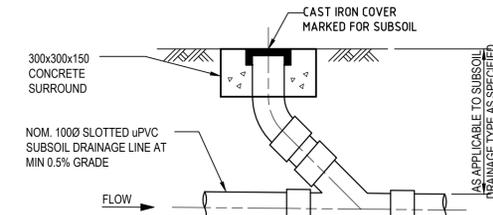
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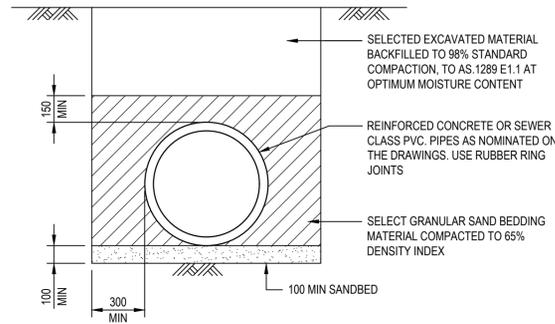
Drawn GM	Date MAY 2022	Client NSW DEPARTMENT OF EDUCATION
Checked MR	Date MAY 2022	Project CRONULLA HIGH SCHOOL CAPTAIN COOK DRIVE, CRONULLA NSW
Designed VJ	Date MAY 2022	Title CIVIL SEDIMENTATION AND EROSION CONTROL DETAILS
Verified PP	Date MAY 2022	Status SCHEMATIC DESIGN NOT TO BE USED FOR CONSTRUCTION PURPOSES
Approved		Datum AHD
		Scale 1:200
		Size A1
		Drawing Number 80821341-CI-0110
		Revision 3



HIGH END RISER
N.T.S.

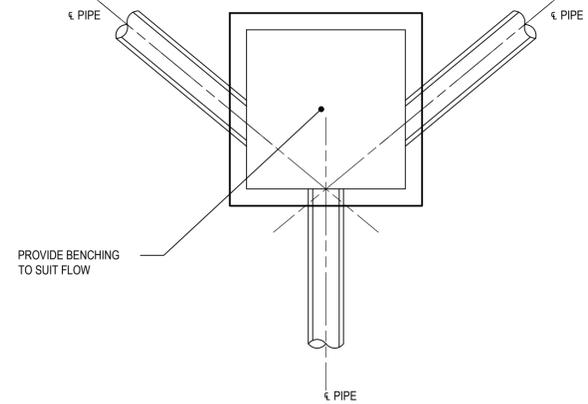


INTERMEDIATE RISER
N.T.S.
(TYPICAL AT 25m. INTERVALS UNLESS OTHERWISE NOTED ON PLAN)

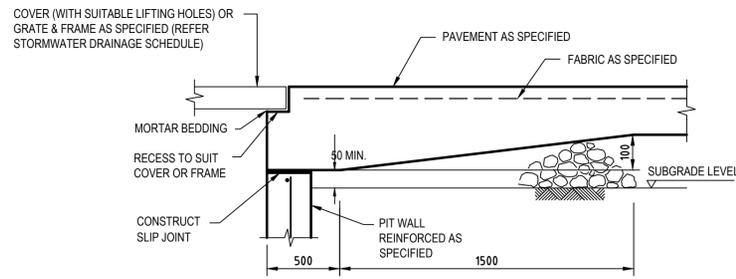


TYPICAL PIPE LAYING DETAIL U.N.O.

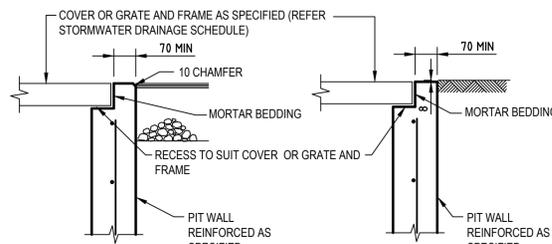
NOTE:
ALLOW TO BUILD 1000 AG. DRAIN IN THE LAST 3.0m OF DRAINAGE PIPE, UPSTREAM OF ALL DRAINAGE PITS.



JUNCTION PIT SET OUT DETAILS
N.T.S.

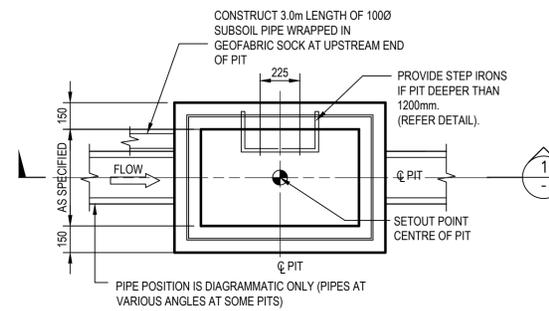


DETAIL "A"
N.T.S.



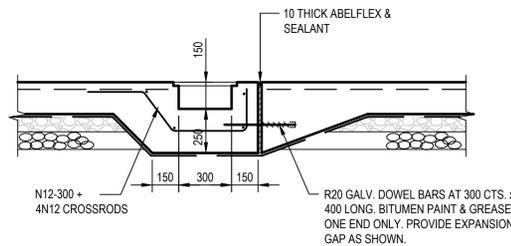
DETAIL "B"
SCALE 1:10

DETAIL "C"
SCALE 1:10

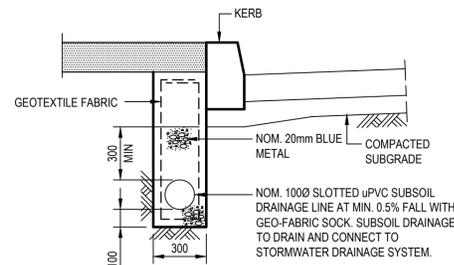


PLAN
N.T.S.

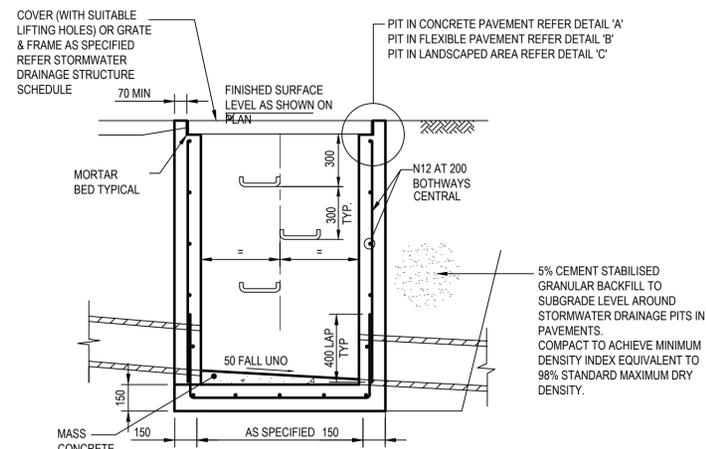
PIT EDGE TREATMENT FOR STORMWATER DRAINAGE AND OTHER SERVICES PITS IN ROAD PAVEMENTS AND LANDSCAPED AREAS



GRATED DRAIN DETAIL
SCALE 1:20



SUBSOIL DRAINAGE LINE IN LANDSCAPED AREAS ADJACENT TO ROADS



SECTION 1
SCALE 1:20

SURFACE INLET/JUNCTION PIT (SIP/JP)

Rev.	Date	Description	Des.	Verif.	Appd.
2	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
1	01/07/2022	ISSUED FOR COORDINATION	VJ	VJ	PP

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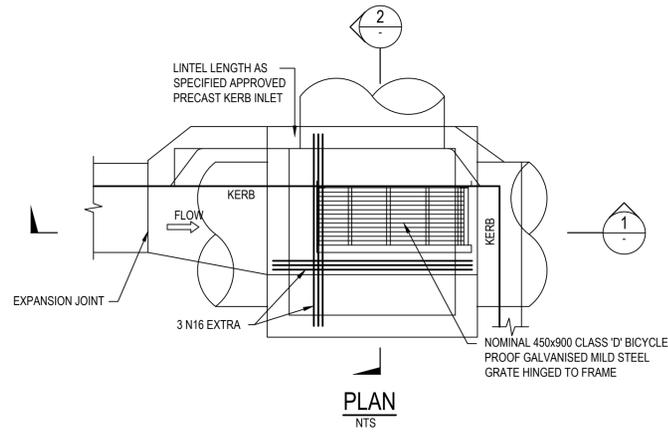


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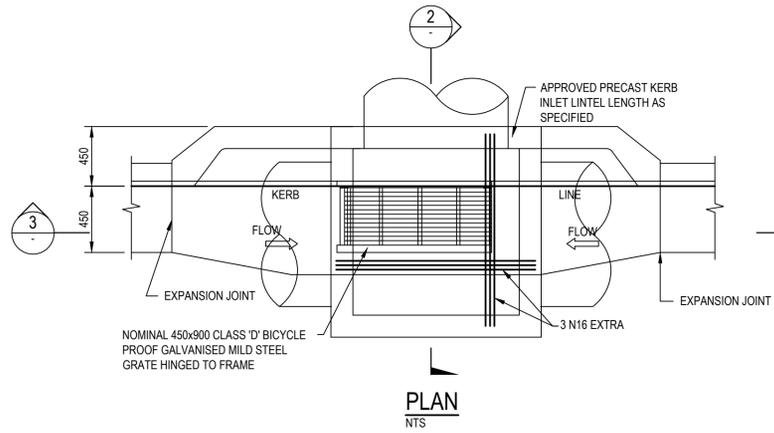
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Checked	MR	Date	MAY 2022
Designed	VJ	Date	MAY 2022
Verified	PP	Date	MAY 2022
Approved			

Client	NSW DEPARTMENT OF EDUCATION
Project	CRONULLA HIGH SCHOOL CAPTAIN COOK DRIVE, CRONULLA NSW
Title	CIVIL STORMWATER STANDARD DEALS SHEET 1

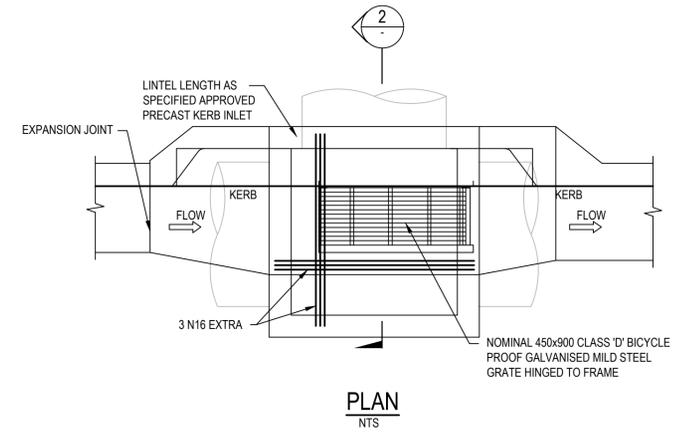
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NOT TO BE USED FOR CONSTRUCTION PURPOSES			
Datum	AHD	Scale	1:200
Size			A1
Drawing Number	80821341-CI-0111		Revision
			2



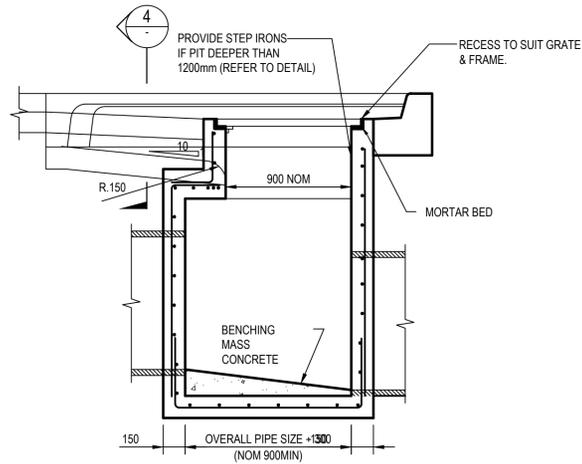
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(CORNER)**



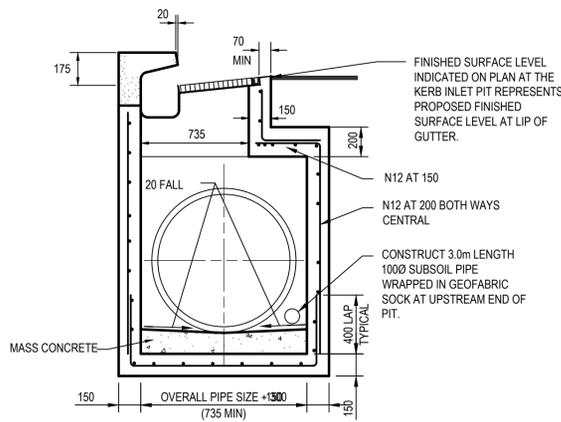
**KORB INLET PIT (KIP-B)
(SAG)**



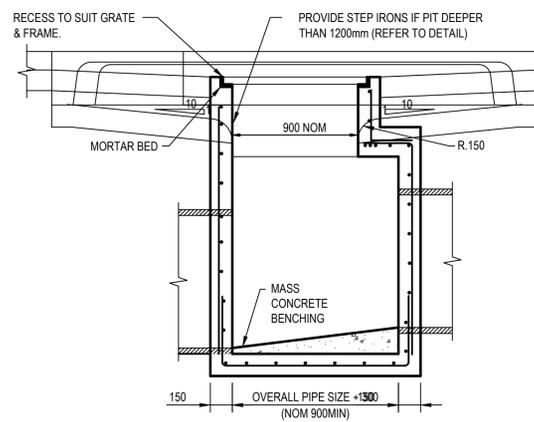
**KORB INLET PIT (KIP-C)
(ON GRADE)**



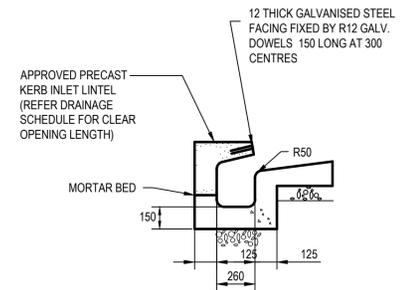
**SECTION 1
SCALE 1:20**



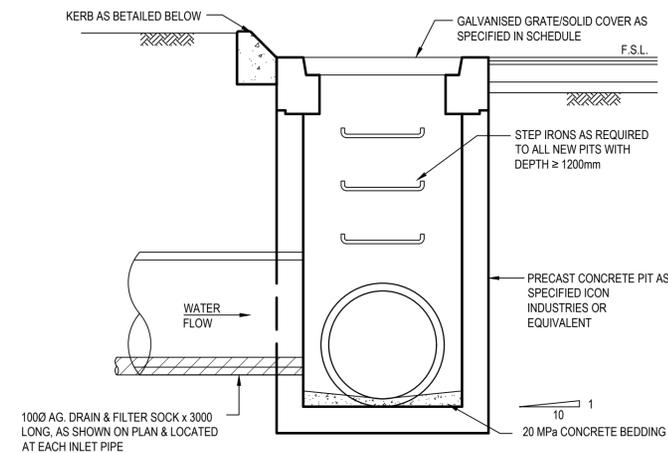
**SECTION 2
SCALE 1:20**



**SECTION 3
SCALE 1:20**



**SECTION 4
SCALE 1:20**



PRECAST STORMWATER PIT

TO BE INSTALLED AND BEDDED IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS. PROVIDE GALVANISED IRON STEP IRONS EPOXIED INTO CONCRETE FOR PITS DEEPER THAN 1200mm.

Rev.	Date	Description	Des.	Verif.	Appd.
2	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
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Verified	PP	Date	MAY 2022
Approved			

Client	NSW DEPARTMENT OF EDUCATION
Project	CRONULLA HIGH SCHOOL CAPTAIN COOK DRIVE, CRONULLA NSW
Title	CIVIL STORMWATER STANDARD DEALS SHEET 2

Status	SCHEMATIC DESIGN NOT TO BE USED FOR CONSTRUCTION PURPOSES		
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Size			A1
Drawing Number	80821341-CI-0112		Revision
			2

APPENDIX

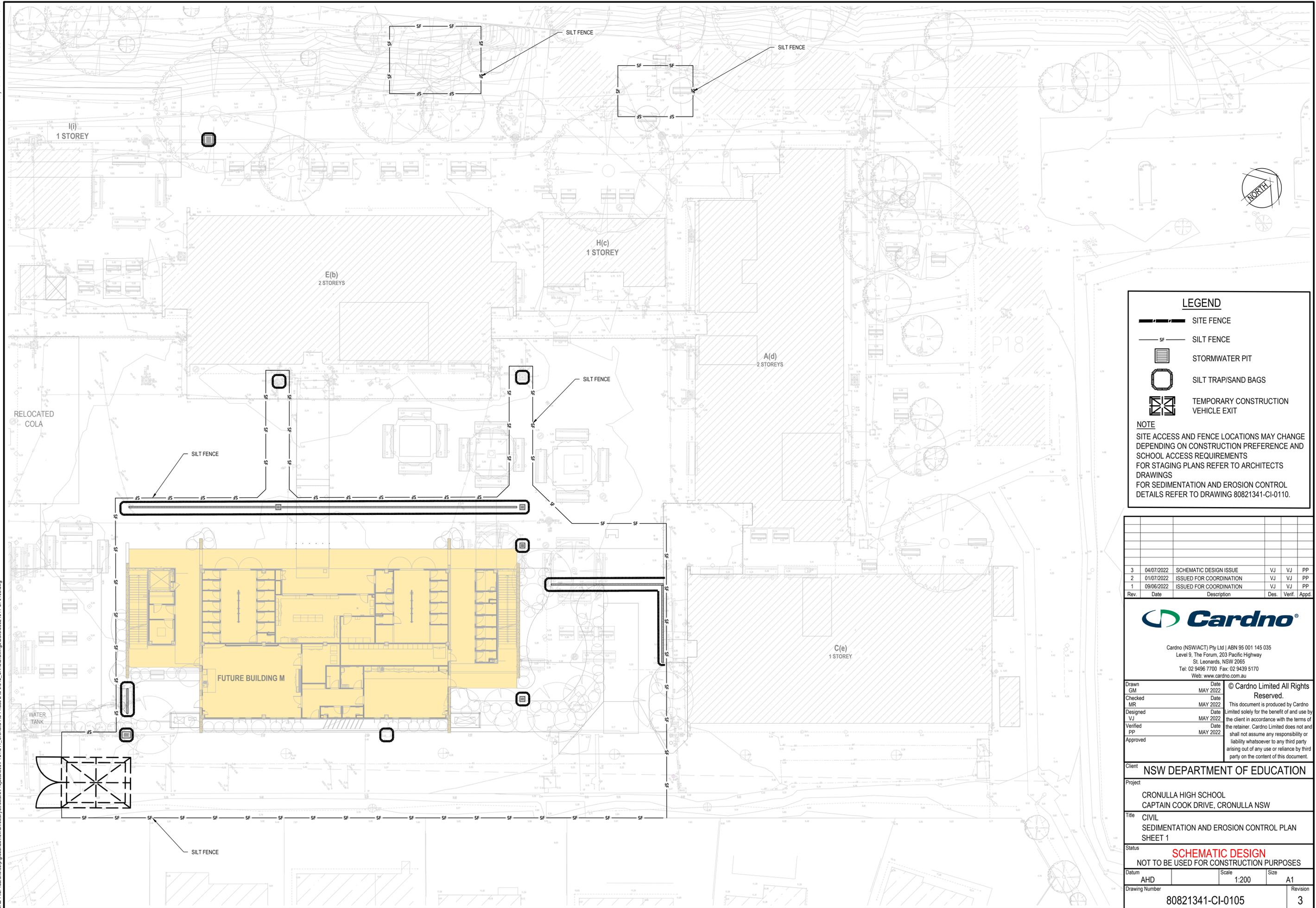
B

SOIL WATER AND MANAGEMENT PLAN



now





LEGEND

- SITE FENCE
- SILT FENCE
- STORMWATER PIT
- SILT TRAP/SAND BAGS
- TEMPORARY CONSTRUCTION VEHICLE EXIT

NOTE
 SITE ACCESS AND FENCE LOCATIONS MAY CHANGE DEPENDING ON CONSTRUCTION PREFERENCE AND SCHOOL ACCESS REQUIREMENTS
 FOR STAGING PLANS REFER TO ARCHITECTS DRAWINGS
 FOR SEDIMENTATION AND EROSION CONTROL DETAILS REFER TO DRAWING 80821341-CI-0110.

Rev.	Date	Description	Des.	Verif.	Appd.
3	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
2	01/07/2022	ISSUED FOR COORDINATION	VJ	VJ	PP
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Verified	PP	Date	MAY 2022	
Approved		Date		

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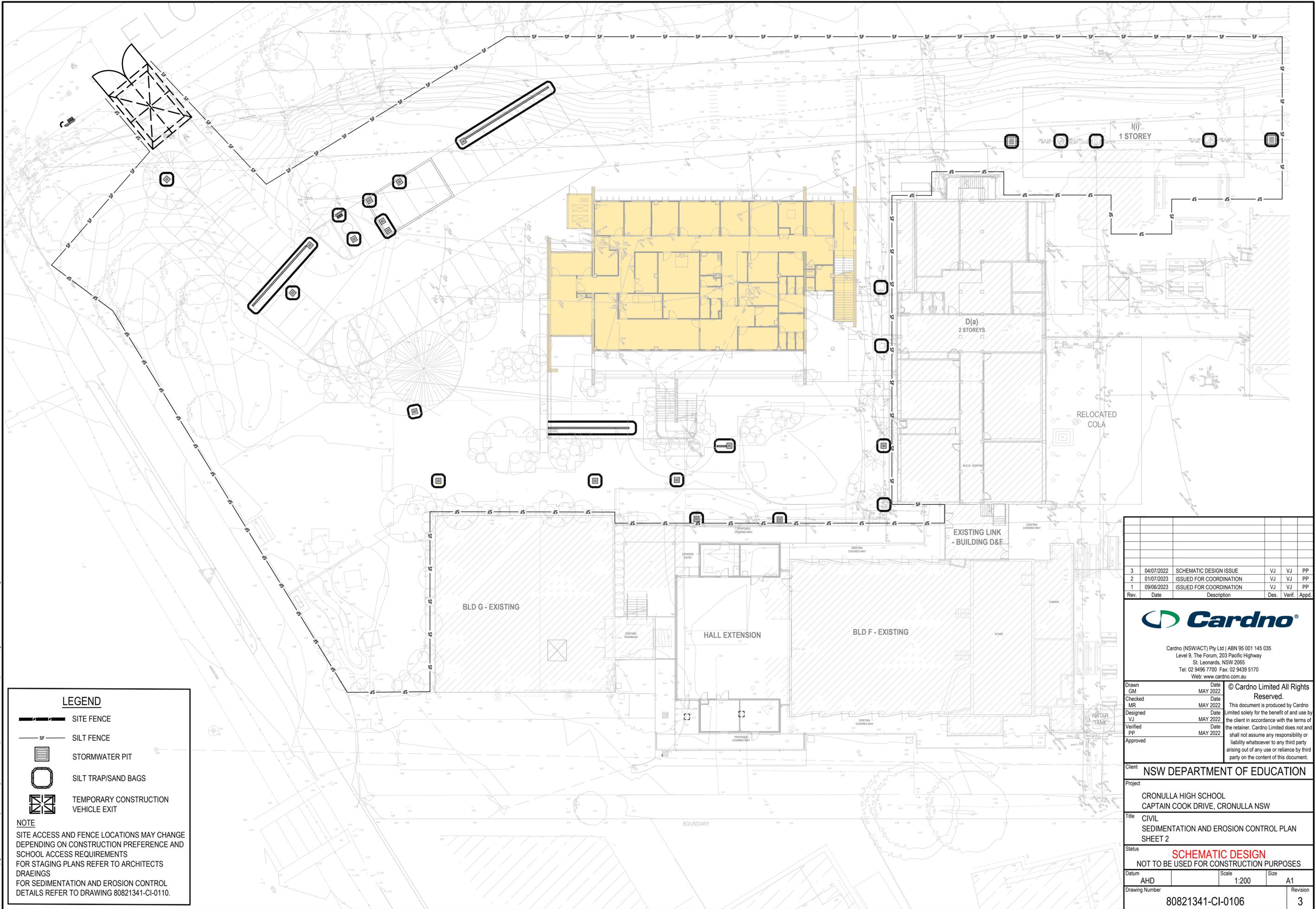
Project **CRONULLA HIGH SCHOOL
 CAPTAIN COOK DRIVE, CRONULLA NSW**

Title **CIVIL
 SEDIMENTATION AND EROSION CONTROL PLAN
 SHEET 1**

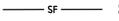
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 NOT TO BE USED FOR CONSTRUCTION PURPOSES

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Drawing Number	80821341-CI-0105	Revision	3
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LEGEND

-  SITE FENCE
-  SILT FENCE
-  STORMWATER PIT
-  SILT TRAP/SAND BAGS
-  TEMPORARY CONSTRUCTION VEHICLE EXIT

NOTE
 SITE ACCESS AND FENCE LOCATIONS MAY CHANGE DEPENDING ON CONSTRUCTION PREFERENCE AND SCHOOL ACCESS REQUIREMENTS
 FOR STAGING PLANS REFER TO ARCHITECTS DRAEINGS
 FOR SEDIMENTATION AND EROSION CONTROL DETAILS REFER TO DRAWING 80821341-CI-0110.

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Client **NSW DEPARTMENT OF EDUCATION**

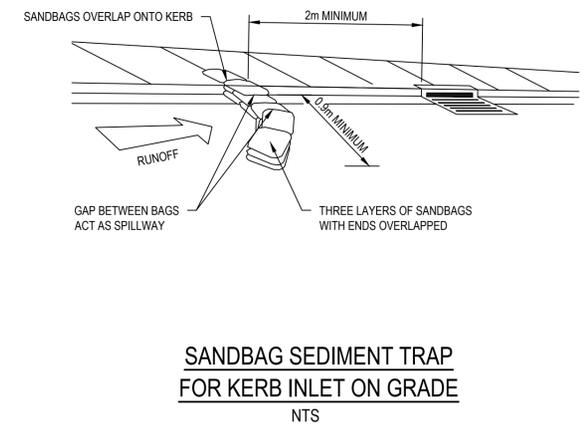
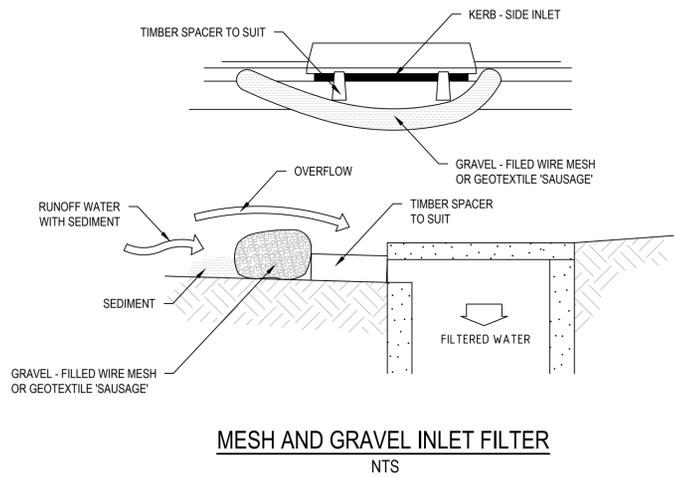
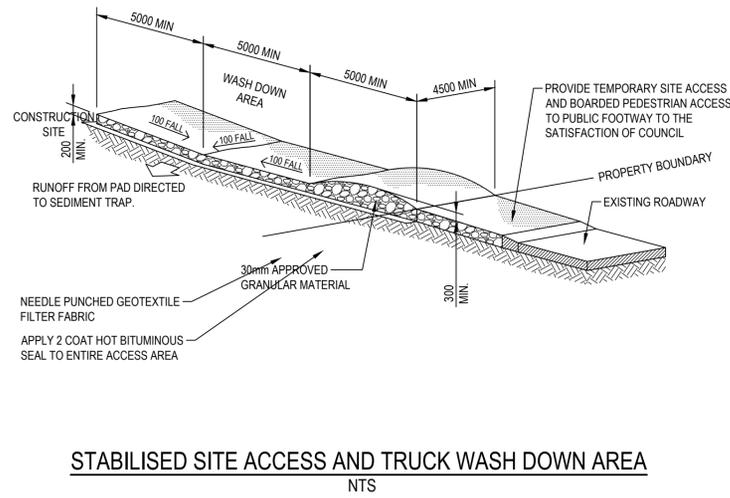
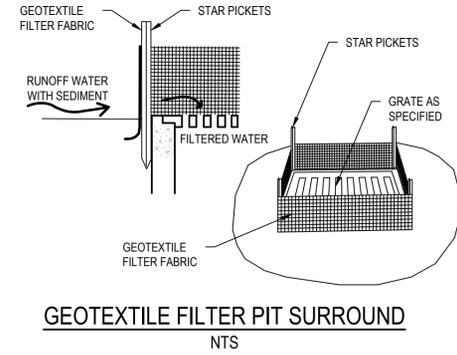
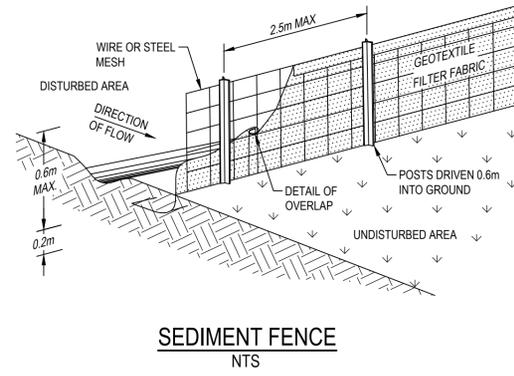
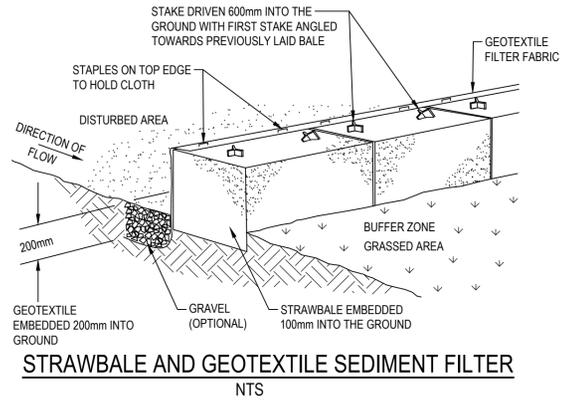
Project **CRONULLA HIGH SCHOOL
 CAPTAIN COOK DRIVE, CRONULLA NSW**

Title **CIVIL
 SEDIMENTATION AND EROSION CONTROL PLAN
 SHEET 2**

Status **SCHEMATIC DESIGN**
 NOT TO BE USED FOR CONSTRUCTION PURPOSES

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Drawing Number	80821341-CI-0106	Revision	3
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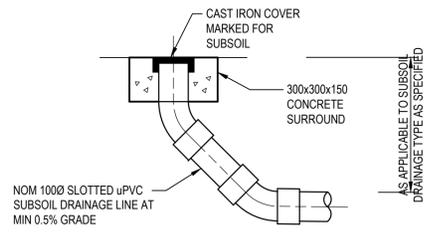


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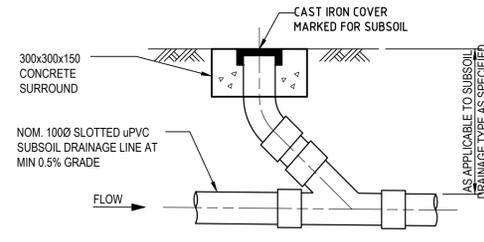
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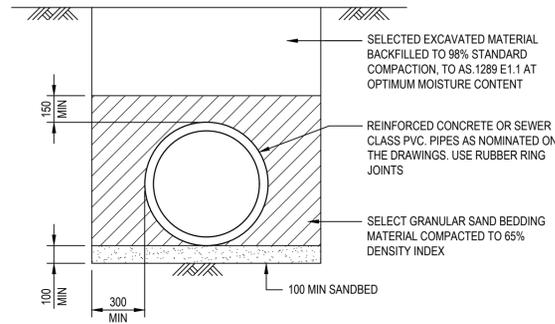
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Checked MR	Date MAY 2022	Project CRONULLA HIGH SCHOOL CAPTAIN COOK DRIVE, CRONULLA NSW
Designed VJ	Date MAY 2022	Title CIVIL SEDIMENTATION AND EROSION CONTROL DETAILS
Verified PP	Date MAY 2022	Status SCHEMATIC DESIGN NOT TO BE USED FOR CONSTRUCTION PURPOSES
Approved		Datum AHD
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		Drawing Number 80821341-CI-0110
		Revision 3



HIGH END RISER
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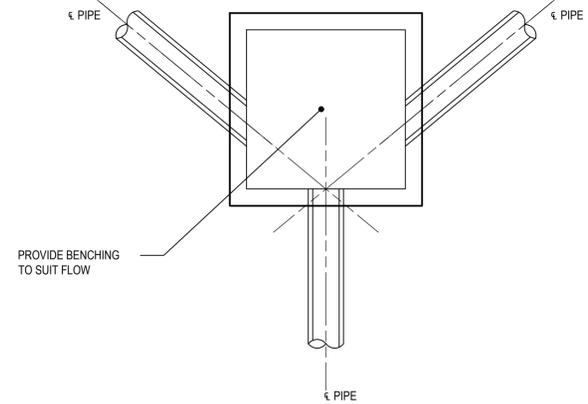


INTERMEDIATE RISER
N.T.S.
(TYPICAL AT 25m. INTERVALS UNLESS OTHERWISE NOTED ON PLAN)

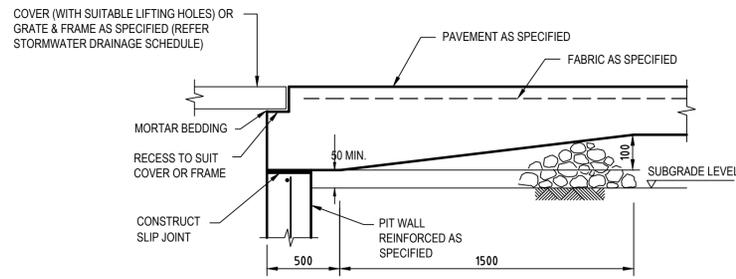


TYPICAL PIPE LAYING DETAIL U.N.O.

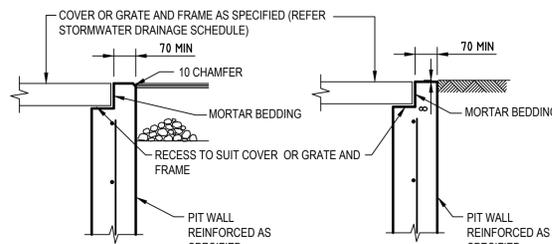
NOTE:
ALLOW TO BUILD 1000 AG. DRAIN IN THE LAST 3.0m OF DRAINAGE PIPE, UPSTREAM OF ALL DRAINAGE PITS.



JUNCTION PIT SET OUT DETAILS
N.T.S.

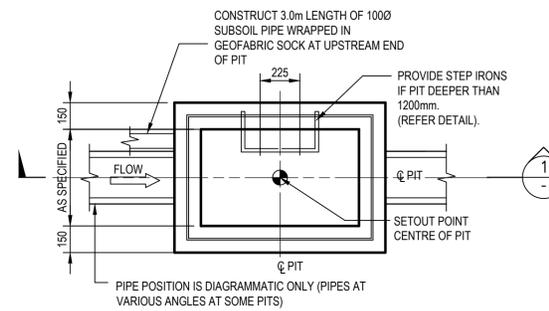


DETAIL "A"
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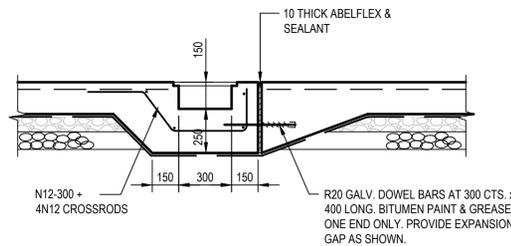
DETAIL "B"
SCALE 1:10

DETAIL "C"
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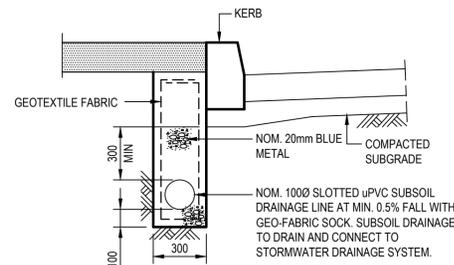


PLAN
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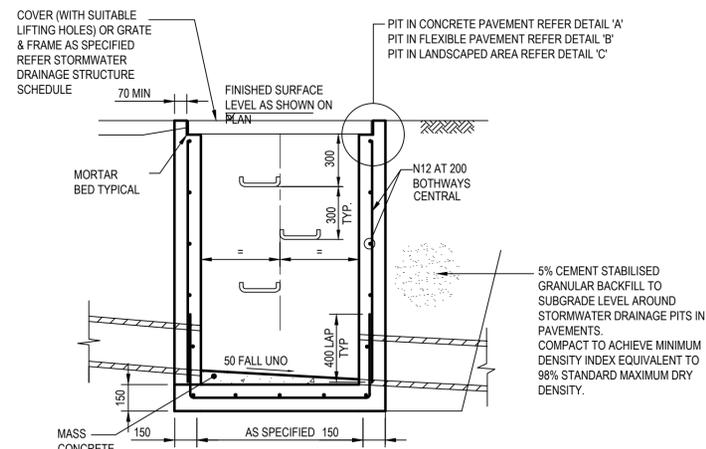
PIT EDGE TREATMENT FOR STORMWATER DRAINAGE AND OTHER SERVICES PITS IN ROAD PAVEMENTS AND LANDSCAPED AREAS



GRATED DRAIN DETAIL
SCALE 1:20



SUBSOIL DRAINAGE LINE IN LANDSCAPED AREAS ADJACENT TO ROADS



SECTION 1
SCALE 1:20

SURFACE INLET/JUNCTION PIT (SIP/JP)

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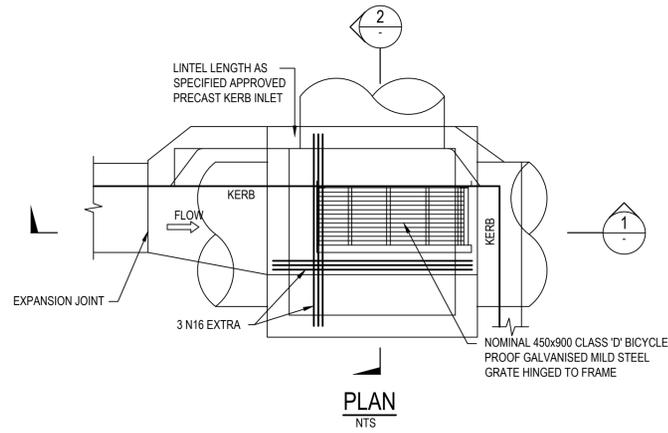


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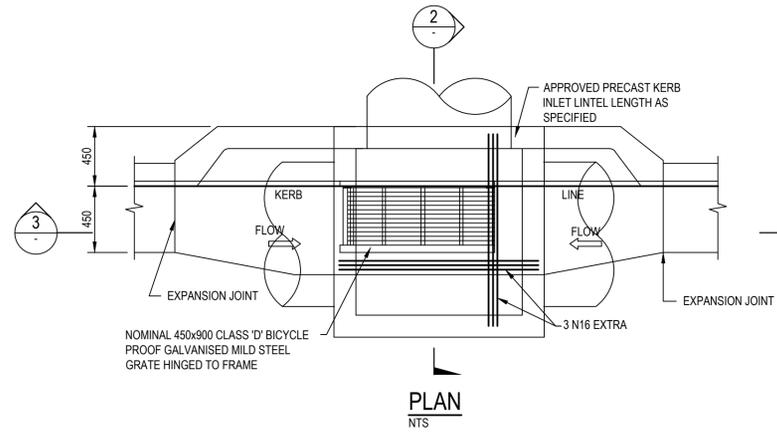
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Designed	VJ	Date	MAY 2022
Verified	PP	Date	MAY 2022
Approved			

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Project	CRONULLA HIGH SCHOOL CAPTAIN COOK DRIVE, CRONULLA NSW
Title	CIVIL STORMWATER STANDARD DEALS SHEET 1

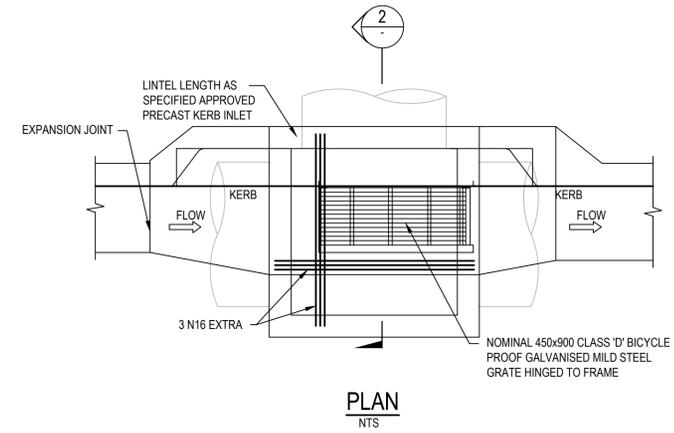
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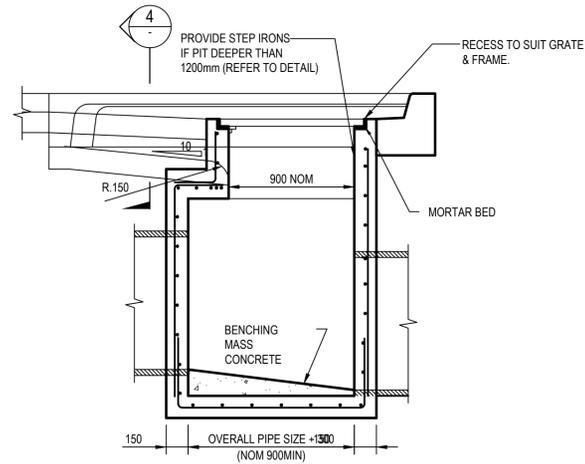
PLAN
NTS
KERB INLET PIT (KIP-A)
(CORNER)



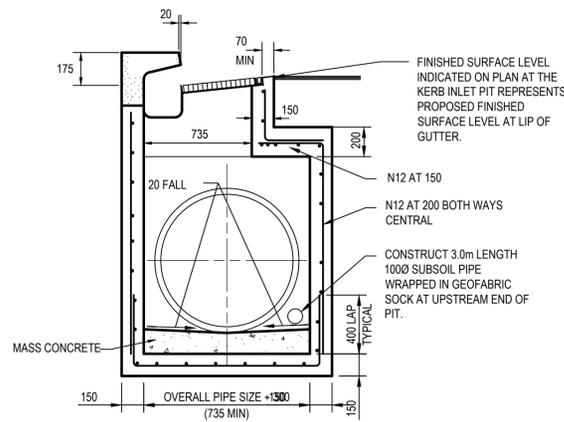
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KERB INLET PIT (KIP-B)
(SAG)



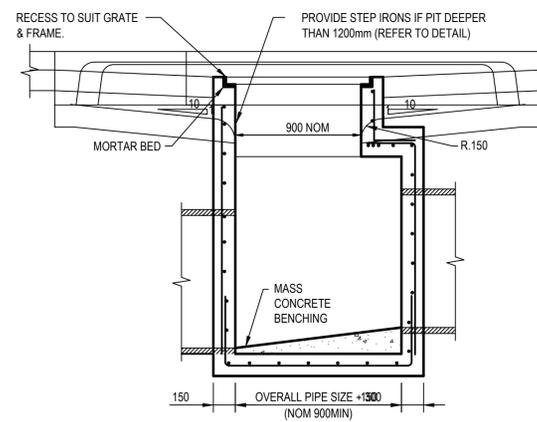
PLAN
NTS
KERB INLET PIT (KIP-C)
(ON GRADE)



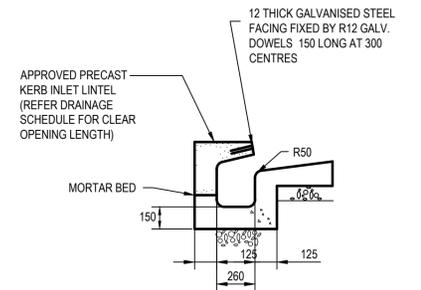
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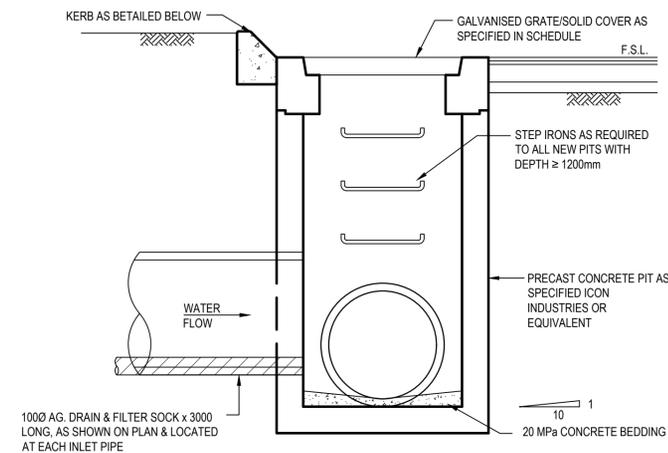
SECTION 2
SCALE 1:20



SECTION 3
SCALE 1:20



SECTION 4
SCALE 1:20



PRECAST STORMWATER PIT
TO BE INSTALLED AND BEDDED IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS. PROVIDE GALVANISED IRON STEP IRONS EPOXIED INTO CONCRETE FOR PITS DEEPER THAN 1200mm.

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Verified	PP	Date	MAY 2022
Approved			

Client	NSW DEPARTMENT OF EDUCATION
Project	CRONULLA HIGH SCHOOL CAPTAIN COOK DRIVE, CRONULLA NSW
Title	CIVIL STORMWATER STANDARD DEALS SHEET 2

Status	SCHEMATIC DESIGN NOT TO BE USED FOR CONSTRUCTION PURPOSES		
Datum	AHD	Scale	1:200
Size			A1
Drawing Number	80821341-CI-0112		Revision
			2

APPENDIX

C

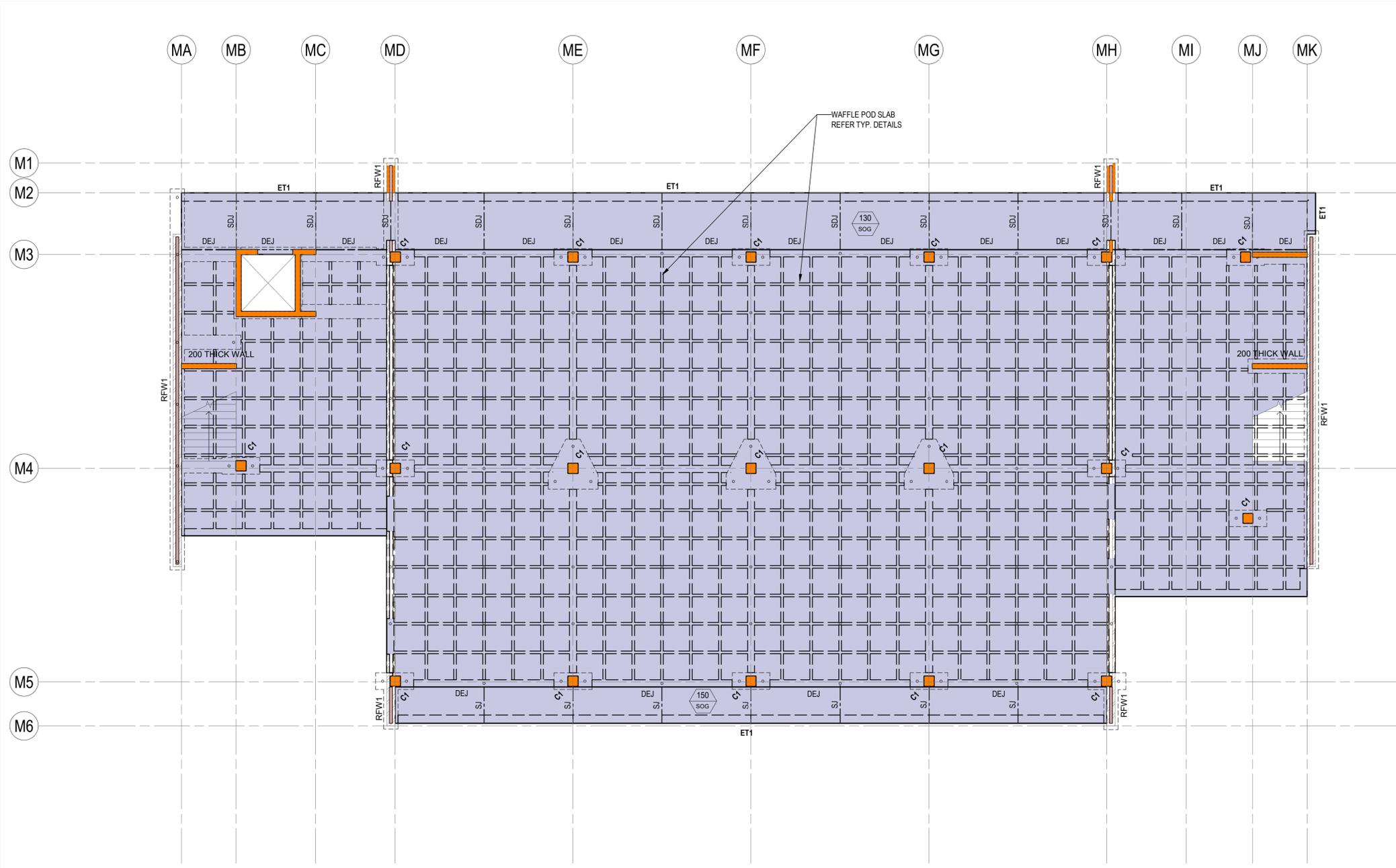
STRUCTURAL ENGINEERING DRAWINGS



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- SLAB ON GRADE PLAN NOTES**
- GENERAL
- FOR CONSTRUCTION NOTES & DRAWING LIST REFER TO DRAWING No. ST-0001
 - FOR BUILDING SETOUT AND DIMENSIONS REFER TO ARCHITECTS DRAWINGS
 - FOR SLAB FLOOR FINISHED REFER TO ARCHITECTS DRAWINGS.
 - FOR ALL SLAB PENETRATIONS REFER TO ARCHITECTS & BUILDING SERVICES DRAWINGS.
 - SURFACE FALLS, DRAINAGE & EMERGENCY OVERFLOWS IN EXTERNAL AREAS TO ARCHITECTS DETAILS.
 - ALL WATERPROOFING TO ARCHITECTS SPECIFICATION.
 - ALL FALLS TO ARCHITECTS DETAILS
 - FOR ADDITIONAL REINFORCEMENT REFER TO PLANS, SECTIONS AND DETAILS.
 - PROVIDE TRIMMER BARS AROUND ALL PENETRATIONS, COLUMNS, JOINT INTERSECTIONS AND RE-ENTRANT CORNERS. REFER TO TYPICAL SLAB ON GROUND TRIMMER DETAILS FOR REINFORCEMENT. ALL TRIMMER BARS TO BE LAID UNDER TOP REINFORCEMENT (FABRIC).
 - PROVIDE FILLER AND APPROVED SEALANT WITH BACKING ROD AROUND ALL COLUMNS, WALLS AND PANELS THAT PENETRATE THE SLAB. REFER TO TYPICAL JOINT DETAILS.
 - ENSURE ALL TILED SURFACES AND MASONRY WALLS ARE JOINTED ACROSS CONCRETE SLAB JOINTS
 - JOINT PATTERN MAY BE ALTERED TO SUIT CONSTRUCTION SEQUENCE. ADVISE ENGINEER OF PROPOSED CHANGES PRIOR TO POURING CONCRETE.

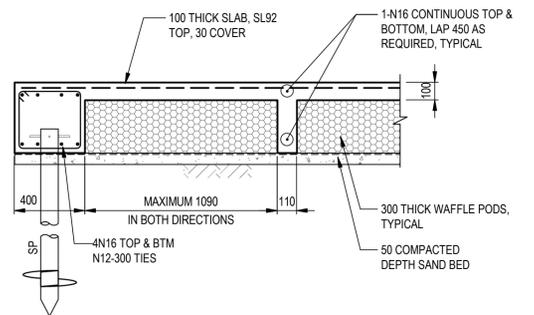
- SLAB ON GRADE PLAN LEGEND**
- 200 SLAB THICKNESS
 - EB_ EDGE BEAM, REFER TO DETAILS
 - IB_ INTERNAL BEAM, REFER TO DETAILS
 - ET_ SLAB THICKENING REFER TO DETAILS
 - C_ CONCRETE COLUMN OVER, REFER TO COLUMN SCHEDULE AND DETAILS
 - SC_ STEEL COLUMN, REFER TO MEMBER SCHEDULE
 - LOADBEARING PILE UNDER PILE CAP / FOOTING, REFER TO DETAILS
 - SD_ SETDOWN IN SLAB SURFACE TO BE CONFIRMED BY THE ARCHITECT
 - 600 x 1200 BEAM DENOTES SIZE OF BEAM (DEPTH x WIDTH)

CONCRETE COLUMN SCHEDULE

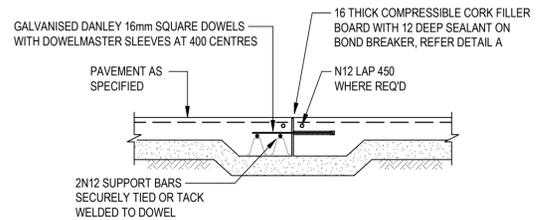
Mark	Member Size	Description
C1	400x400	CONCRETE COLUMN

REINFORCED WALL:

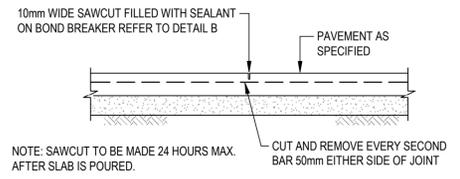
- RFW1 : 350 REINFORCED BRICKWALL



WAFFLE SLAB EDGE BEAM DETAIL
SCALE 1:20



DOWELLED EXPANSION JOINT (DEJ) (TYPICAL UNO)
SCALE 1:20



SAWN JOINT (SJ) (TYPICAL)
SCALE 1:20

BLOCK M GROUND FLOOR GENERAL ARRANGEMENT PLAN
SCALE 1 : 100

- NOTE:**
- GROUND FLOOR SLAB TO BE RAFT SLAB IN ACCORDANCE WITH AS2870 FOR A CLASS P SITE. THIS WILL COMPRISE OF A 400mm DEEP RAFT WAFFLE SLAB WITH SCREW PILES.
 - ALL COLUMNS AND LOAD BEARING WALLS TO BE PILED USING SCREWED PILES. PILES TO BE FOUNDED IN MEDIUM DENSE SAND, ESTIMATED AT 6m DEPTH BELOW GROUND LEVEL, REFER GEOTECHNICAL INVESTIGATION REPORT BY MARTENS, REF P2108205JR2V01
 - CONCRETE STRENGTH, $f_c = 40$ MPa.

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6	04-07-2022	RE-ISSUED FOR SCHEMATIC DESIGN	AK	PP	
5	02-07-2022	RE-ISSUED FOR SCHEMATIC DESIGN	AK	PP	
4	23-06-2022	RE-ISSUED FOR SCHEMATIC DESIGN	AK	PP	
3	01-06-2022	ISSUED FOR SCHEMATIC DESIGN	AK	PP	
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1	12-05-2022	ISSUED FOR SCHEMATIC DESIGN	AK	PP	

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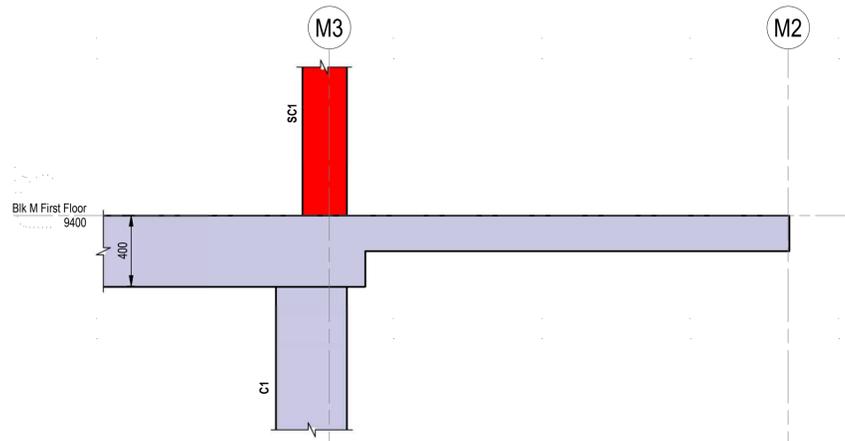
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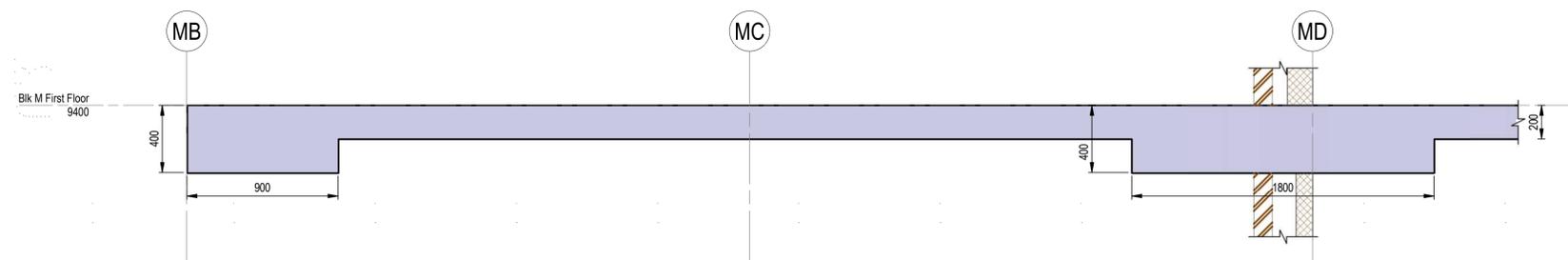
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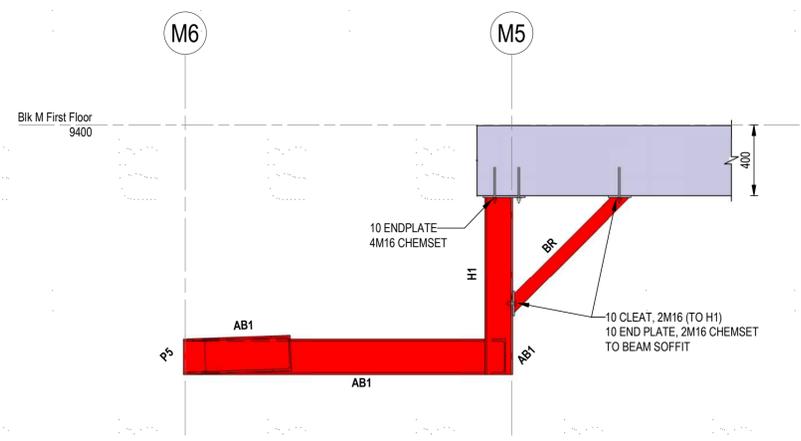
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Date	Project	CRONULLA HIGH SCHOOL
Date	Status	SCHEMATIC DESIGN
Date	NOT TO BE USED FOR CONSTRUCTION PURPOSES	
Date	Datum	Date Scale Size
Date		1:100 A1
Date	Drawing Number	Revision
Date	BLOCK M GROUND FLOOR G.A. PLAN	80821341-ST-0221 6



SECTION **ML1-1**
SCALE 1:20
ST-0241



SECTION **ML1-2**
SCALE 1:20
ST-0241



SECTION **ML1-3**
SCALE 1:20
ST-0241

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4	04-07-2022	RE-ISSUED FOR SCHEMATIC DESIGN	AK	PP	
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Project	CRONULLA HIGH SCHOOL		
Status	SCHEMATIC DESIGN		
NOT TO BE USED FOR CONSTRUCTION PURPOSES			
Datum	Date	Scale	Size
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Drawing Number	80821341-ST-0251		Revision
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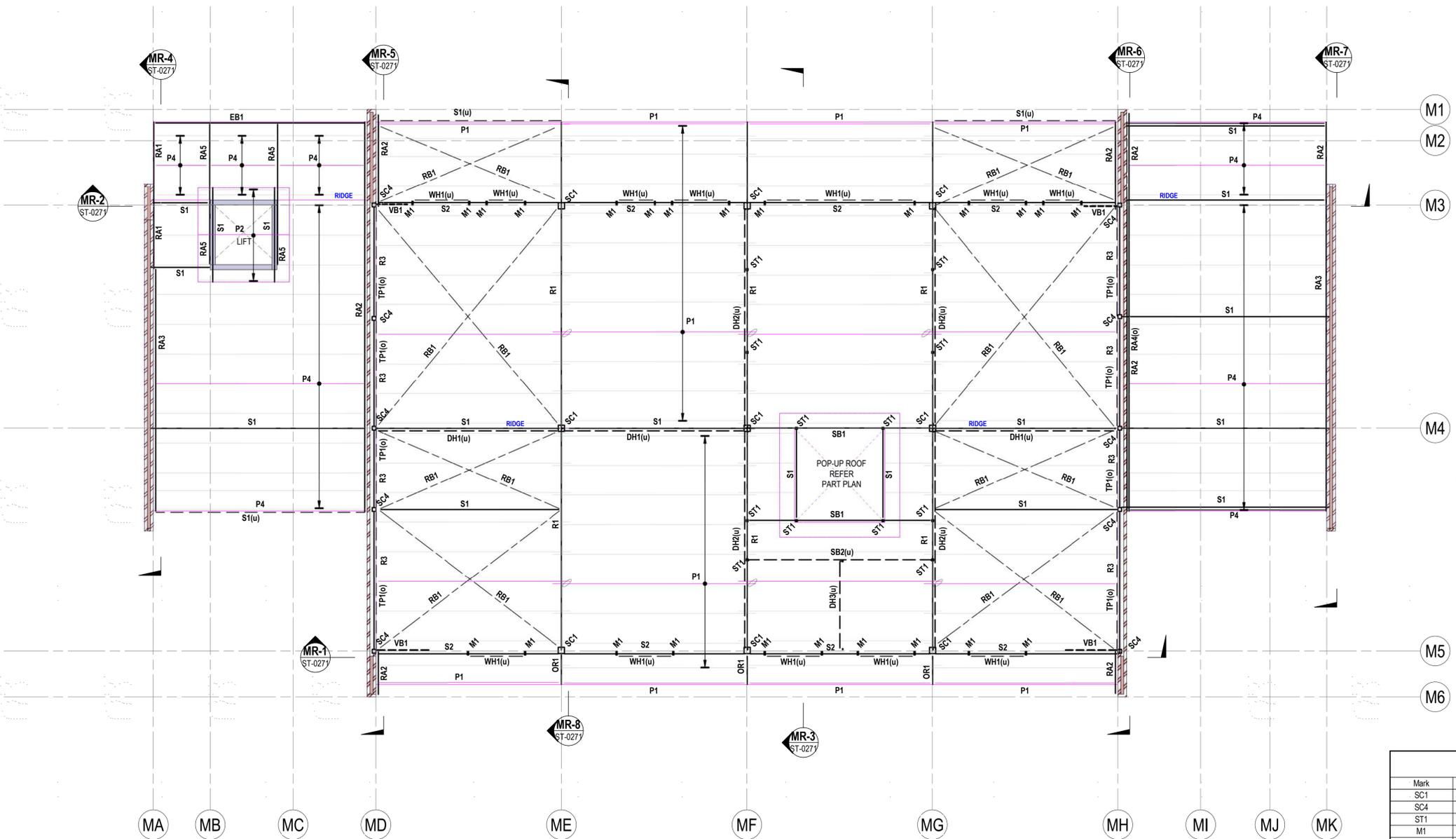
BLOCK M FIRST FLOOR SECTIONS

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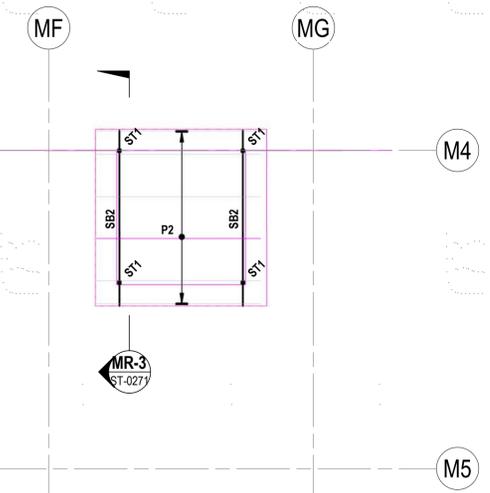


- ROOF FRAMING PLAN NOTES**
- FOR CONSTRUCTION NOTES REFER TO DRAWING ST-0001
 - FOR BUILDING SETOUT AND DIMENSIONS REFER TO ARCHITECTS DRAWINGS
 - FOR ALL ROOF PENETRATIONS REFER TO ARCHITECT AND SERVICE ENGINEERS DRAWINGS
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 - CONTRACTOR SHALL GIVE DUE CONSIDERATION TO ACCURACY OF FABRICATION AND PLANNING OF ERECTION PROCEDURE
 - FOR FIREPROOFING REQUIREMENTS REFER TO ARCHITECTS DRAWINGS

- LEGEND:**
- (u) : DENOTES MEMBER UNDER
 - (o) : DENOTES MEMBER OVER
 - (b) : DENOTES MEMBER BEYOND



M1
M2
M3
M4
M5
M6



BLOCK M POP-UP ROOF STEEL PLAN
SCALE 1 : 100

BLOCK M ROOF FRAMING PLAN
SCALE 1 : 100

STEEL ROOF MEMBER SCHEDULE (BLOCK M)			
Mark	Member Size	MemberDescription_CDD	Comments
SC1	250x250x9 SHS	STEEL COLUMN	
SC4	150x150x5 SHS	STEEL COLUMN	
ST1	75x75x3 SHS	STUB COLUMN	
M1	150x50x4 RHS	MULLION	
R1	460UB67	RAFTER	FLY BRACING AT EVERY 2nd PURLIN
R3	310UB32	RAFTER	
EB1	200 PFC	STEEL BEAM	IN PURLIN DEPTH
SB1	200 PFC	STEEL BEAM	
SB2	200 PFC	STEEL BEAM	
RA1	200 PFC	RACKING BEAM	
RA2	200 PFC	RACKING BEAM	TOES DOWN
RA3	150 PFC	RAFTER	
RA4	150x90x8 UA	RACKING ANGLE	LOWER ROOF (FIRST FLOOR)
RA5	300PFC	RACKING BEAM	
OR1	TW 100x200	OUTRIGGER	
S1	89x89x5 SHS	STRUT	
S2	200 PFC	STEEL BEAM	TOES DOWN
DH1	310UB40	DOOR HEADER	
DH2	200 PFC	DOOR HEADER	
DH3	200 PFC	DOOR HEADER	
WH1	200 PFC	WINDOW HEAD	TOES UP
P1	Z20015	PURLIN	@ 1200 MAX CTS. 900 AT END SPANS. LAP 900 OVER SUPPORTS, 2 ROWS OF BRIDGING
P2	Z15015	PURLIN	@ 1200 MAX CTS. 900 AT END SPANS. LAP 900 OVER SUPPORTS, 2 ROWS OF BRIDGING
P4	C20015	PURLIN	@ 1200 MAX CTS. 900 AT END SPANS. LAP 900 OVER SUPPORTS, 2 ROWS OF BRIDGING
TP1	C20015	PURLIN	GIRT (PARAPET) TOES DOWN
RB1	20 DIA ROD	ROOF BRACING	
VB1	150x100x5 RHS	VERTICAL BRACING	

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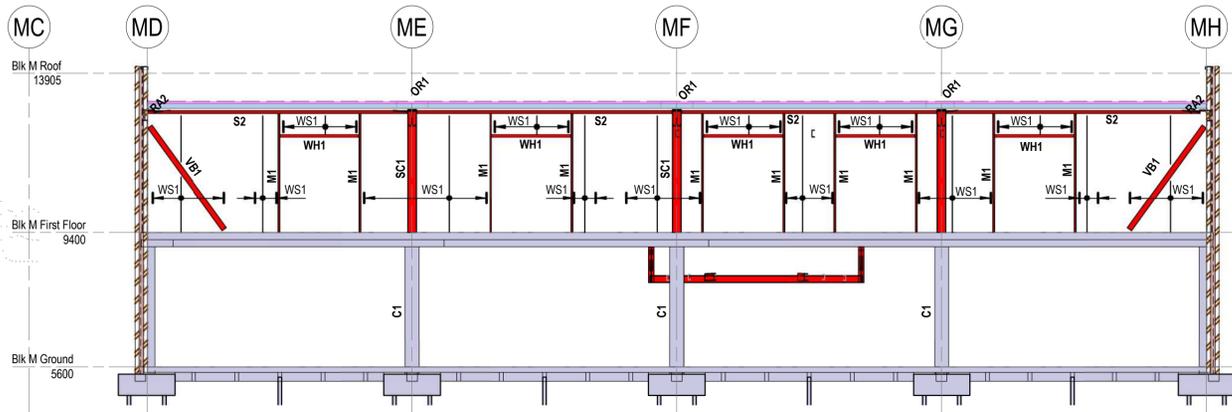
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Date Client **NSW DEPARTMENT OF EDUCATION (SCHOOLS INFRASTRUCTURE)**
Date Project **CRONULLA HIGH SCHOOL**
Date Status **SCHEMATIC DESIGN**
Date Title **BLOCK M ROOF FRAMING PLAN**
Date Drawing Number **80821341-ST-0261**
Date Revision **6**

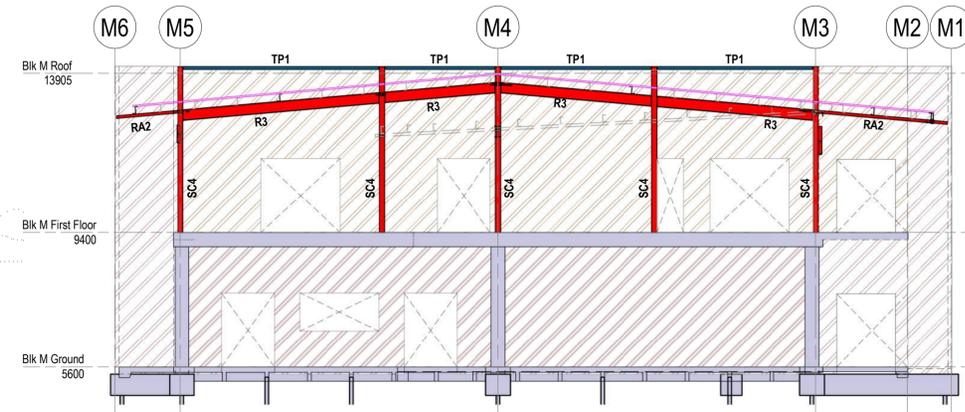
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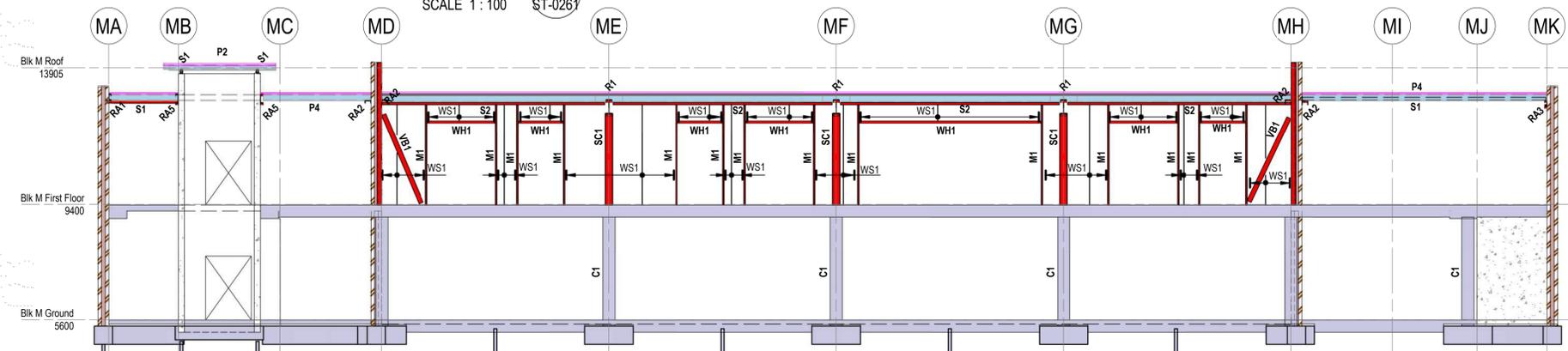
NOTE:
 • FOR CONSTRUCTION NOTES REFER TO DRAWING ST-0001
 • FOR STEEL ROOF MEMBER SCHEDULE REFER TO ST-0261
 • WS1 : RONDO OR SIMILAR APPROVED WALL STUDS



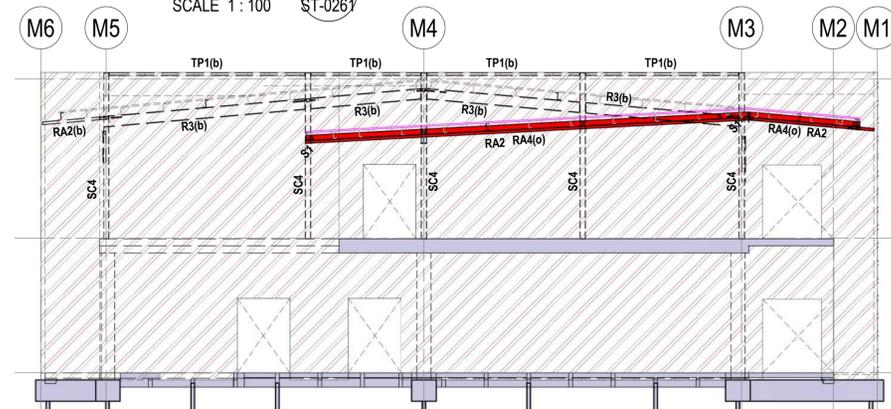
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ST-0261



SECTION MR-5
SCALE 1 : 100
ST-0261

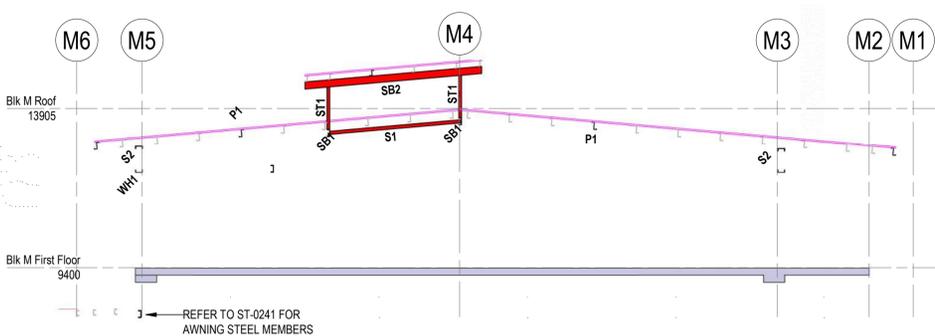


SECTION MR-2
SCALE 1 : 100
ST-0261

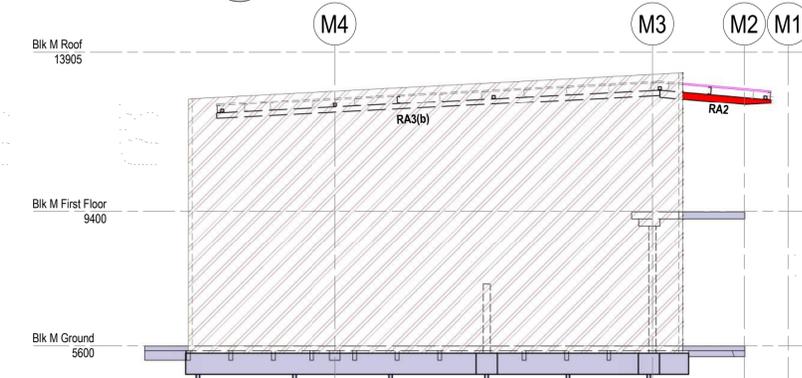


SECTION MR-6
SCALE 1 : 100
ST-0261

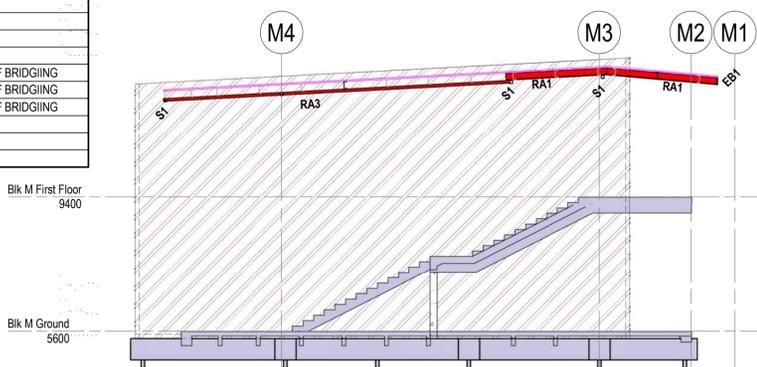
STEEL ROOF MEMBER SCHEDULE (BLOCK M)			
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SC4	150x150x5 SHS	STEEL COLUMN	
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SB1	200 PFC	STEEL BEAM	
SB2	200 PFC	STEEL BEAM	
RA1	200 PFC	RACKING BEAM	
RA2	200 PFC	RACKING BEAM	TOES DOWN
RA3	150 PFC	RAFTER	
RA4	150x90x8 UA	RACKING ANGLE	LOWER ROOF (FIRST FLOOR)
RA5	300PFC	RACKING BEAM	
OR1	TW 100x200	OUTRIGGER	
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S2	200 PFC	STEEL BEAM	TOES DOWN
DH1	310UB40	DOOR HEADER	
DH2	200 PFC	DOOR HEADER	
DH3	200 PFC	DOOR HEADER	
WH1	200 PFC	WINDOW HEAD	TOES UP
P1	220015	PURLIN	@ 1200 MAX CTS. 900 AT END SPANS. LAP 900 OVER SUPPORTS. 2 ROWS OF BRIDGING
P2	215015	PURLIN	@ 1200 MAX CTS. 900 AT END SPANS. LAP 900 OVER SUPPORTS. 2 ROWS OF BRIDGING
P4	C20015	PURLIN	@ 1200 MAX CTS. 900 AT END SPANS. LAP 900 OVER SUPPORTS. 2 ROWS OF BRIDGING
TP1	C20015	PURLIN	GIRT (PARAPET) TOES DOWN
RB1	20 DIA ROD	ROOF BRACING	
VB1	150x100x5 RHS	VERTICAL BRACING	



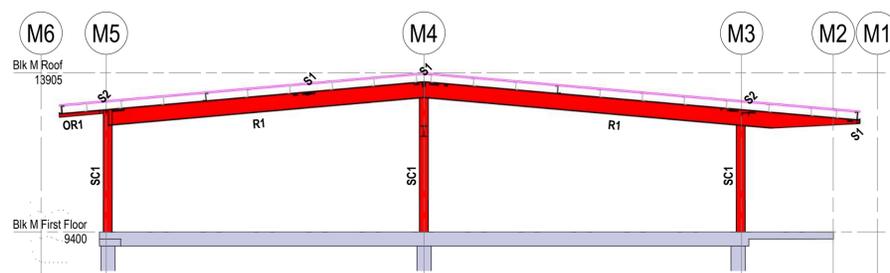
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SCALE 1 : 100
ST-0261



SECTION MR-7
SCALE 1 : 100
ST-0261



SECTION MR-4
SCALE 1 : 100
ST-0261



SECTION MR-8
SCALE 1 : 100
ST-0241

- ROOF FRAMING PLAN NOTES**
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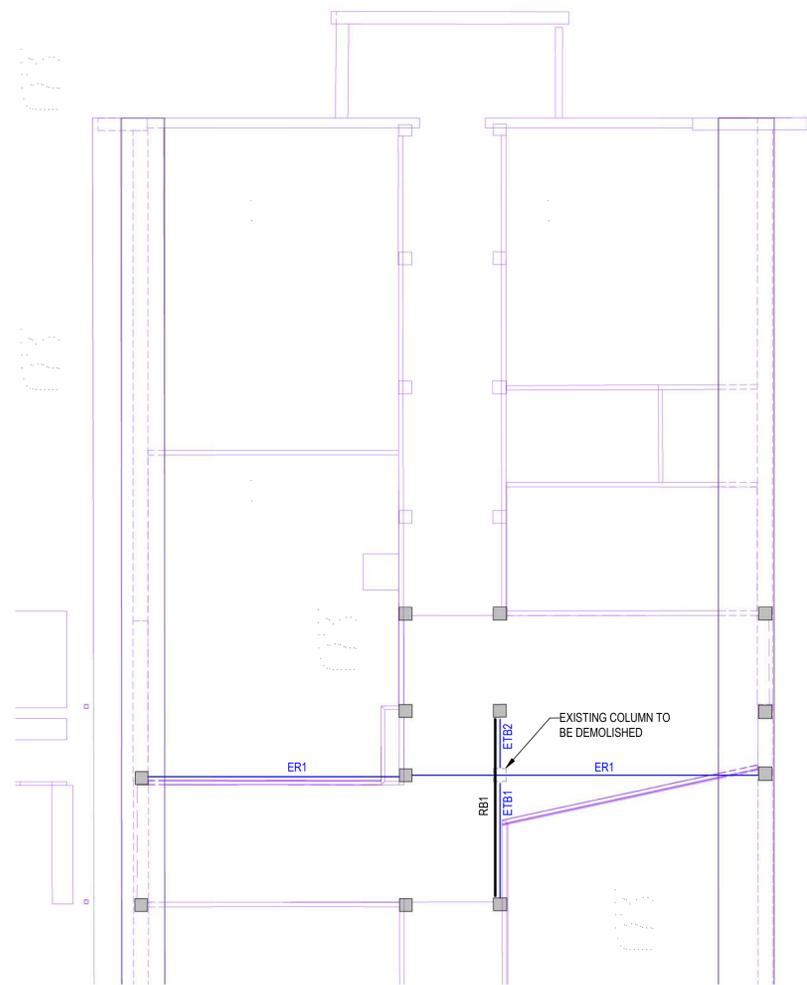
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Date	Project	CRONULLA HIGH SCHOOL
Date	Status	SCHEMATIC DESIGN
Date	NOT TO BE USED FOR CONSTRUCTION PURPOSES	
Date	Title	STEEL FRAMING ELEVATION
Date	Drawing Number	80821341-ST-0271
Date	Revision	6



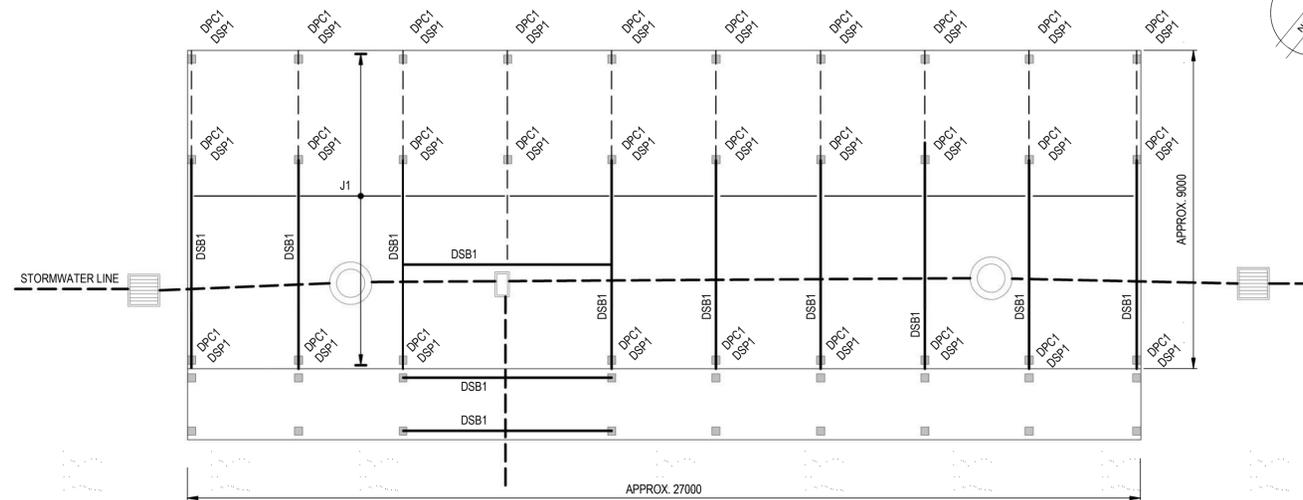
BLOCK D FIRST FLOOR
SCALE 1 : 100

NOTE:

- ETB1, ETB2 : EXISTING TIE BEAM - 150x76 UB
- ER1 : EXISTING RAFTER - 228x102 UB, WELD PROFILED 75x4 SHS, 100mm LONG WITH 10 BASE PLATE, 4M16 TO RB1
- RB1 : ROOF BEAM - 250UB31, 10 END PLATE, 4M16 CHEMSET TO EXISTING COLUMN

METHOD STATEMENT FOR DEMOLITION OF EXISTING COLUMN:

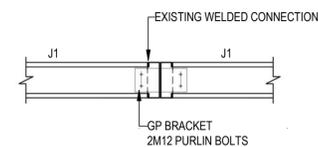
1. PROP EXISTING RAFTER ER1 WITH 2.5t SWL PROPS EACH SIDE OF COLUMN.
2. PROP EXISTING TIE BEAMS ETB1 AND ETB2 WITH 2t SWL EACH SIDE OF COLUMN.
3. ARRANGE FOR INSPECTION BY ENGINEER.
4. DEMOLISH COLUMN.
5. CONNECT ETB1 AND ETB2 TO ER1 WITH 10 WELDED CLEATS.
6. ARRANGE FOR INSPECTION BY ENGINEER.
7. REMOVE PROPS TO ETB1 AND ETB2.
8. INSTALL RB1.



FOOTING PLAN

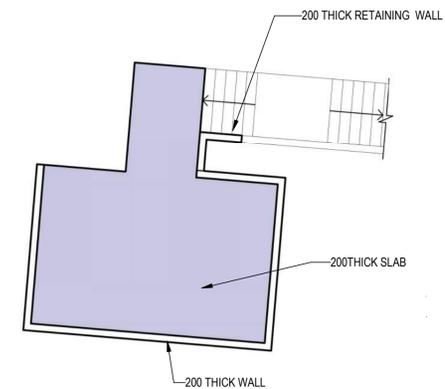
- NOTE:**
- DPC1 : 600x600x400 PILE CAP WITH 230x230 BRICK PIER
 - DSP1 : SCREW PILE WITH SWL= 50 kN
 - DSB1 : 310UB32
 - DSB2 : 310UB46

BLOCK I DEMOUNTABLE PLAN
SCALE 1 : 100



- NOTES:**
1. ALL CORRODED STEEL SECTIONS SHALL BE REPLACED.
 2. ALL C PURLIN TO PURLIN CONNECTIONS SHALL BE RECTIFIED USING BOLTED CONNECTIONS AS SHOWN.

SUB-FLOOR FRAMING RECTIFICATION DETAIL
SCALE 1 : 20



SUBSTATION PLAN
SCALE 1 : 100

Rev	Date	Description	Des.	Ver.	Appr.
2	04-07-2022	RE-ISSUED FOR SCHEMATIC DESIGN	AK	PP	
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Approved	Date

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Project	CRONULLA HIGH SCHOOL		
Status	SCHEMATIC DESIGN		
NOT TO BE USED FOR CONSTRUCTION PURPOSES			
Datum	Date	Scale	Size
		1:100	A1
Drawing Number	80821341-ST-0301		Revision
			2
BLOCK D FIRST FLOOR, BLOCK I & SUBSTATION PLAN			

DATE PLOTTED: 4/07/2022 7:14:02 PM

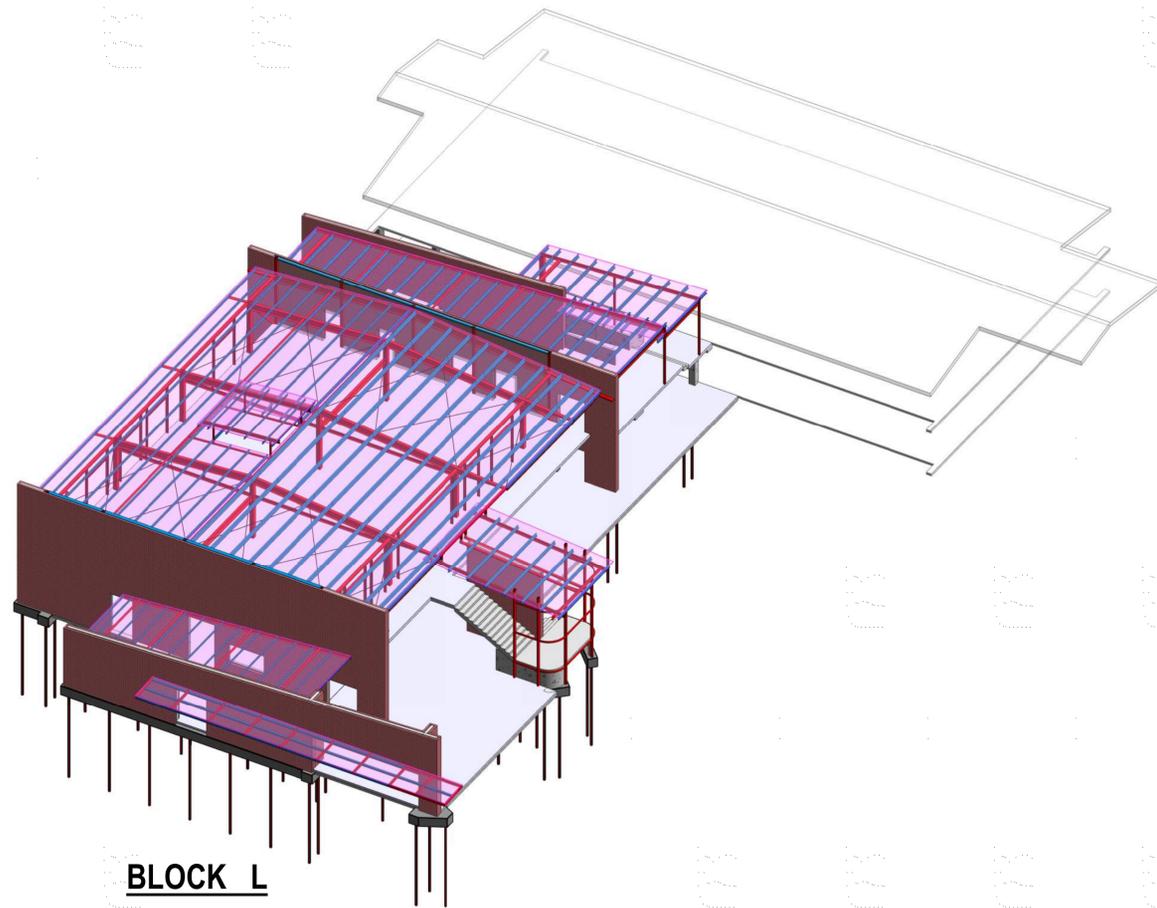


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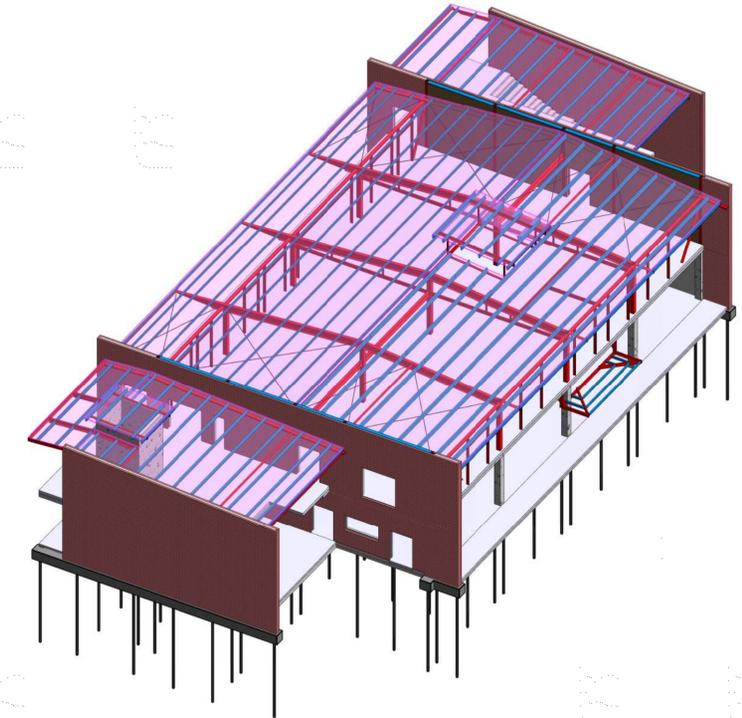
NSW DEPARTMENT OF EDUCATION (SCHOOLS INFRASTRUCTURE)

CRONULLA HIGH SCHOOL

COVER SHEET



BLOCK L



BLOCK M

DRAWING INDEX	
DRG. No.	DRAWING NAME
ST-0000	COVER SHEET
ST-0001	STRUCTURAL CONSTRUCTION NOTES
ST-0101	BLOCK L FOUNDATION PLAN
ST-0121	BLOCK L GROUND FLOOR G.A. PLAN
ST-0141	BLOCK L FIRST FLOOR G.A. PLAN
ST-0151	BLOCK L FIRST FLOOR SECTIONS
ST-0161	BLOCK L ROOF FRAMING PLAN
ST-0171	STEEL FRAMING ELEVATION
ST-0201	BLOCK M FOUNDATION PLAN
ST-0221	BLOCK M GROUND FLOOR G.A. PLAN
ST-0241	BLOCK M FIRST FLOOR G.A. PLAN
ST-0251	BLOCK M FIRST FLOOR SECTIONS
ST-0261	BLOCK M ROOF FRAMING PLAN
ST-0271	STEEL FRAMING ELEVATION
ST-0301	BLOCK D FIRST FLOOR, BLOCK I & SUBSTATION PLAN

MODEL PATH: C:\Temp\Revit\80821341 - Cronulla HS - Phase 2 to 6 (Central 2019) - amsl.sabongari

Rev	Date	Description	Des.	Ver.	Appr.
5	04-07-2022	RE-ISSUED FOR SCHEMATIC DESIGN	AK	PP	
4	02-07-2022	RE-ISSUED FOR SCHEMATIC DESIGN	AK	PP	
3	01-06-2022	ISSUED FOR SCHEMATIC DESIGN	AK	PP	
2	20-05-2022	ISSUED FOR SCHEMATIC DESIGN	AK	PP	
1	12-05-2022	ISSUED FOR SCHEMATIC DESIGN	AK	PP	

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Drawn	AS	Date	Status
Checked <td>AK <td>Date</td> <td rowspan="5"> SCHEMATIC DESIGN NOT TO BE USED FOR CONSTRUCTION PURPOSES </td> </td>	AK <td>Date</td> <td rowspan="5"> SCHEMATIC DESIGN NOT TO BE USED FOR CONSTRUCTION PURPOSES </td>	Date	SCHEMATIC DESIGN NOT TO BE USED FOR CONSTRUCTION PURPOSES
Designed <td>AK <td>Date</td> </td>	AK <td>Date</td>	Date	
Verified <td>PP <td>Date</td> </td>	PP <td>Date</td>	Date	
Approved <td> <td>Date</td> </td>	<td>Date</td>	Date	
		Date	

Datum	Date	Scale	Size
			A1

Drawing Number	Revision
80821341-ST-0000	5

GENERAL NOTES

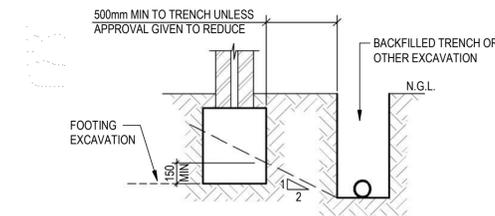
- THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL OTHER CONSULTANTS DRAWINGS, SPECIFICATIONS AND ADDITIONAL WRITTEN INSTRUCTIONS THAT MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. THE INFORMATION CONTAINED ON THESE DRAWINGS IS FOR STRUCTURAL ENGINEERING PURPOSES ONLY. ALL DISCREPANCIES THAT COULD RESULT IN CHANGES TO THE STRUCTURAL DETAILS SHALL BE REFERRED TO THE STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.
IF IN DOUBT, ASK.
- THE CONTRACTOR SHALL CHECK AND BE RESPONSIBLE FOR THE CORRECTNESS OF ALL DIMENSIONS AND ANY DISCREPANCY SHALL BE REPORTED IMMEDIATELY TO THE SUPERINTENDENT. DIMENSIONS SHALL NOT BE OBTAINED BY SCALING FROM THE DRAWINGS.
- IT IS THE BUILDER'S RESPONSIBILITY TO ENSURE THE SAFETY AND STABILITY OF NEW AND EXISTING STRUCTURES DURING CONSTRUCTION. TEMPORARY STRUCTURES, FORMWORK, FALSEWORK, TEMPORARY BRACING, SHORING AND THE LIKE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- THE CONTRACTOR SHALL NOTIFY THE ENGINEER **FORTY-EIGHT (48) HOURS** BEFORE THE REINFORCEMENT IS COMPLETED. THE CONTRACTOR SHALL ALLOW **TWO (2) HOURS** AFTER THE COMPLETION OF THE REINFORCEMENT FOR THE ENGINEER'S INSPECTION.
- ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE RELEVANT AND CURRENT AUSTRALIAN STANDARDS AND WITH THE BY-LAWS AND ORDINANCES OF THE RELEVANT BUILDING AUTHORITIES EXCEPT WHERE VARIED BY THE PROJECT SPECIFICATION.
- ALL WORK IS TO BE CARRIED OUT IN ACCORDANCE WITH ALL WORK-COVER REQUIREMENTS AND OCCUPATIONAL HEALTH AND SAFETY ACT REGULATIONS.
- NO CHANGES SHALL BE MADE WITHOUT THE WRITTEN CONSENT OF THE ENGINEER.
- CONSTRUCTION FROM THESE DRAWINGS AND ASSOCIATED CONSULTANTS' DRAWINGS SHALL NOT COMMENCE UNTIL APPROVED BY THE LOCAL AUTHORITIES AND PRINCIPAL CERTIFYING AUTHORITY. 'U.N.O.' OR 'UNO' DENOTES 'UNLESS NOTED OTHERWISE' ON THE DRAWINGS.
- ALL LEVELS ARE IN METRES AND ALL DIMENSIONS ARE IN MILLIMETRES U.N.O.

GROUND PREPARATION

- EXCAVATION AND GROUND PREPARATION SHALL BE CARRIED OUT IN ACCORDANCE WITH THE SPECIFICATION FOLLOWING THE RECOMMENDATIONS OF THE PROJECT GEOTECHNICAL REFERENCES AND ANY ADDITIONAL INSTRUCTIONS THAT MAY BE PROVIDED BY A GEOTECHNICAL ENGINEER DURING THE COURSE OF THE PROJECT.

FOUNDATION NOTES

- FOOTINGS HAVE BEEN DESIGNED FOR AN ALLOWABLE BEARING PRESSURE IN ACCORDANCE WITH THE **GEOTECHNICAL REFERENCES TABLE** ON THIS DRAWING. THIS FOUNDATION MATERIAL SHALL BE UNIFORM AND BE APPROVED BY THE ENGINEER FOR THIS PRESSURE BEFORE PLACING MEMBRANE, REINFORCEMENT OR CONCRETE.
- FOOTING EXCAVATIONS SHALL BE CLEARED TO REMOVE ALL LOOSE OR SOFTENED MATERIAL AND DEBRIS PRIOR TO PLACING OF CONCRETE.
- WHERE SKIN FRICTION IS REQUIRED TO BE DEVELOPED, CLEAN AND ROUGHEN THE SIDES OF THE EXCAVATION TO THE SATISFACTION OF THE ENGINEER.
- CONCRETE SHOULD BE POURED AS SOON AS POSSIBLE AFTER EXCAVATION. IF EXCAVATIONS ARE LIKELY TO REMAIN OPEN FOR **MORE THAN 24 HOURS** A BLINDING LAYER OF CONCRETE SHALL BE PLACED TO PROTECT THE FOUNDATION BASE.
- FOOTINGS SHALL BE CONCRETED ON THE DAY OF APPROVAL UNLESS PERMISSION IS GIVEN OTHERWISE.
- FOOTINGS SHALL BE LOCATED CENTRALLY UNDER WALLS AND COLUMNS U.N.O.
- DO NOT EXCEED NOMINATED **ANGLE OF REPOSE** BETWEEN ADJACENT FOOTINGS OR EXCAVATIONS.
- RESIDENTIAL SLABS & FOOTINGS HAVE BEEN DESIGNED IN ACCORDANCE WITH AS2870-2011 FOR THE **SITE CLASSIFICATION AND CONSTRUCTION TYPE** NOMINATED IN THE PROJECT **GEOTECHNICAL REFERENCES TABLE** ON THIS DRAWING.
- RETAINING WALLS OTHER THAN CANTILEVER WALLS SHALL NOT BE BACKFILLED UNTIL FLOOR CONSTRUCTION AT TOP AND BOTTOM IS COMPLETED. COMPACTION OF BACKFILL, IF REQUIRED, SHALL BE COMPLETED AS SPECIFIED ON DRAWINGS. ENSURE FREE-DRAINING BACKFILL MATERIAL AND DRAINAGE IS IN PLACE AS SPECIFIED ON THE DRAWINGS.
- FOOTING LEVELS, WHERE SHOWN ARE ESTIMATES ONLY AND WILL BE ESTABLISHED DURING SITE INSPECTION OF WORK IN PROGRESS.
- PRIOR TO ANY EXCAVATION NEAR EXISTING FOOTINGS, THE BUILDER SHALL DETERMINE THE DEPTH OF FOUNDINGS OF EXISTING FOOTINGS BY LOCAL INVESTIGATORY EXCAVATION. GENERAL EXCAVATION SHALL NOT PROCEED BELOW A LEVEL 150mm ABOVE THE UNDERSIDE OF EXISTING FOOTINGS STRICTLY UNTIL APPROVAL HAS BEEN PROVIDED BY THE ENGINEER.
- THE LIMITS OF EXCAVATIONS NEAR FOOTINGS SHALL BE AS SET OUT IN THE DETAIL BELOW UNLESS OTHERWISE APPROVED BY THE ENGINEER.



PROJECT GEOTECHNICAL REFERENCES

REPORT No. & REVISION	P2108205JR02V01
DATED	June 2021
PREPARED BY	MARTENS
FOUNDATION DESIGN PARAMETERS:	
REFER TO GEOTECHNICAL INVESTIGATION REPORT	

CONCRETE

- ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS3600, AS1379 & AS3610 CURRENT EDITIONS WITH AMENDMENTS, EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.
- FOR CONCRETE QUALITY REFER TO **CONCRETE QUALITY & COVER SUMMARY** TABLE ON THIS DRAWING. ALL CEMENT TO BE TYPE 'SL' SHRINKAGE-LIMITED CEMENT IN ACCORDANCE WITH AS3972-2010, EXCEPT THAT THE MAXIMUM SHRINKAGE OF THE CEMENT IN THE MORTAR TEST SAMPLE IN ACCORDANCE WITH AS2350 SHALL BE LESS THAN **600 MICROSTRAIN**.
- PROJECT ASSESSMENT SHALL BE CARRIED OUT IN ACCORDANCE WITH AS 1379:2007 CLAUSE B7.
 - ALL CONCRETE IN SLABS AND BEAMS TO BE PROPORTIONED TO LIMIT DRYING SHRINKAGE TO **650 MICROSTRAIN AT 56 DAYS**.
 - DETAILS OF THE PROPOSED MIX TO BE SUBMITTED & APPROVAL OBTAINED PRIOR TO POURING ANY CONCRETE.
 - SHRINKAGE TESTS SHALL BE CARRIED OUT BY AN APPROVED NATA REGISTERED LABORATORY IN ACCORDANCE WITH AS1012 PART 13. TESTS SHALL BE CONDUCTED ON THE FIRST BATCH OF CONCRETE USED IN SUSPENDED SLABS AND SUBSEQUENTLY AT THE RATE OF **ONE TEST EVERY ADDITIONAL 100 CUBIC METRES OF CONCRETE SUPPLIED**. THREE SPECIMENS SHALL BE TAKEN FOR EACH TEST AND THE SHRINKAGE SHALL BE THE AVERAGE OF THE THREE RESULTS. THE COST OF TESTING SHALL BE BORNE BY THE CONTRACTOR AS SHALL ANY ADDITIONAL TESTS REQUIRED IF THE CONCRETE FAILS TO MEET THE SPECIFIED SHRINKAGE LIMITS.
- NO ADMIXTURES OTHER THAN THOSE SPECIFIED IN THE **CONCRETE QUALITY & COVER SUMMARY** TABLE (OR APPROVED EQUIVALENTS) SHALL BE USED IN CONCRETE UNLESS APPROVED BY THE STRUCTURAL ENGINEER, WITH THE ONLY EXCEPTION BEING LOW RANGE WATER REDUCING ADMIXTURE.

CONCRETE CONTINUATION

- NO ADMIXTURES OTHER THAN THOSE SPECIFIED IN THE **CONCRETE QUALITY & COVER SUMMARY** TABLE (OR APPROVED EQUIVALENTS) SHALL BE USED IN CONCRETE UNLESS APPROVED BY THE STRUCTURAL ENGINEER, WITH THE ONLY EXCEPTION BEING LOW RANGE WATER REDUCING ADMIXTURE.
- CLEAR CONCRETE COVER TO REINFORCEMENT SHALL BE IN ACCORDANCE WITH **CONCRETE QUALITY & COVER SUMMARY** UNLESS NOTED OTHERWISE WHERE COVER MAY NEED TO BE INCREASED FOR FIRE RATING.
- COVER TO REINFORCEMENT IN THE FACE OF CONCRETE POURED OVER A VAPOUR PROOF MEMBRANE ON THE GROUND IS INCLUDED AS INTERNAL.
- CONDUITS, PIPES, ETC. SHALL ONLY BE LOCATED **IN THE MIDDLE THIRD OF THE SLAB DEPTH** AND SHALL NOT BE PLACED IN THE CONCRETE COVER TO REINFORCEMENT. NO HOLES OR CHASES OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ALLOWED WITHOUT THE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.
- CONCRETE CHAIRS DO NOT INCLUDE THE THICKNESS OF APPLIED FINISHES.
- THE DEPTH OF BEAMS IS GIVEN FIRST AND INCLUDES THE SLAB THICKNESS.
- CONSTRUCTION JOINTS, WHERE NOT SHOWN, SHALL BE LOCATED TO THE APPROVAL OF THE STRUCTURAL ENGINEER.
- CONSTRUCTION SUPPORT PROPPING IS TO REMAIN IN PLACE WHERE NEEDED TO AVOID OVERSTRESSING THE STRUCTURE DUE TO CONSTRUCTION LOADING. WHERE SLABS AND BEAMS ARE TO SUPPORT MASONRY WORK, FORMWORK AND PROPS MUST BE REMOVED PRIOR TO THE CONSTRUCTION OF MASONRY.
- ALL CONCRETE SHALL BE MECHANICALLY VIBRATED TO ACHIEVE A DENSE HOMOGENEOUS MASS, COMPLETELY FILLING THE FORMWORK THOROUGHLY EMBEDDING THE REINFORCEMENT AND FREE OF STONE POCKETS. ALL CONCRETE INCLUDING SLABS ON GROUND AND FOOTINGS SHALL BE COMPACTED WITH MECHANICAL VIBRATORS.
- CURING OF ALL CONCRETE IS TO BE ACHIEVED BY KEEPING SURFACES CONTINUOUSLY WET FOR A PERIOD OF **THREE (3) DAYS** AND THE PREVENTION OF LOSS OF MOISTURE FOR A TOTAL OF **SEVEN (7) DAYS** FOLLOWED BY A GRADUAL DRYING OUT. APPROVED SPRAY ON CURING COMPOUNDS THAT COMPLY WITH AS370 MAY BE USED WHERE FLOOR FINISHES WILL NOT BE AFFECTED (REFER MANUFACTURERS SPECIFICATION). PVA BASED CURING COMPOUNDS ARE NOT ACCEPTABLE. POLYTHENE SHEETING OR WET HESSIAN MAY BE USED IF PROTECTED FROM WIND AND TRAFFIC.
- REPAIRS TO CONCRETE SHALL NOT BE ATTEMPTED WITHOUT THE PERMISSION OF THE ENGINEER.
- CAST-IN FIXINGS, BOLTS ETC. SHALL NOT BE ALTERED WITHOUT THE PERMISSION OF THE ENGINEER.
- SLABS AND BEAMS SHALL BE CONSTRUCTED TO BEAR ONLY ON THE BEAMS, WALLS, COLUMNS ETC. SHOWN ON THE DRAWINGS. ALL OTHER BUILDING ELEMENTS SHALL BE KEPT **12mm CLEAR** OF SOFFITS OF STRUCTURE.

CONCRETE QUALITY & COVER SUMMARY

- MIN. CEMENT CONTENT = 300 kg/m³
- MAX. PERMISSIBLE DRYING SHRINKAGE = 600 MICROSTRAIN AT 56 DAYS.

STRUCTURAL ELEMENT	SLUMP (MAX)	MAX. AGG. SIZE	CEMENT TYPE	STRENGTH GRADE	ADMIXTURE	REBAR COVER	
						TOP	BTM
SLABS ON GROUND							
EXTERNAL	80	Ø20mm	SL	32MPa	PENETRON	40	40
INTERNAL	80	Ø20mm	SL	25MPa	-	30	30
SUSPENDED SLABS & BEAMS							
CONVENTIONAL							
EXTERNAL	80	Ø20mm	SL	32MPa	PENETRON	40	40
INTERNAL	80	Ø20mm	SL	32MPa	-	20	20
POST-TENSIONED							
EXTERNAL	80	Ø20mm	SL	40MPa	PENETRON	50	50
INTERNAL	80	Ø20mm	SL	40MPa	-	40	40
COLUMNS							
EXTERNAL	80	Ø20mm	SL	40MPa	-	-	40
INTERNAL	80	Ø20mm	SL	40MPa	-	-	30
LIFT & STAIR WALLS							
EXTERNAL	80	Ø20mm	SL	40MPa	-	-	40
INTERNAL	80	Ø20mm	SL	40MPa	-	-	30
BLOCK WALL CORE GROUT FILLING (U.N.O)							
	230	Ø10mm	SL	25MPa	-	-	55
BORED PIERS							
	80	Ø20mm	SL	32MPa	-	-	50
PAD/STRIP FOOTINGS							
	80	Ø20mm	SL	32MPa	-	-	50
LIFT PIT							
	80	Ø20mm	SL	40MPa	PENETRON	65	65

SCREW PILE NOTES

- ALL SCREW PILES ARE TO BE DESIGNED BY A SPECIALIST PILING CONTRACTOR AND ARE TO BE DESIGNED TO RESIST THE PILE LOADS SHOWN ON THE DESIGN DRAWINGS.
- FOR SITE AND PILE DESIGN PARAMETERS REFER TO FOUNDATION NOTES TABLE F.1 FOR GEOTECHNICAL INVESTIGATION REPORT.
- STEEL SCREW PILE DESIGN AND INSTALLATION IS TO COMPLY WITH THE REQUIREMENTS OF AS2159 AND AS410. DESIGN SHALL CONSIDER BUT NOT BE LIMITED TO ULTIMATE STRENGTH, SERVICEABILITY, DURABILITY AND SETTLEMENTS.
- THE TOP OF THE SCREW PILE SHALL BE POSITIVELY CONNECTED TO THE STRUCTURE OVER THE DESIGN OF THE CONNECTION IS THE RESPONSIBILITY OF PILING CONTRACTOR. THE DESIGN OF THE CONNECTION SHALL BE CAPABLE OF TRANSMITTING THE PILE LOADS SHOWN ON THE DESIGN DRAWINGS.
- FOR ULTIMATE LIMIT STATE DESIGN, THE GEOTECHNICAL STRENGTH REDUCTION FACTOR, ϕ_g , SHALL NOT EXCEED 0.40 UNLESS OTHERWISE STATED IN THE GEOTECHNICAL REPORT. SHOULD THE PILING CONTRACTOR ADOPT A VALUE HIGHER THAN 0.1, APPROVAL SHALL BE SORT FROM THE STRUCTURAL AND GEOTECHNICAL ENGINEERS.
- TORQUE TO LOAD CORRELATIONS ARE NOT DETERMINED SUITABLE FOR PILE CAPACITY CALCULATIONS. THE PILING CONTRACTORS SHALL SUBMIT CLAIMED VALUES FOR THE PILE DESIGN FOR REVIEW BY CARDNO. FOR ALL RELEVANT LOAD CASES, STATIC TESTS SHALL BE UNDERTAKEN (MINIMUM OF ONE TEST TO CONFIRM COMPRESSION AND TENSION CAPACITY). THE GEOTECHNICAL STRENGTH REDUCTION FACTOR, ϕ_g , IN ULTIMATE LIMIT STATE DESIGN SHALL BE APPLIED TO THE MAXIMUM TEST LOAD, IF THE TEST IS NOT TAKEN TO FAILURE AS REQUIRED BY AS2159. THE GEOTECHNICAL STRENGTH REDUCTION FACTOR CAN BE INCREASED DEPENDING ON PERCENTAGE OF STATIC LOAD TESTS CONDUCTED. REFER TO AS2159.
- CORROSION ALLOWANCE FOR STEEL PILES TO APPLY TO ALL EXPOSED STEEL SURFACES FOR A DESIGN LIFE OF 50 YEARS UNLESS STATED OTHERWISE. ANY SURFACE COATING APPLIED TO THE PILE SHALL BE IGNORED WHEN CALCULATING CORROSION ALLOWANCE AS THE INTEGRITY OF THE COATING CANNOT BE GUARANTEED DURING THE INSTALLATION PROCESS.
- INSTALLATION TORQUES TO BE MONITORED TO RECONCILE EXPECTED GEOTECHNICAL CONDITIONS AND THOSE ACTUALLY ENCOUNTERED DURING INSTALLATION.
- PILE LOAD TEST RESULTS TO BE SUBMITTED TO CARDNO FOR REVIEW AND APPROVAL.
- ALL TOLERANCES FOR THE PILES ARE TO BE WITHIN THE PERMISSIBLE LIMITS DEFINED BY AS2159, UNLESS NOTED OTHERWISE BY CARDNO.
- PILE SHAFTS TO BE COMPLETELY FILLED WITH 40MPa BLOCK CORE FILL MIX.
- AT THE COMPLETION OF THE PILING WORKS THE PILING CONTRACTOR IS TO ISSUE A CERTIFICATE THAT THE PILES HAVE BEEN DESIGNED AND INSTALLED IN ACCORDANCE WITH THE APPROPRIATE AUSTRALIAN STANDARDS AND CAN SUPPORT THE LOADS SPECIFIED ON THE DESIGN DRAWINGS FOR THE NOMINATED DESIGN LIFE OF THE STRUCTURE. CERTIFICATION IS TO BE PROVIDED BY AN APPROPRIATELY QUALIFIED ENGINEER.
- DETAIL ENGINEERING CALCULATIONS AND ENGINEERING CERTIFICATE BY AN NER REGISTERED ENGINEER SHALL BE PROVIDED TO THE PROJECT ENGINEER FOR REVIEW A MINIMUM OF 7 DAYS PRIOR TO COMMENCEMENT OF PILING.

REINFORCEMENT

- THE **REBAR DEVELOPMENT AND LAP SPLICE TABLES** ON THIS DRAWING HAVE BEEN CALCULATED IN ACCORDANCE WITH SECTION 13 OF AS3600-2009 FOR DEFORMED BARS.
- REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY, IT IS NOT NECESSARILY SHOWN IN TRUE PROJECTION.
- WELDING OF REINFORCEMENT SHALL NOT BE PERMITTED UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.
- SPLICES IN THE MAIN REINFORCEMENT SHALL BE MADE ONLY IN THE POSITIONS SHOWN. SPLICES IN THE DISTRIBUTION REINFORCEMENT MAY BE POSITIONED AS NECESSARY WITH SPLICES OF SUFFICIENT LENGTH TO DEVELOP THE FULL STRENGTH OF THE BARS. MINIMUM LAPS TO FABRIC SHALL BE TO OVERLAP **TWO CROSS WIRES PLUS 50mm U.N.O.** REINFORCEMENT SHALL BE SECURELY TIED AT ALL LAPS AND INTERSECTIONS WITH **1.25mm BLACK ANNEALED WIRE**. THE WRITTEN APPROVAL OF THE ENGINEER SHALL BE OBTAINED FOR OTHER SPLICES - WHERE THE LAP LENGTH IS NOT SHOWN IT SHALL DEVELOP THE FULL STRENGTH OF THE REINFORCEMENT.
- ALL UNSUPPORTED BARS SHALL BE TIED IN A TRANSVERSE DIRECTION WITH **W2-300 UNO**. REINFORCEMENT SHALL BE SUPPORTED ON APPROVED CHAIRS NOT MORE THAN **600mm CENTRES BOTH WAYS** IN SLABS AND AT **1000mm CENTRES IN BEAMS** IN EXPOSURE CONDITIONS **B2 OR C** USE ONLY PLASTIC OR CONCRETE CHAIRS.
- ALL REINFORCEMENT SHALL BE FIRMLY SUPPORTED ON CHAIRS AT NOT MORE THAN **600mm CENTRES BOTH WAYS** IN SLABS, **800mm EACH WAY** FOR FABRIC, AND AT **1000mm CENTRES IN BEAMS** IN EXPOSURE CONDITIONS **B2 OR C**. WHEN POURED ON GROUND AS FORMWORK PROVIDE PLATES UNDER ALL BAR CHAIRS. PLASTIC TIPPED STEEL CHAIRS SHALL NOT BE USED ON EXPOSED FACES IN EXPOSURE CLASSIFICATION B1, B2 AND C, ONLY PLASTIC OR CONCRETE CHAIRS.
- STEEL REINFORCEMENT MATERIAL QUALITY TO AS4671:2007, DENOTED AS FOLLOWS:
 - SL = GRADE 500 DEFORMED WIRE REINFORCING SQUARE FABRIC OF DUCTILITY CLASS 'L'
 - RL = GRADE 500 DEFORMED WIRE REINFORCING RECTANGULAR FABRIC OF DUCTILITY CLASS 'L'
 - R = GRADE 280 ROUNDED BARS OF DUCTILITY CLASS 'N'
 - N = GRADE 500 DEFORMED BARS OF DUCTILITY CLASS 'N'
 - S = GRADE 250 DEFORMED BARS OF DUCTILITY CLASS 'N'
- TEST CERTIFICATES CONFIRMING COMPLIANCE TO AS4671 SHALL BE SUPPLIED TO THE STRUCTURAL ENGINEER.
- FABRIC SHALL BE SUPPLIED IN FLAT SHEETS, ROLLS WILL NOT BE ACCEPTED.
- TYPICAL REINFORCEMENT NOTATION SHALL BE INTERPRETED AS FOLLOWS:
 - '5N24-200' INDICATES '5' NUMBER OF BARS REQUIRED 'N' DENOTES GRADE OF REINFORCEMENT '24' DENOTES BAR DIAMETER IN MILLIMETRES '200' DENOTES MAXIMUM BAR SPACING IN MILLIMETRES
- TYPICAL BAR LAYING ABBREVIATIONS:
 - BTM = BARS IN BOTTOM LAYER
 - TOP = BARS IN TOP LAYER
 - ALT = BARS ALTERNATING
 - NF = BARS IN NEAR FACE
 - FF = BARS IN FAR FACE
 - EF = BARS IN EACH FACE
- FOR SLAB FALLS, CHAMFERS, REGLETS, DRIP GROOVES, ETC., REFER TO THE ARCHITECT'S DRAWINGS. MAINTAIN CONCRETE COVER AT THESE DETAILS.
- SITE BENDING OF REINFORCEMENT SHALL BE AVOIDED IF POSSIBLE. WHERE SITE BENDING IS UNAVOIDABLE IT SHALL BE CARRIED OUT COLD, WITHOUT THE APPLICATION OF HEAT, AND IN ACCORDANCE WITH THE PRACTICE NOTE **RPN1 OF THE STEEL REINFORCEMENT INSTITUTE OF AUSTRALIA**.
- LAPPED SPLICES SHALL NOT BE USED FOR BARS WITH DIAMETER LARGER THAN 40mm. CONTRACTOR/DETAILER TO EITHER WORK OUT OR REQUEST THE INCREASED VALUES FROM THE ENGINEER FOR SITUATIONS THAT LIE OUTSIDE THE ASSUMPTIONS.
- USE COMPRESSION LAP SPLICE LENGTH AT COLUMN AND WALL VERTICAL SPLICE LOCATIONS UNLESS INDICATED OTHERWISE ON PLANS, DETAILS OR SCHEDULES. USE TENSION SPLICE LENGTH FOR ALL OTHER SPLICES, WHERE SPLICING IS PERMITTED IN CODE.
- CONTRACTOR/DETAILER MAY SUBMIT LESSER SPLICE LENGTH FOR REVIEW AND APPROVAL.
- DEVELOPMENT LENGTHS AT SLAB FOLDS SHALL BE MEASURED IN ACCORDANCE WITH THE DIAGRAM ON THIS DRAWING.
- LAPPED SPLICE LENGTHS FOR HORIZONTAL BARS WITH MORE THAN 300 mm CONCRETE CAST BELOW THE BAR AND SPACED AT ≥ 150 mm CENTRES TO COMPLY WITH THE FOLLOWING UNO:

COVER	f _c	N12	N16	N20	N24	N28	N32
≥ 25	≥ 20	770	1150	1570	-	-	-
≥ 30	≥ 25	600	980	1350	1740	-	-
≥ 40	≥ 32	510	770	1100	1440	1810	2220
≥ 50	≥ 40	460	630	890	1200	1030	1890

- DO NOT INTERPOLATE INTERMEDIATE VALUES OF SPLICE LENGTHS. LAPPED SPLICE LENGTHS FOR BARS IN COLUMNS REFER TO AS3600 OR SUPERINTENDENT. EPOXY COATED BARS, BARS IN LIGHTWEIGHT CONCRETE AND SLIP FORMED CONCRETE WILL REQUIRE LONGER SPLICE LENGTHS. REFER TO AS3600 OR SUPERINTENDENT.

COVER	f _c	N12	N16	N20	N24	N28	N32
≥ 25	≥ 20	590	890	1210	-	-	-
≥ 30	≥ 25	490	750	1040	1340	-	-
≥ 40	≥ 32	390	660	840	1110	1400	1710
≥ 50	≥ 40	350	480	690	920	1180	1450

- NOT APPLICABLE FOR BARS IN COLUMNS. DO NOT INTERPOLATE INTERMEDIATE VALUES OF SPLICE LENGTHS. LAPPED SPLICE LENGTHS FOR BARS IN COLUMNS REFER TO AS3600 OR SUPERINTENDENT. EPOXY COATED BARS, BARS IN LIGHTWEIGHT CONCRETE AND SLIP FORMED CONCRETE WILL REQUIRE LONGER SPLICE LENGTHS. REFER TO AS3600 OR SUPERINTENDENT.

COVER	f _c	N12	N16	N20	N24	N28	N32
≥ 25	≥ 20	700	940	1190	-	-	-
≥ 30	≥ 25	620	830	1060	1300	-	-
≥ 40	≥ 32	540	700	900	1110	1340	1590
≥ 50	≥ 40	510	610	970	970	1170	1390

- DO NOT INTERPOLATE INTERMEDIATE VALUES OF SPLICE LENGTHS. LAPPED SPLICE LENGTHS FOR BARS IN COLUMNS REFER TO AS3600 OR SUPERINTENDENT. EPOXY COATED BARS, BARS IN LIGHTWEIGHT CONCRETE AND SLIP FORMED CONCRETE WILL REQUIRE LONGER SPLICE LENGTHS. REFER TO AS3600 OR SUPERINTENDENT.

COVER	f _c	N12	N16	N20	N24	N28	N32
≥ 25	≥ 20	700	940	1190	-	-	-
≥ 30	≥ 25	620	830	1060	1300	-	-
≥ 40	≥ 32	540	700	900	1110	1340	1590
≥ 50	≥ 40	510	610	970	970	1170	1390

- DO NOT INTERPOLATE INTERMEDIATE VALUES OF SPLICE LENGTHS. LAPPED SPLICE LENGTHS FOR BARS IN COLUMNS REFER TO AS3600 OR SUPERINTENDENT. EPOXY COATED BARS, BARS IN LIGHTWEIGHT CONCRETE AND SLIP FORMED CONCRETE WILL REQUIRE LONGER SPLICE LENGTHS. REFER TO AS3600 OR SUPERINTENDENT.

COVER	f _c	N12	N16	N20	N24	N28	N32
≥ 25	≥ 20	700	940	1190	-	-	-
≥ 30	≥ 25	620	830	1060	1300	-	-
≥ 40	≥ 32	540	700	900	1110	1340	1590
≥ 50	≥ 40	510	610	970	970	1170	1390

BRICKWORK AND BLOCKWORK

- ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS3700, CURRENT EDITION.
- UNCONFINED COMPRESSIVE STRENGTH AND TYPE OF MORTAR SHALL BE AS FOLLOWS:

ELEMENT	UNCONFINED COMPRESSIVE STRENGTH (f _{cu})	MORTAR MIX RATIO (CEMENT : LIME : SAND)
CLAY BRICKWORK	20 MPa	1 : 1 : 6
CONCRETE BLOCKWORK	15 MPa	1 : 0.25 : 3

NOTE: MIX RATIO IS BY VOLUME AND SHALL ALWAYS BE MEASURED WITH A BUCKET OR GAUGE BOX.

- MORTAR ADMIXTURES SHALL NOT BE USED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.
- NO CHASES OR RECESSES ARE PERMITTED IN LOAD BEARING MASONRY WITHOUT THE APPROVAL OF THE ENGINEER.
- WITH THE EXCEPTION OF REINFORCED CONCRETE BLOCKWORK, ALL MASONRY WHICH IS SUPPORTING SLABS AND BEAMS SHALL BE TROWELLED SMOOTH WITH MORTAR FILLING ALL VOIDS. TWO LAYERS OF ALCOHOL OR EQUIVALENT LOW-FRICTION METAL SLIP JOINT MATERIAL SHALL BE PLACED FULL WIDTH ACROSS SUCH LOAD BEARING SURFACES U.N.O. ON THE DRAWINGS.
- PROVIDE FULL DEPTH **10mm ABELFLEX OR EQUIVALENT** AGAINST ALL VERTICAL SURFACES WHICH ARE TO BE USED AS FORMWORK U.N.O.
- ALL RETAINING WALL BLOCKWORK SHALL BE **SERIES 20.90 OR 30.90 OPEN-ENDED BLOCKS** U.N.O.
- PROVIDE A **300mm WIDE** STRIP OF COARSE FREE DRAINING GRANULAR BACKFILL WRAPPED WITH **BIDIM A34 GEOTEXTILE FABRIC** OR APPROVED EQUIVALENT BEHIND ALL RETAINING WALLS. PROVIDE SUBSOIL DRAINAGE LINE WITH SUFFICIENT FALL TO AN APPROPRIATE OUTLET.
- PROVIDE CONTROL JOINTS IN BLOCKWORK RETAINING WALLS AT **6.0m MAX CENTRES U.N.O.**
- ALL CONTROL JOINTS IN BRICKWORK IN ACCORDANCE WITH **TECHNICAL NOTE 61 - AUG 2008 'BY CEMENT, CONCRETE & AGGREGATES AUSTRALIA** U.N.O.
- CHASES, HOLES, RECESSES ETC. SHALL NOT BE MADE IN LOAD BEARING MASONRY WALLS WITHOUT PRIOR APPROVAL FROM ENGINEER.
- ALL CLAY BRICKS USED EXTERNALLY MUST HAVE EXPOSURE CLASSIFICATION - REFER TO ARCHITECT FOR SPECIFICATIONS.
- ALL DOUBLE SKIN SOLID WALLS SUCH AS 230mm THICK BRICKWORK SHALL BE BONDED BY A **HEADER COURSE EVERY 4th COURSE**.
- NON LOAD BEARING WALLS BUILT PRIOR TO POURING CONCRETE SHALL BE SEPARATED FROM CONCRETE ABOVE BY **16mm THICK CLOSED CELL POLYSTYRENE STRIP**, WHERE BUILT AFTER CONCRETE IS POURED LEAVE **12mm CLEAR** OF CONCRETE SOFFIT.
- REFER TO CONCRETE NOTES FOR DE-PROPPING PRIOR TO CONSTRUCTION OF MASONRY WALLS ON SUSPENDED SLABS.
- BRICKWORK AND UNREINFORCED BLOCKWORK SHALL NOT BE ARTICULATED IN ACCORDANCE WITH **PART 3.3 OF THE NATIONAL CONSTRUCTION CODE**, UNLESS OTHERWISE DIRECTED BY ENGINEER, WHERE POSSIBLE WALL JOINTS ARE TO ALIGN WITH CONTROL JOINTS IN CONCRETE SLABS. JOINTS SHALL GENERALLY BE LOCATED A MAXIMUM 5m FROM WALL CORNERS.
- INSTALL APPROVED CAVITY TIES IN ACCORDANCE WITH AS3700 BETWEEN SKINS OF BRICKS, BETWEEN CONCRETE WALLS AND BETWEEN MASONRY WALLS AT NOT MORE THAN **600mm SPACING VERTICALLY** AND **500mm SPACING HORIZONTALLY** OVER WHOLE AREA OF WALLS. ABOVE GROUND EXTERIOR WALL TIES SHALL BE **GRADE 316 STAINLESS STEEL**.
- PROVIDE **1 ROW OF 'CLEAN-OUT' BLOCKS** TO ALL CORE-FILLED BLOCKWORK WALLS. ALL MORTAR AND DEBRIS IS TO BE REMOVED FROM CAVITY PRIOR TO PLACEMENT OF REINFORCEMENT AND CORE FILLING WITH GROUT. PROVIDE ADDITIONAL ROWS OF 'CLEAN-OUT' BLOCKS AT THE BASE OF EACH LIFT.
- CONCRETE COVER TO STEEL REINFORCEMENT WITHIN CORE-FILLED BLOCKWORK WALLS AS INDICATED INCLUDES THE WALL THICKNESS OF THE BLOCK. PROVIDE **65mm NOMINAL COVER** TO THE RETAINING FACE OF BLOCKWORK WALLS U.N.O.
- GROUT ALL CORES IN REINFORCED BLOCKWORK U.N.O. HEIGHT OF BLOCKWORK TO BE GROUTED ON ONE DAY SHALL BE **2400mm**. GROUT SHALL BE PLACED IN LIFTS OF **1200mm** MAXIMUM AND COMPACTED BY POKER VIBRATOR. ALLOW **1 HOUR** BETWEEN SUCCESSIVE LIFTS TO ALLOW PLASTIC SETTLEMENT PROJECTING FROM FOUNDATION OR SLABS INTO CORES. SHALL BE SET ACCURATELY IN PLACE USING TEMPLATES TO ALIGN WITH THE CENTRE OF THE LENGTH OF CORES AND WITH COVER AS NOTED. WHERE HORIZONTAL BARS ARE INDICATED, THE WEBS OF THE BLOCKS BELOW THE BARS SHALL BE CUT DOWN TO ACCOMMODATE THE BARS.

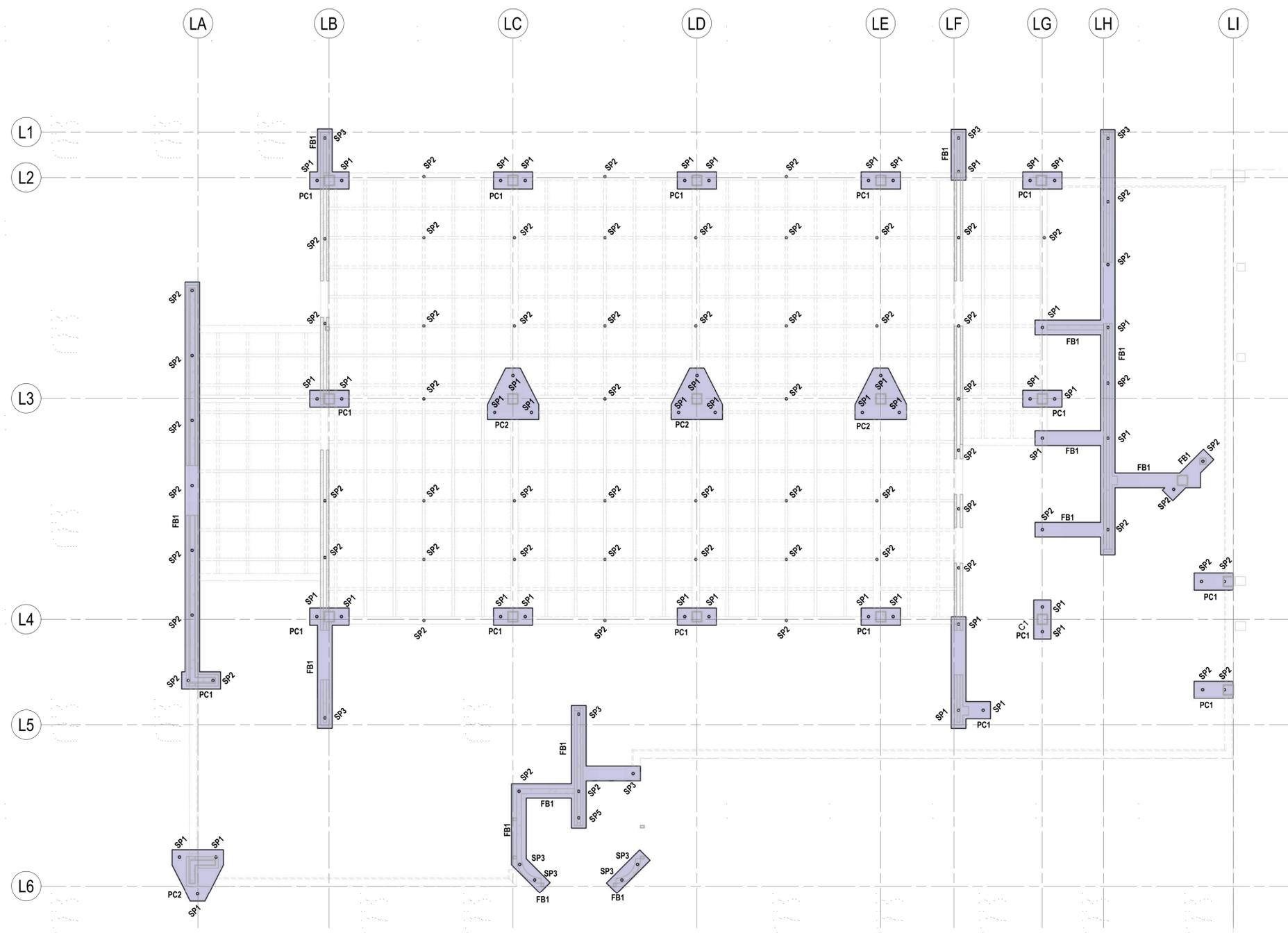
PROJECT-SPECIFIC DESIGN PARAMETERS:

- WHERE THERE IS A DISCREPANCY BETWEEN CONSTRUCTION NOTES AND PROJECT-SPECIFIC DESIGN PARAMETERS, THE TABLE BELOW SHALL TAKE PRECEDENCE.

DESIGN LOADINGS SUMMARY

DESIGN LIFE: 50 YEARS	
DESIGN LOADS (IN ACCORDANCE WITH AS/NZS1170.1:2002)	
BUILDING ELEMENTS/ZONES	LIVE LOAD
NON-TRAFFICABLE ROOF	0.25 kPa
STAIRS, CORRIDORS	4.0 kPa
SUSPENDED SLABS	5.00 kPa
DESIGN WIND LOADS (IN ACCORDANCE WITH AS/NZS1170.2:2011)	
• TERRAIN CATEGORY = 3 IMPORTANCE LEVEL = 3	

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- FOOTING PLAN NOTES**
- GENERAL**
- FOR CONSTRUCTION NOTES REFER TO DRAWING ST-001
 - FOR BUILDING SETOUT AND DIMENSIONS REFER TO ARCHITECTS DRAWINGS
- FOOTINGS**
- ALL FOOTING LEVELS TO BE CHECKED AGAINST IN-GROUND SERVICES
 - FOR ALLOWABLE BEARING PRESSURE REFER TO CONSTRUCTION NOTES DRAWING
 - BUILDER TO CONFIRM ALL TOP OF FOOTING LEVELS WITH THE ARCHITECT PRIOR TO POURING CONCRETE
 - THE ACTUAL DEPTH OF EACH FOOTING SHALL BE CHECKED AND APPROVED BY A GEOTECHNICAL ENGINEER
 - ALL CONCRETE COLUMNS TO BE PLACED CENTRALLY ON PAD FOOTING U.N.O.
 - ALL WATERPROOFING TO ARCHITECTS SPECIFICATIONS
- PILES/PIERS**
- ALL PILES/PIERS ARE TO BE LOCATED CENTRALLY UNDER COLUMNS AND WALLS U.N.O. REFER TO ARCHITECTS DRAWINGS FOR SETOUT
 - THE ACTUAL DEPTH OF EACH FOOTING SHALL BE CHECKED AND APPROVED BY A GEOTECHNICAL ENGINEER
 - ALL LOADS SHOWN ARE UNFACTORED
 - LOADS ARE TO BE USED FOR PILE/PIER DESIGN PURPOSES ONLY
 - PILE/PIER DESIGN TO COMPLY WITH AS2159
- EXISTING SERVICES**
- THE POSITION OF ALL EXISTING SERVICES SHOWN SHOULD BE REGARDED AS APPROXIMATE ONLY AND NOT NECESSARILY COMPREHENSIVE. IT IS THE CONTRACTORS RESPONSIBILITY TO DETERMINE THE EXACT LOCATIONS AND INFORM ALL AUTHORITIES PRIOR TO ANY EXCAVATION
- A. APPLICABLE TO ALL PAD FOOTING AND FOOTING BEAMS INCREASE PAD DEPTH WITH 15MPa MASS CONCRETE AS REQUIRED TO:**
- OBTAIN AN ALLOWABLE BEARING PRESSURE OF 500kPa TO GEOTECHNICAL ENGINEERS APPROVAL ON SITE
 - HAVE A LOAD DISPERSION LINE OF 45 FROM UNDERSIDE OF PAD TO CLEAR EXISTING SERVICES
 - ENSURE PAD IS FOUNDED AT UNDERSIDE OF ADJACENT SERVICES EXCAVATIONS

BLOCK L SCREW PILE SCHEDULE

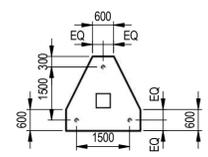
Mark	SWL (kN)
SP1	250
SP2	150
SP3	100

FOOTING BEAM SCHEDULE

MARK	SIZE		REINFORCEMENT	
	WIDTH (mm)	DEPTH (mm)	BARS	TIES
FB1	600	600	5N20 T&B	N12-300

CONCRETE PILE CAP FOOTING SCHEDULE (BLOCK L)

MARK	SIZE			REINFORCEMENT		
	WIDTH (mm)	LENGTH (mm)	DEPTH (mm)	BARS	TIES	PILES
PC1	700	1600	600	5N20 T&B	N12-300	2 x SP1
PC2	1500	1500	600	5N20 T&B	N12-300	3 x SP1



PC2 : PILES AT 1500 CENTRES AT 60 DEGREES

PILE CAP (PC2) DETAIL
SCALE 1:100

BLOCK L FOUNDATION PLAN
SCALE 1:100

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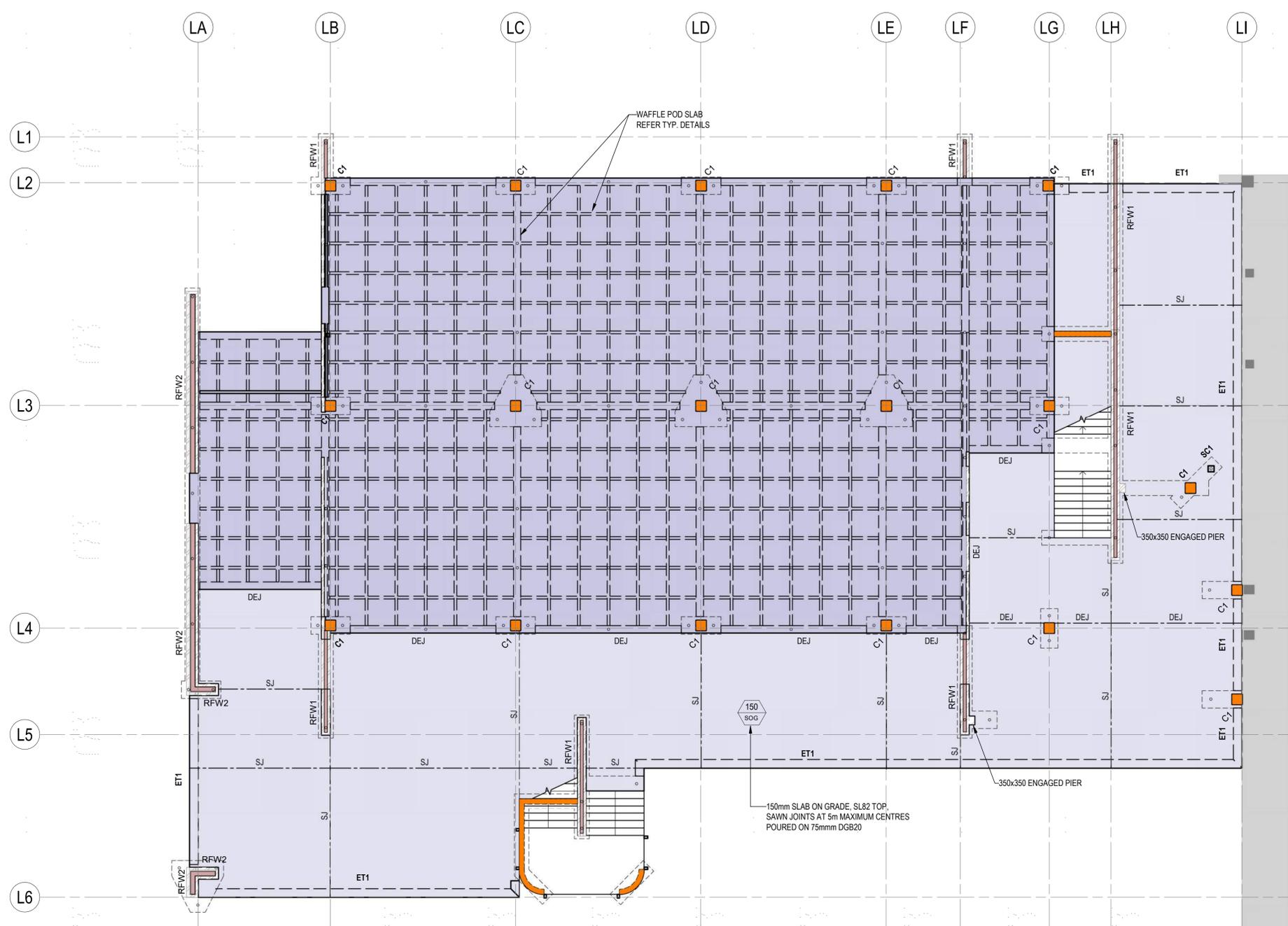
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Project	CRONULLA HIGH SCHOOL		
Status	SCHEMATIC DESIGN		
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BLOCK L GROUND FLOOR GENERAL ARRANGEMENT
SCALE 1:100

- NOTE:**
- GROUND FLOOR SLAB TO BE RAFT SLAB IN ACCORDANCE WITH AS2870 FOR A CLASS P SITE. THIS WILL COMPRISE OF A 400mm DEEP RAFT WAFFLE SLAB WITH SCREW PILES.
 - ALL COLUMNS AND LOAD BEARING WALLS TO BE PILED USING SCREWED PILES. PILES TO BE FOUNDED IN MEDIUM DENSE SAND, ESTIMATED AT 6m DEPTH BELOW GROUND LEVEL, REFER GEOTECHNICAL INVESTIGATION REPORT BY MARTENS, REF P2108205JR2V01
 - CONCRETE STRENGTH, $f_c = 40$ MPa.

CONCRETE COLUMN SCHEDULE		
Mark	Member Size	Description
C1	400x400	CONCRETE COLUMN

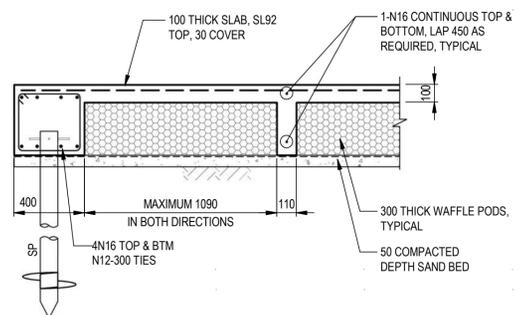
- REINFORCED WALL:**
- RFW1 : 350 WITH 130 RC WALL
 - RFW2 : 470 WITH 180 RC WALL

SLAB ON GRADE PLAN NOTES

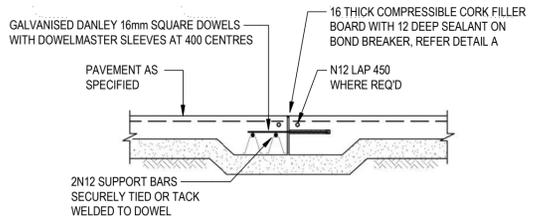
- GENERAL**
- FOR CONSTRUCTION NOTES & DRAWING LIST REFER TO DRAWING No ST-001
 - FOR BUILDING SETOUT AND DIMENSIONS REFER TO ARCHITECTS DRAWINGS
 - FOR SLAB FLOOR FINISHED REFER TO ARCHITECTS DRAWINGS.
 - FOR ALL SLAB PENETRATIONS REFER TO ARCHITECTS & BUILDING SERVICES DRAWINGS.
 - SURFACE FALLS, DRAINAGE & EMERGENCY OVERFLOWS IN EXTERNAL AREAS TO ARCHITECTS DETAILS.
 - ALL WATERPROOFING TO ARCHITECTS SPECIFICATION.
 - ALL FALLS TO ARCHITECTS DETAILS!
 - FOR ADDITIONAL REINFORCEMENT REFER TO PLANS, SECTIONS AND DETAILS.
 - PROVIDE TRIMMER BARS AROUND ALL PENETRATIONS, COLUMNS, JOINT INTERSECTIONS AND RE-ENTRANT CORNERS. REFER TO TYPICAL SLAB ON GROUND TRIMMER DETAILS FOR REINFORCEMENT. ALL TRIMMER BARS TO BE LAID UNDER TOP REINFORCEMENT (FABRIC).
 - PROVIDE FILLER AND APPROVED SEALANT WITH BACKING ROD AROUND ALL COLUMNS, WALLS AND PANELS THAT PENETRATE THE SLAB. REFER TO TYPICAL JOINT DETAILS.
 - ENSURE ALL TILED SURFACES AND MASONRY WALLS ARE JOINTED ACROSS CONCRETE SLAB JOINTS
 - JOINT PATTERN MAY BE ALTERED TO SUIT CONSTRUCTION SEQUENCE. ADVISE ENGINEER OF PROPOSED CHANGES PRIOR TO POURING CONCRETE.

SLAB ON GRADE PLAN LEGEND

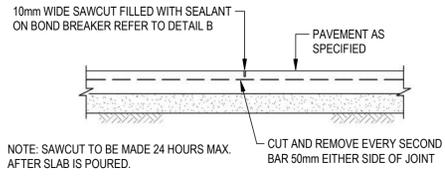
- SLAB THICKNESS
- EDGE BEAM, REFER TO DETAILS
- INTERNAL BEAM, REFER TO DETAILS
- SLAB THICKENING REFER TO DETAILS
- CONCRETE COLUMN OVER, REFER TO COLUMN SCHEDULE AND DETAILS
- STEEL COLUMN, REFER TO MEMBER SCHEDULE
- LOADBEARING PILE UNDER PILE CAP / FOOTING, REFER TO DETAILS
- SETDOWN IN SLAB SURFACE TO BE CONFIRMED BY THE ARCHITECT
- 600 x 1200 BEAM DENOTES SIZE OF BEAM (DEPTH x WIDTH)



WAFFLE SLAB EDGE BEAM DETAIL
SCALE 1:20



DOWELLED EXPANSION JOINT (DEJ) (TYPICAL UNO)
SCALE 1:20



SAWN JOINT (SJ) (TYPICAL)
SCALE 1:20

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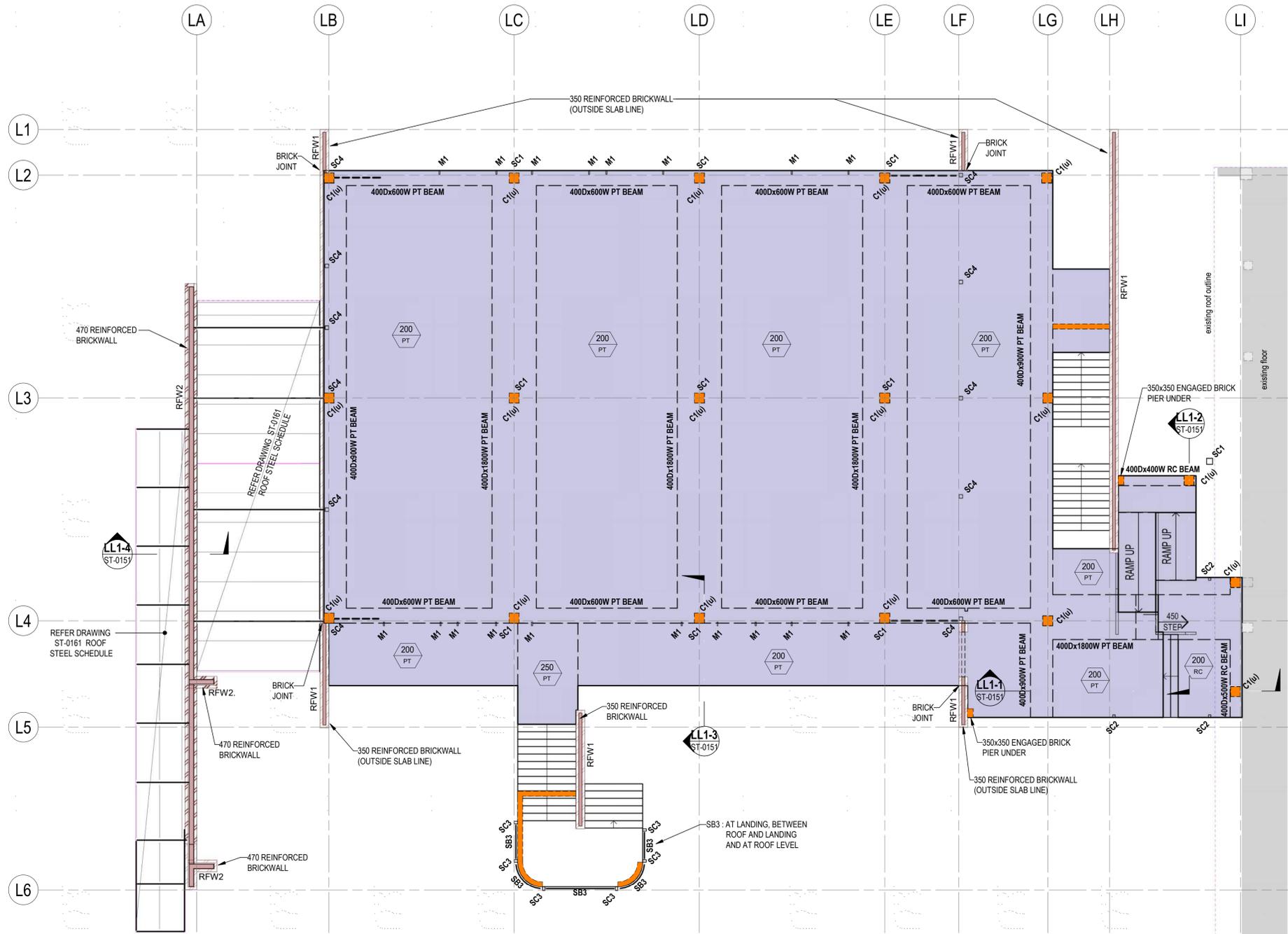
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Date	Client	NSW DEPARTMENT OF EDUCATION (SCHOOLS INFRASTRUCTURE)
Date	Project	CRONULLA HIGH SCHOOL
Date	Status	SCHEMATIC DESIGN
Date	Title	BLOCK L GROUND FLOOR G.A. PLAN

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		1:100	A1
Drawing Number	Revision		
80821341-ST-0121	7		



- SUSPENDED LAB PLAN NOTES**
- GENERAL**
- FOR CONSTRUCTION NOTES & DRAWING LIST REFER TO DRAWING No ST-001 & ST-002
 - FOR BUILDING SETOUT AND DIMENSIONS REFER TO ARCHITECTS DRAWINGS
- SLABS**
- FOR SLAB FLOOR FINISHED REFER TO ARCHITECTS DRAWINGS.
 - FOR ALL SLAB PENETRATIONS REFER TO ARCHITECTS & BUILDING SERVICES DRAWINGS.
 - SURFACE FALLS, DRAINAGE & EMERGENCY OVERFLOWS IN EXTERNAL AREAS TO ARCHITECTS DETAILS.
 - ALL WATERPROOFING TO ARCHITECTS SPECIFICATION.
 - ALL FALLS TO ARCHITECTS DETAILS/
 - FOR ADDITIONAL REINFORCEMENT REFER TO PLANS, SECTIONS AND DETAILS.

- SUSPENDED SLAB PLAN LEGEND**
- 200 SLAB THICKNESS
 - EB_ EDGE BEAM, REFER TO DETAILS
 - IB_ INTERNAL BEAM, REFER TO DETAILS
 - PL_ CONCRETE PLINTH REFER TO DETAILS
 - C CONCRETE COLUMN OVER, REFER TO COLUMN SCHEDULE AND DETAILS
 - S STEEL COLUMN, REFER TO MEMBER SCHEDULE
 - LOADBEARING PILE UNDER PILE CAP / FOOTING, REFER TO DETAILS
 - ▽ SETDOWN IN SLAB SURFACE TO BE CONFIRMED BY THE ARCHITECT
 - 600 x 1200 BEAM DENOTES SIZE OF BEAM (DEPTH x WIDTH)

STEEL COLUMN SCHEDULE		
Mark	Member Size	Description
M1	150x50x4 RHS	MULLION
SC1	250x250x9 SHS	STEEL COLUMN
SC2	100x100x6 SHS	STEEL COLUMN
SC4	150x150x5 SHS	STEEL COLUMN
ST2	100x100x4 SHS	STUB COLUMN

BLOCK L FIRST FLOOR GENERAL ARRANGEMENT
SCALE 1:100

- NOTES:**
- ALL CONCRETE COLUMN 400x400
 - ALL SLABS 200 THICK
 - ALL SLABS AND BEAMS TO BE POST-TENSIONED UNO
 - FOR STEEL MEMBER SCHEDULE REFER ST-0161

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4	01-06-2022	ISSUED FOR SCHEMATIC DESIGN	AK	PP	
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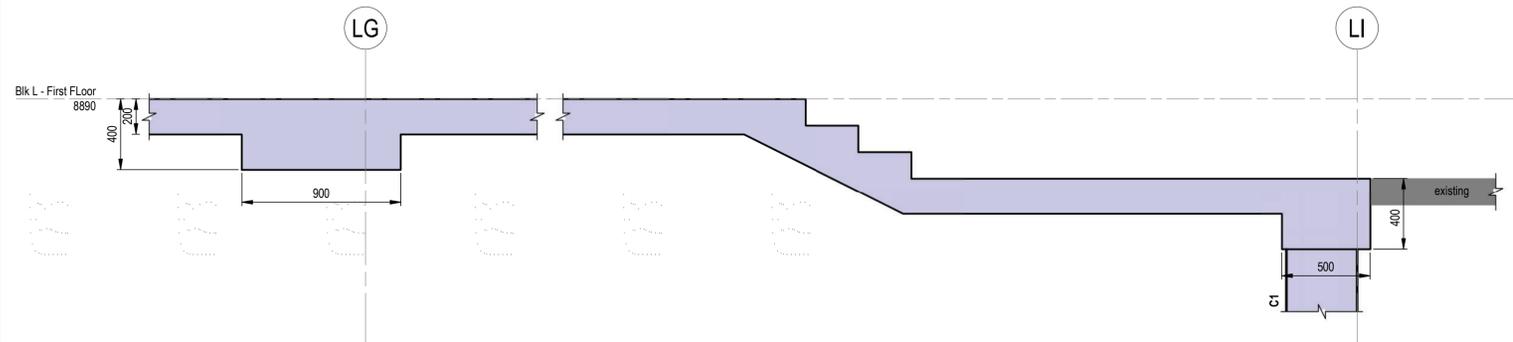
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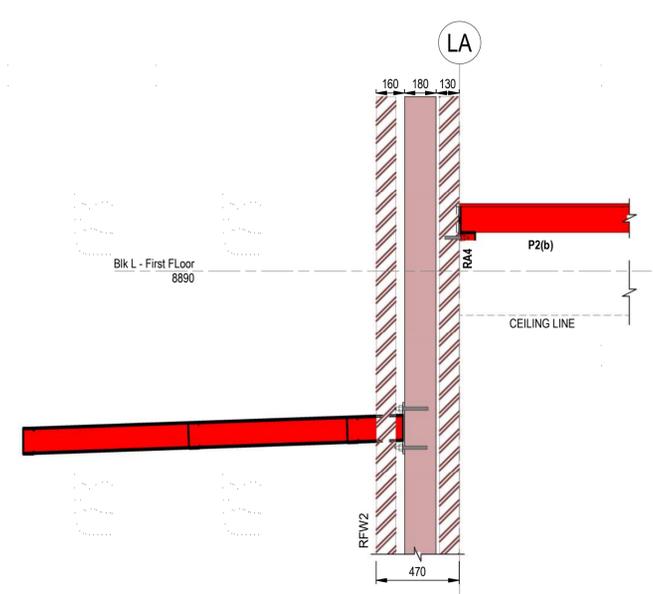
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Date	Project	CRONULLA HIGH SCHOOL
Date	Status	SCHEMATIC DESIGN
Date	Title	BLOCK L FIRST FLOOR G.A. PLAN

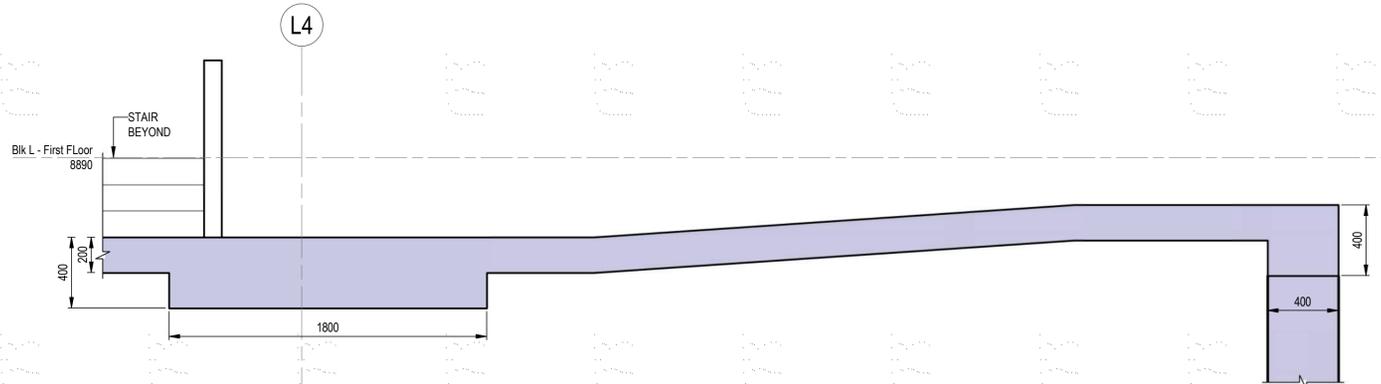
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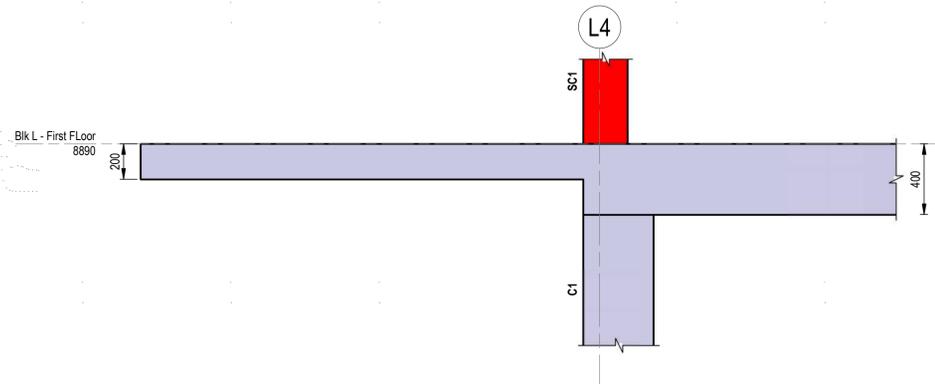
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ST-0141



SECTION LL1-4
SCALE 1:20
ST-0141



SECTION LL1-2
SCALE 1:20
ST-0141



SECTION LL1-3
SCALE 1:20
ST-0141

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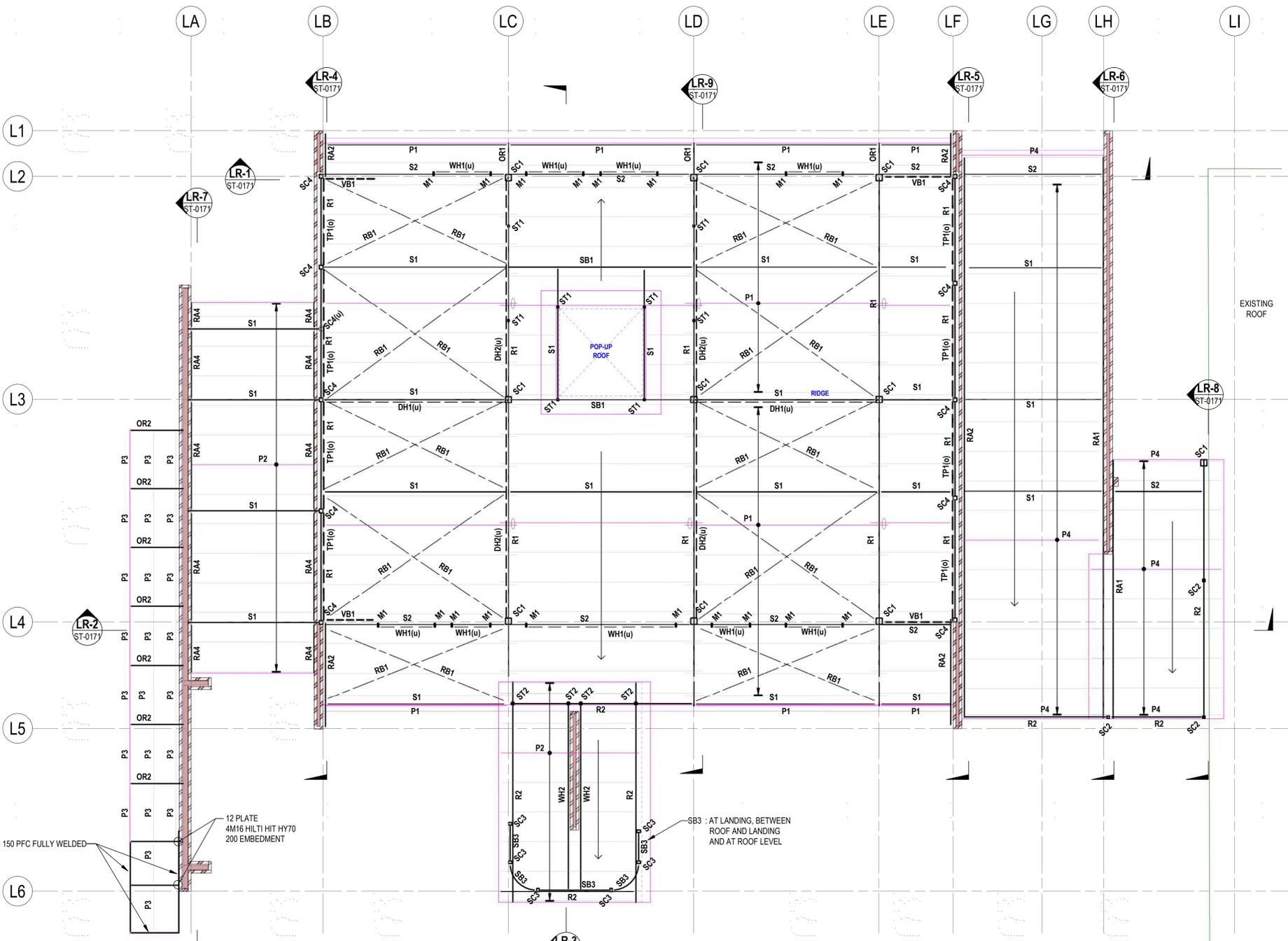
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Project	CRONULLA HIGH SCHOOL		
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Datum	Date	Scale	Size
		1:20	A1
Drawing Number	80821341-ST-0151		Revision
			5

BLOCK L FIRST FLOOR SECTIONS

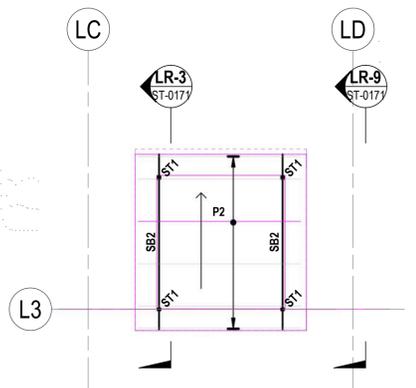
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BLOCK L ROOF FRAMING PLAN
SCALE 1 : 100

- ROOF FRAMING PLAN NOTES**
- FOR CONSTRUCTION NOTES REFER TO DRAWING ST-0001
 - FOR BUILDING SETOUT AND DIMENSIONS REFER TO ARCHITECTS DRAWINGS
 - FOR ALL ROOF PENETRATIONS REFER TO ARCHITECT AND SERVICE ENGINEERS DRAWINGS
 - ALL WATERPROOFING, FLASHING, GUTTERING AND SHEETING ARCHITECTS SPECIFICATION
 - ALL FALLS AND LEVELS TO ARCHITECTS DETAILS
 - ALL CONNECTIONS NOT DETAILED SHALL BE IN ACCORDANCE WITH AISC STANDARDIZED STRUCTURAL STEEL CONNECTIONS U.N.O.
 - FOR SURFACE TREATMENT TO ALL STRUCTURAL STEELWORK REFER TO CONSTRUCTION NOTES DRAWING
 - CONTRACTOR SHALL GIVE DUE CONSIDERATION TO ACCURACY OF FABRICATION AND PLANNING OF ERECTION PROCEDURE
 - FOR FIREPROOFING REQUIREMENTS REFER TO ARCHITECTS DRAWINGS

- LEGEND:**
- (u) : DENOTES MEMBER UNDER
 - (o) : DENOTES MEMBER OVER
 - (b) : DENOTES MEMBER BEYOND



BLOCK L POP-UP ROOF STEEL PLAN
SCALE 1 : 100

STEEL ROOF MEMBER SCHEDULE (BLOCK L)			
Mark	Member Size	MemberDescription_CDD	Comments
SC1	250x250x9 SHS	STEEL COLUMN	
SC2	100x100x6 SHS	STEEL COLUMN	
SC3	150x100x6 RHS	STEEL COLUMN	
SC4	150x150x5 SHS	STEEL COLUMN	
ST1	75x75x3 SHS	STUB COLUMN	
ST2	100x100x4 SHS	STUB COLUMN	
M1	150x50x4 RHS	MULLION	
R1	460UB67	RAFTER	FLY BRACING AT EVERY 2nd PURLINE
R2	200 PFC	RAFTER	
SB1	200 PFC	STEEL BEAM	
SB2	200 PFC	STEEL BEAM	
SB3	100x50x3 RHS	STEEL BEAM	
RA1	200 PFC	RACKING BEAM	
RA2	200 PFC	RACKING BEAM	TOES DOWN
RA4	150x90x8 UA	RACKING ANGLE	LOWER ROOF (FIRST FLOOR)
OR1	TW 100x200	OUTRIGGER	
S1	89x89x5 SHS	STRUT	
S2	200 PFC	STEEL BEAM	TOES DOWN
DH1	310UB40	DOOR HEADER	
DH2	200 PFC	DOOR HEADER	
WH1	200 PFC	WINDOW HEAD	TOES UP
WH2	150 PFC	WINDOW HEAD	TOES DOWN
P1	Z20015	PURLIN	@ 1200 MAX CTS. 900 AT END SPANS. LAP 900 OVER SUPPORTS. 2 ROWS OF BRIDGING
P2	Z15015	PURLIN	@ 1200 MAX CTS. 900 AT END SPANS. LAP 900 OVER SUPPORTS. 2 ROWS OF BRIDGING
P4	C20015	PURLIN	@ 1200 MAX CTS. 900 AT END SPANS. LAP 900 OVER SUPPORTS. 2 ROWS OF BRIDGING
TP1	C20015	PURLIN	GIRT (PARAPET) TOES DOWN
RB1	20 DIA ROD	ROOF BRACING	
VB1	150x100x5 RHS	VERTICAL BRACING	

Rev	Date	Description	Des.	Ver.	Appr.
7	04-07-2022	RE-ISSUED FOR SCHEMATIC DESIGN	AK	PP	
6	02-07-2022	RE-ISSUED FOR SCHEMATIC DESIGN	AK	PP	
5	23-06-2022	RE-ISSUED FOR SCHEMATIC DESIGN	AK	PP	
4	01-06-2022	ISSUED FOR SCHEMATIC DESIGN	AK	PP	
3	27-05-2022	ISSUED FOR SCHEMATIC DESIGN	AK	PP	
2	20-05-2022	ISSUED FOR SCHEMATIC DESIGN	AK	PP	
1	12-05-2022	ISSUED FOR SCHEMATIC DESIGN	AK	PP	

12 PLATE 4M16 HILTI HIT HY70 200 EMBEDMENT	150 PFC FULLY WELDED
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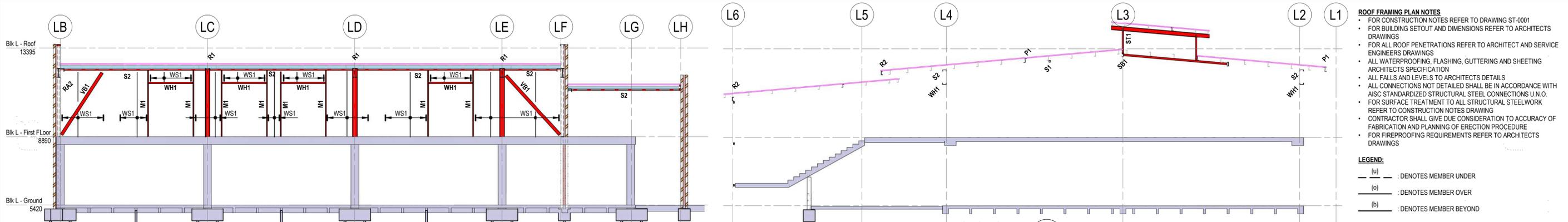
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Designed AK	Date
Verified PP	Date
Approved	Date

Client NSW DEPARTMENT OF EDUCATION (SCHOOLS INFRASTRUCTURE)	Date
Project CRONULLA HIGH SCHOOL	Date
Status SCHEMATIC DESIGN	Date
NOT TO BE USED FOR CONSTRUCTION PURPOSES	
Datum	Date
Scale 1:100	Size A1
Drawing Number 80821341-ST-0161	Revision 7
BLOCK L ROOF FRAMING PLAN	

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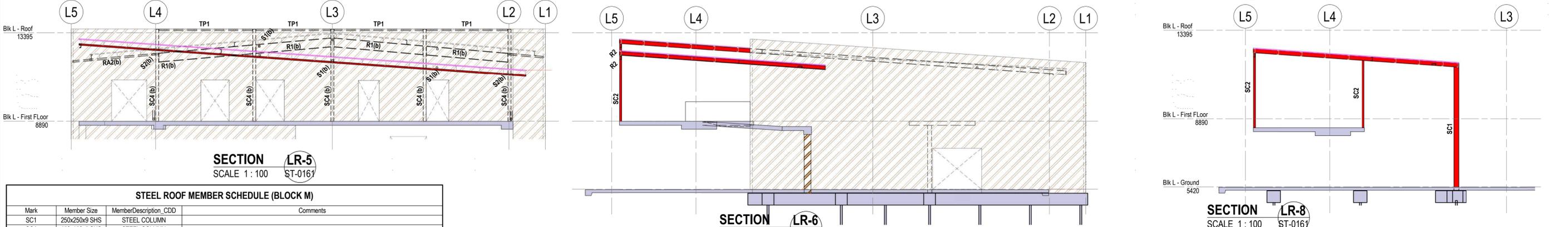
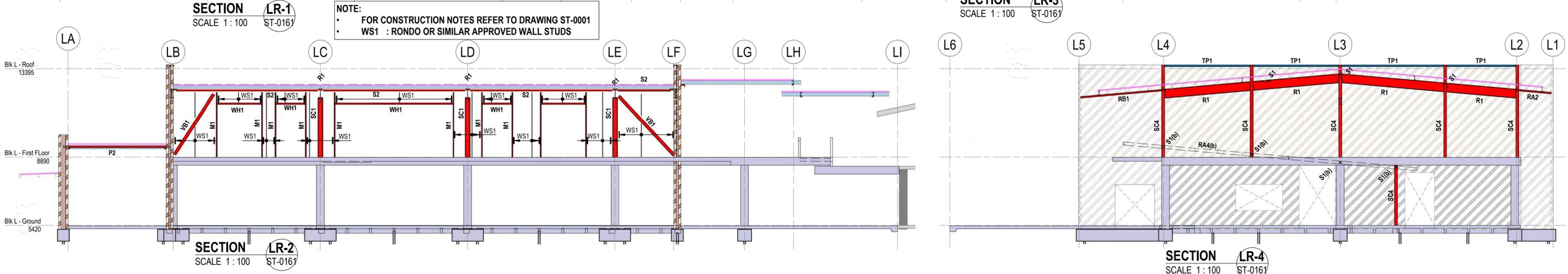


- ROOF FRAMING PLAN NOTES**
- FOR CONSTRUCTION NOTES REFER TO DRAWING ST-0001
 - FOR BUILDING SETOUT AND DIMENSIONS REFER TO ARCHITECTS DRAWINGS
 - FOR ALL ROOF PENETRATIONS REFER TO ARCHITECT AND SERVICE ENGINEERS DRAWINGS
 - ALL WATERPROOFING, FLASHING, GUTTERING AND SHEETING ARCHITECTS SPECIFICATION
 - ALL FALLS AND LEVELS TO ARCHITECTS DETAILS
 - ALL CONNECTIONS NOT DETAILED SHALL BE IN ACCORDANCE WITH AISC STANDARDIZED STRUCTURAL STEEL CONNECTIONS U.N.O.
 - FOR SURFACE TREATMENT TO ALL STRUCTURAL STEELWORK REFER TO CONSTRUCTION NOTES DRAWING
 - CONTRACTOR SHALL GIVE DUE CONSIDERATION TO ACCURACY OF FABRICATION AND PLANNING OF ERECTION PROCEDURE
 - FOR FIREPROOFING REQUIREMENTS REFER TO ARCHITECTS DRAWINGS

LEGEND:

- (u) : DENOTES MEMBER UNDER
- (o) : DENOTES MEMBER OVER
- (b) : DENOTES MEMBER BEYOND

NOTE:
FOR CONSTRUCTION NOTES REFER TO DRAWING ST-0001
WS1 : RONDO OR SIMILAR APPROVED WALL STUDS



STEEL ROOF MEMBER SCHEDULE (BLOCK M)			
Mark	Member Size	MemberDescription_CDD	Comments
SC1	250x250x9 SHS	STEEL COLUMN	
SC4	150x150x5 SHS	STEEL COLUMN	
ST1	75x75x3 SHS	STUB COLUMN	
M1	150x50x4 RHS	MULLION	
R1	460UB67	RAFTER	FLY BRACING AT EVERY 2nd PURLIN
R3	310UB32	RAFTER	
EB1	200 PFC	STEEL BEAM	IN PURLIN DEPTH
SB1	200 PFC	STEEL BEAM	
SB2	200 PFC	STEEL BEAM	
RA1	200 PFC	RACKING BEAM	
RA2	200 PFC	RACKING BEAM	TOES DOWN
RA3	150 PFC	RAFTER	
RA4	150x90x8 UA	RACKING ANGLE	LOWER ROOF (FIRST FLOOR)
RA5	300PFC	RACKING BEAM	
OR1	TW 100x200	OUTRIGGER	
S1	89x89x5 SHS	STRUT	
S2	200 PFC	STEEL BEAM	TOES DOWN
DH1	310UB40	DOOR HEADER	
DH2	200 PFC	DOOR HEADER	
DH3	200 PFC	DOOR HEADER	
WH1	200 PFC	WINDOW HEAD	TOES UP
P1	Z20015	PURLIN	@ 1200 MAX CTS. 900 AT END SPANS. LAP 900 OVER SUPPORTS, 2 ROWS OF BRIDGING
P2	Z15015	PURLIN	@ 1200 MAX CTS. 900 AT END SPANS. LAP 900 OVER SUPPORTS, 2 ROWS OF BRIDGING
P4	C20015	PURLIN	@ 1200 MAX CTS. 900 AT END SPANS. LAP 900 OVER SUPPORTS, 2 ROWS OF BRIDGING
TP1	C20015	PURLIN	GIRT (PARAPET) TOES DOWN
RB1	20 DIA ROD	ROOF BRACING	
VB1	150x100x5 RHS	VERTICAL BRACING	

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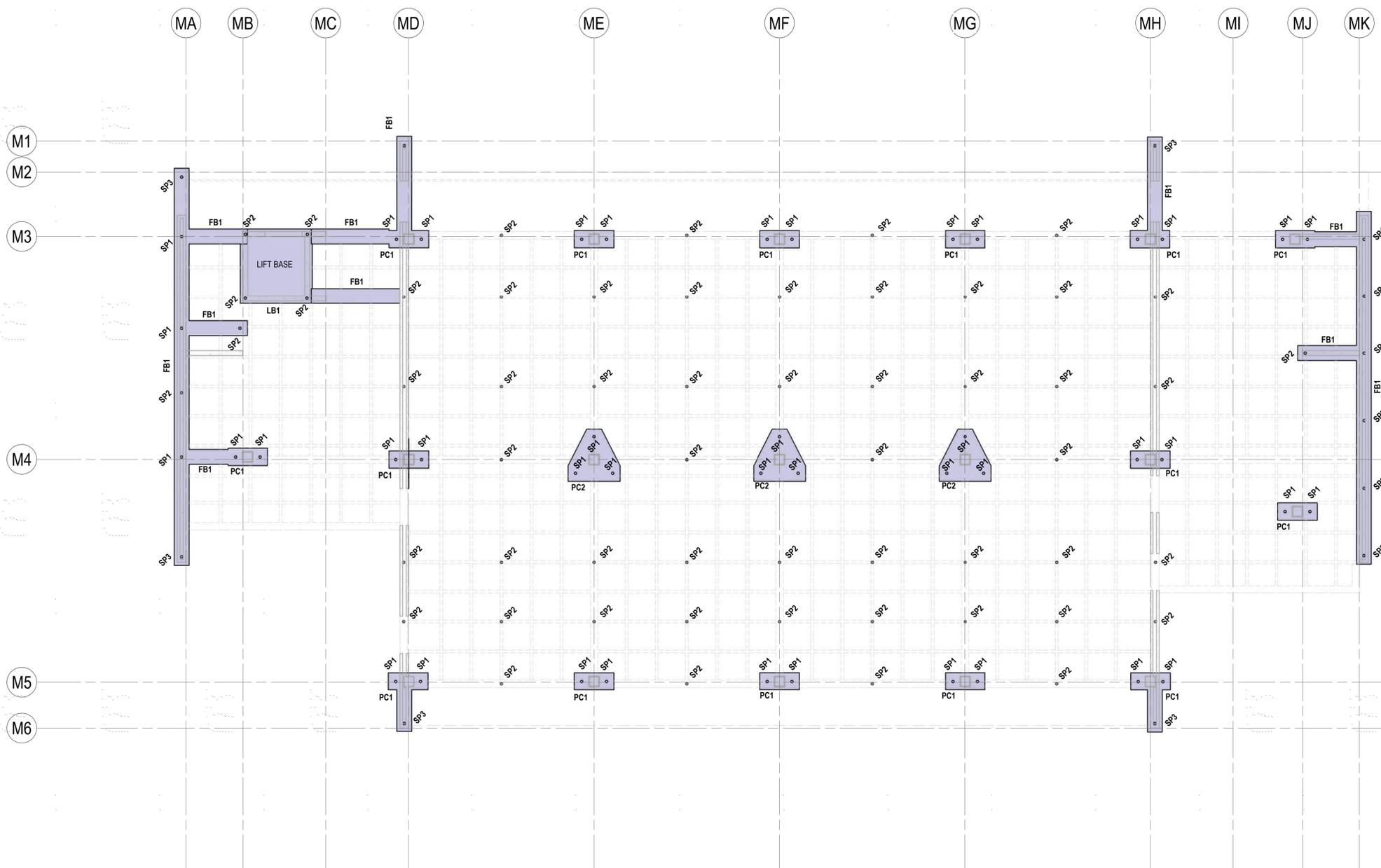
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Checked	AK
Designed	AK
Verified	PP
Approved	

Date	Client	NSW DEPARTMENT OF EDUCATION (SCHOOLS INFRASTRUCTURE)
Date	Project	CRONULLA HIGH SCHOOL
Date	Status	SCHEMATIC DESIGN
Date	NOT TO BE USED FOR CONSTRUCTION PURPOSES	
Date	Datum	Date
Date	Scale	1:100
Date	Size	A1
Date	Drawing Number	80821341-ST-0171
Date	Revision	7

STEEL FRAMING ELEVATION



BLOCK M FOUNDATION PLAN
SCALE 1 : 100

FOOTING PLAN NOTES

- GENERAL**
- FOR CONSTRUCTION NOTES REFER TO DRAWING ST-001
 - FOR BUILDING SETOUT AND DIMENSIONS REFER TO ARCHITECTS DRAWINGS
- FOOTINGS**
- ALL FOOTING LEVELS TO BE CHECKED AGAINST IN-GROUND SERVICES.
 - FOR ALLOWABLE BEARING PRESSURE REFER TO CONSTRUCTION NOTES DRAWING.
 - BUILDER TO CONFIRM ALL TOP OF FOOTING LEVELS WITH THE ARCHITECT PRIOR TO POURING CONCRETE.
 - THE ACTUAL DEPTH OF EACH FOOTING SHALL BE CHECKED AND APPROVED BY A GEOTECHNICAL ENGINEER.
 - ALL CONCRETE COLUMNS TO BE PLACED CENTRALLY ON PAD FOOTING U.N.O.
 - ALL WATERPROOFING TO ARCHITECTS SPECIFICATIONS
- PILES/PIERS**
- ALL PILES/PIERS ARE TO BE LOCATED CENTRALLY UNDER COLUMNS AND WALLS U.N.O. REFER TO ARCHITECTS DRAWINGS FOR SETOUT.
 - THE ACTUAL DEPTH OF EACH FOOTING SHALL BE CHECKED AND APPROVED BY A GEOTECHNICAL ENGINEER.
 - ALL LOADS SHOWN ARE UNFACTORED
 - LOADS ARE TO BE USED FOR PILE/PIER DESIGN PURPOSES ONLY.
 - PILE/PIER DESIGN TO COMPLY WITH AS2159
- EXISTING SERVICES**
- THE POSITION OF ALL EXISTING SERVICES SHOWN SHOULD BE REGARDED AS APPROXIMATE ONLY AND NOT NECESSARILY COMPREHENSIVE. IT IS THE CONTRACTORS RESPONSIBILITY TO DETERMINE THE EXACT LOCATIONS AND INFORM ALL AUTHORITIES PRIOR TO ANY EXCAVATION.
- A. APPLICABLE TO ALL PAD FOOTING AND FOOTING BEAMS INCREASE PAD DEPTH WITH 15MPa MASS CONCRETE AS REQUIRED TO:**
- OBTAIN AN ALLOWABLE BEARING PRESSURE OF 500kPa TO GEOTECHNICAL ENGINEERS APPROVAL ON SITE.
 - HAVE A LOAD DISPERSION LINE OF 45 FROM UNDERSIDE OF PAD TO CLEAR EXISTING SERVICES.
 - ENSURE PAD IS FOUNDED AT UNDERSIDE OF ADJACENT SERVICES EXCAVATIONS.

BLOCK L SCREW PILE SCHEDULE

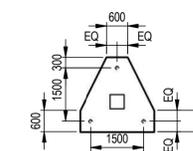
Mark	SWL (kN)
SP1	250
SP2	150
SP3	100

FOOTING BEAM SCHEDULE

MARK	SIZE		REINFORCEMENT	
	WIDTH (mm)	DEPTH (mm)	BARs	TIES
FB1	600	600	5N20 T&B	N12-300

CONCRETE PILE CAP FOOTING SCHEDULE (BLOCK M)

MARK	Description	SIZE			REINFORCEMENT		PILES
		WIDTH (mm)	LENGTH (mm)	DEPTH (mm)	BARs	TIES	
LB1	LIFT BASE	3000	2900	450	N16-250 T&B E/W	-	4 x SP2
PC1	PILE CAP	700	1600	600	5N20 T&B	N12-300	2 x SP1
PC2	PILE CAP	1500	1500	600	5N20 T&B	N12-300	3 x SP1



PC2 : PILES AT 1500 CENTRES AT 60 DEGREES

PILE CAP (PC2) DETAIL

SCALE 1 : 100

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		Date
		Scale 1:100
		Size A1
		Drawing Number 80821341-ST-0201
		Revision 7

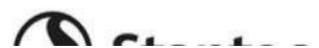
APPENDIX

A

STORMWATER DISPOSAL PLAN



now





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CIVIL WORKS

NSW DEPARTMENT OF EDUCATION
CRONULLA HIGH SCHOOL
CAPTAIN COOK DRIVE, CRONULLA NSW

COVER SHEET, LOCALITY PLAN AND SCHEDULE OF DRAWINGS

SCHEDULE OF DRAWINGS	
DRAWING No.	DESCRIPTION
CIVIL WORKS	
80821341-CI-0001	CIVIL COVER SHEET, LOCALITY PLAN AND SCHEDULE OF DRAWINGS
80821341-CI-0002	CIVIL CONSTRUCTION NOTES
80821341-CI-0101	CIVIL SITE AND STORMWATER DRAINAGE PLAN SHEET 1
80821341-CI-0102	CIVIL SITE AND STORMWATER DRAINAGE PLAN SHEET 2
80821341-CI-0103	CIVIL OSD TANK SECTIONS AND DETAILS
80821341-CI-0105	CIVIL SEDIMENTATION AND EROSION CONTROL PLAN SHEET 1
80821341-CI-0106	CIVIL SEDIMENTATION AND EROSION CONTROL PLAN SHEET 2
80821341-CI-0109	CIVIL CAR PARK PAVEMENT PLAN AND DETAILS
80821341-CI-0110	CIVIL SEDIMENTATION AND EROSION CONTROL DETAILS
80821341-CI-0111	CIVIL STORMWATER STANDARD DETAILS SHEET 1
80821341-CI-0112	CIVIL STORMWATER STANDARD DETAILS SHEET 2

INDICATIVE AREA
 OF WORKS



LOCALITY PLAN

Rev	Date	Description	Des	Verif	Appr
1	04/07/2022	SCHEMATIC DESIGN ISSUE		1/1	1/1

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Designed	DM	Date	04/07/2022	Scale		
Reviewed	DM	Date	04/07/2022	Scale		
Approved	DM	Date	04/07/2022	Scale		
Drawing Number				AHD	NTS	A1
80821341-CI-0001						1

CIVIL CONSTRUCTION NOTES

GENERAL NOTES

- G1. THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER CONSULTANT DRAWINGS AND SPECIFICATIONS AND WITH EACH OTHER WRITTEN INSTRUCTIONS THAT MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ANY DISCREPANCIES IN THESE DOCUMENTS SHALL BE REFERRED TO THE SUPERINTENDENT FOR A DECISION BEFORE PROCEEDING WITH THE WORK.
- G2. THE CONTRACTOR SHALL CHECK AND BE RESPONSIBLE FOR THE CORRECTNESS OF ALL DIMENSIONS AND ANY DISCREPANCY SHALL BE REPORTED IMMEDIATELY TO THE SUPERINTENDENT. DIMENSIONS SHALL NOT BE OBTAINED BY SCALING FROM THE DRAWINGS.
- G3. THE STABILITY OF THE BUILDING DURING CONSTRUCTION AND EXCAVATION IN THE VICINITY OF ADJACENT BUILDINGS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. NO PART OF THE STRUCTURE SHALL BE OVER STRESSED. APPROVAL OF ALL PROPOSALS MUST BE OBTAINED BY THE ARCHITECT PRIOR TO THE COMMENCEMENT OF WORK.
- G4. THE CONTRACTOR SHALL NOTIFY THE ENGINEER FORTY EIGHT (48) HOURS BEFORE THE REINFORCEMENT IS COMPLETED. THE CONTRACTOR SHALL ALLOW TWO (2) HOURS AFTER THE COMPLETION OF THE REINFORCEMENT FOR THE ENGINEER'S INSPECTION. CONCRETE SHALL NOT BE ORDERED UNTIL THE REINFORCEMENT IS APPROVED BY THE ENGINEER.
- G5. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT S.A. CODES. THE BY-LAWS AND ORDINANCES OF THE RELEVANT BUILDING AUTHORITY AND THE SPECIFICATION.
- G6. NO CHANGES SHALL BE MADE WITHOUT THE WRITTEN CONSENT OF THE ENGINEER.
- G7. U.N.O. DENOTES UNLESS NOTED OTHERWISE ON THE DRAWINGS.

SITE PREPARATION

- R1. REMOVE TOP SOIL, ROOT AFFECTED SOIL, FILL AND OTHER DELETERIOUS MATERIAL TO EXPOSE NATURAL SUBGRADE.
- R2. THE EXPOSED SUBGRADE SHOULD BE PROOF ROLLED WITH AT LEAST EIGHT (8) PASSES OF A 10 TONNE MIN. ROAD WEIGHT ROLLER. ANY SOFT OR HEAVING AREAS SHOULD BE REMOVED TO A MINIMUM DEPTH OF 200mm AND REPLACED WITH CLEAN WELLS GRADED MATERIAL, SUCH AS RPPEP OR CRUSHED SANDSTONE COMPACTED TO AT LEAST 100% OF STANDARD MAXIMUM DRY DENSITY (SMDO) AT 4% OPTIMUM MOISTURE CONTENT IN ACCORDANCE WITH AS 1080.
- R3. COMPACTED FILL SHOULD BE PLACED IN LAYERS NOT EXCEEDING 150mm THICK AND COMPACTED TO AT LEAST 100% SMDO. FILL SHALL CONSIST OF CLEAN WELLS GRADED MATERIAL, SUCH AS RPPEP OR CRUSHED SANDSTONE WITH A MIN. CBR OF 5%.
- R4. DENSITY TESTING SHALL BE CARRIED OUT TO LEVEL 2 CERTIFICATION IN ACCORDANCE WITH AS 2878.
- R5. SUBGRADE COURSE: TO BE DOSD OR DOSAD OR RPPEP OR CRUSHED SANDSTONE WITH A CBR GREATER THAN 4%L, MAXIMUM PARTICLE SIZE OF 80mm, WELLS GRADED WITH A PLASTIC INDEX LESS THAN 10, COMPACT TO AN AVERAGE OF NOT LESS THAN 10% SMDO WITH A MINIMUM VALUE OF 8% SMDO.
- R6. ALL WORKS TO BE FINISHED BY HAND MACHINE AND NOT BY HAND.
- R7. EXTERNAL FOOTPATHS/PAVEMENTS
- R8. REMOVE TOP SOIL, ROOT AFFECTED SOIL, FILL AND OTHER DELETERIOUS MATERIAL TO EXPOSE NATURAL SUBGRADE.
- R9. THE EXPOSED SUBGRADE SHOULD BE PROOF ROLLED, ANY SOFT OR HEAVING AREAS SHOULD BE REPLACED WITH CLEAN WELLS GRADED MATERIAL. FILL IF REQUIRED, SHOULD BE CLEAN AND WELLS GRADED COMPACT TO 100% SMDO.

STORMWATER CONSTRUCTION NOTES

- S1. ESTABLISH EXACT LOCATION AND INVERT OF EXISTING SERVICES PRIOR TO COMMENCING WORKS.
- S2. ALLOW TO PAY ALL LOCAL AUTHORITY FEES AS REQUIRED FOR PERIODIC INSPECTIONS/APPROVALS.
- S3. ALL WORK TO BE IN ACCORDANCE WITH THE LOCAL COUNCIL STORMWATER POLICY.
- S4. MAJOR FLOW RATE + 10 YEARS
- S5. MAJOR FLOW RATE + 10 YEARS
- S6. MAJOR FLOW RATE + 10 YEARS
- S7. MAJOR FLOW RATE + 10 YEARS
- S8. ALL WORKS TO COMPLY TO AS 3500
- S9. ALL NEW PIPES 300 DIA. LARGER SHALL BE REINFORCED CONCRETE CLASS 2 WITH RUBBER RING JOINTS, WHERE ANGLED THE MINIMUM RADIUS SHALL BE 150x DIA. 1500mm DIA. STORMWATER PIPES SHALL BE BEVER CLASS UPVC.
- S10. MINIMUM PIPE SIZE FOR AN UNDERGROUND PIPE TO BE 150mm MINIMUM PIPE GRADEMENTS 1% U.N.O.
- S11. ALL DRAINAGE TRENCHES SHALL BE IN SOUND EXCAVATED MATERIAL. IF SOFT SPOTS EXIST, REMOVE AND BACKFILL WITH COMPACTED ROAD BASE (DOSAD) WITH A MINIMUM CBR OF 3% COMPACT TO 95% STANDARD MAXIMUM DRY DENSITY TO AS 1288 E.1.1.
- S12. ALL PIPES SHALL BE BEDDED ON 150mm SAND BED AND BACKFILLED WITH SAND TO 150mm ABOVE BARREL OF PIPE. THE REMAINDER OF THE TRENCH WILL BE BACKFILLED IN 150mm COMPACTED LAYERS IN IRREGULAR FILL NON COMPRESSIVE (EMERSON CLASS 2 OR 3) MATERIAL. NO TOP SOIL, GRASS, ROOTS, OR DELETERIOUS MATERIAL, COMPACT TO 95% STANDARD MAXIMUM DRY DENSITY AT 4% MO.
- S13. PROVIDE A 150mm DIA. UPVC SLOTTED DRAINAGE PIPE 3000 LONG WRAPPED IN FILTER FABRIC SOCK IN ALL TRENCHES ADJACENT TO INLET PIPES TO PITS & CONNECTED TO PIT.
- S14. OTHER SUB SOIL DRAINAGE PIPES SHALL BE 100mm DIA. UPVC SLOTTED BEDDED AND BACKFILLED WITH 200mm GRADE BLUE METAL. CLEAN CUTS SHALL BE EXPOSED TO THE SURFACE AND PROVIDED WITH A SCREENED COVER PLATE FLUSH WITH THE FINISHED SURFACE LEVEL.
- S15. ALL CONCRETE PITS CONSTRUCTED SHALL BE BEDDED AS PER PIPE SPECIFICATION. FIT BASES SHALL BE SMOOTH CONToured WITH MASS CONCRETE BENCHING PROVIDE STEP RINGS AS PER FIT SCHEDULE. PROVIDE HEAVY, MEDIUM OR LIGHT DUTY GALVANISED GRATE COVERS AS SPECIFIED IN SCHEDULE. ALL DRAINAGE WORKS ARE TO BE COMPLETED TO THE SATISFACTION OF THE SUPERVISING CIVIL ENGINEER. ALL WORKS TO COMPLY TO AS 3500. HEADWALLS SHALL BE PRECAST CONCRETE BY TOP PRECAST OR EQUAL. ALL PITS SHALL BE PRECAST CONCRETE TYPE OPT FOR DEEP PITS BY CONJUNCTIONS OR EQUAL. PRECAST DRAINAGE PITS DEEPER THAN 1800mm SHALL HAVE 1800mm MIN. WALL THICKNESS. 300mm DIA. GALV. MET. STEP RINGS SHALL BE RETIRED IN ITS 1200mm AND DEEPER.
- S16. UNLESS NOTED OTHERWISE ON THE PLANS, PROVIDE THE FOLLOWING MIN. COVER TO PIPE:
 - UNDER LANDSCAPE / PAVEMENT: 300mm
 - UNDER ROAD (TRAFFIC): 400mm

CONCRETE

- C1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE CURRENT S.A. CODE AS 3500, WITH AMENDMENTS EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.
- C2. CONCRETE QUALITY

ELEMENT	SUB-BASE	MIN. MAX. SIZE	MIN. MAX. SLAB THICKNESS	ADMX	CONCRETE CLASS
CONCRETE DRIVEWAYS	60	20mm	A	N/A	40 MPa
CONCRETE KERBS, RETAINING WALLS	60	20mm	A	N/A	30 MPa
FOOTPATHS	60	20mm	A	N/A	25 MPa
PISHS	60	20mm	A	N/A	25 MPa
- C3. SUPERFENDED CONCRETE FLOORS
 - MIN. COVER: 20mm
 - MIN. RUBBER RING JOINTS: 150mm
 - MAX. PERMISSIBLE DRYING SHRINKAGE: +750 MICROSTRAIN AT 28 DAYS
- C4. CLEAR CONCRETE COVER IN mm TO REINFORCEMENT U.N.O. SHALL BE AS FOLLOWS:

STRUCTURAL ELEMENT	REINFORCEMENT COVER			
	INTERNAL	EXTERNAL	TOP	BTM
FOOTPATHS & PISHS	-	-	50	50
DRAINAGE PITS	-	-	50	50
CONCRETE DRIVEWAY	-	-	40	40
- C5. NOTES:
 - CONCRETE POURED OVER A MEMBRANE ON THE GROUND IS INCLUDED AS INTERNAL.
 - CONCRETE EXPOSED TO CORROSIVE VAPOURS, CORROSIVE GROUND WATER, SEA WATER OR DRAY IS TO HAVE REINFORCEMENT COVER AS NOTED ON THE DRAWINGS.
 - CONCRETE REQUIRING A FIRE RESISTANCE RATING SHALL HAVE REINFORCEMENT COVER AS NOTED ON THE DRAWINGS.
 - EXTERNAL ABOVE GROUND ELEMENTS ARE CLASSIFIED IN NEAR COASTAL ENVIRONMENT AND NO HOLES OR GAPS OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ALLOWED WITHOUT THE PRIOR APPROVAL OF THE SUPERINTENDENT.
 - CONCRETE SIZES DO NOT INCLUDE THE THICKNESS OF APPLIED FINISHES.
 - THE DEPTH OF BEAMS IS GIVEN FIRST AND INCLUDES THE SLAB THICKNESS.
 - CONSTRUCTION JOINTS, WHERE NOT SHOWN, SHALL BE LOCATED TO THE APPROVAL OF THE SUPERINTENDENT.
 - FORMWORK SHALL REMAIN IN POSITION FOR THE TIME SPECIFIED, WHERE SLABS AND BEAMS ARE TO SUPPORT MASONRY OVER, FORMWORK AND PROPS MUST BE REMOVED PRIOR TO THE CONSTRUCTION OF MASONRY.
 - ALL CONCRETE SHALL BE MECHANICALLY VIBRATED. THE VIBRATOR SHALL NOT BE USED TO SPREAD CONCRETE.
 - CONCRETE SHALL BE CURED IN ACCORDANCE WITH AS 3500 WITH A PRODUCT COMPATIBLE WITH THE APPLIED FINISHES. CURING COMPOUNDS SHALL COMPLY WITH AS 3799. PVA BASED CURING COMPOUNDS ARE NOT ACCEPTABLE.
 - REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY. IT IS NOT NECESSARILY SHOWN IN TRUE PROJECTION.
 - WELDING OF REINFORCEMENT SHALL NOT BE PERMITTED UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.
 - SPLICES IN THE MAIN REINFORCEMENT SHALL BE MADE ONLY IN THE POSITIONS SHOWN. SPLICES IN THE DISTRIBUTION REINFORCEMENT MAY BE POSITIONED AS NECESSARY WITH SPLICES OF SUFFICIENT LENGTH TO DEVELOP THE FULL STRENGTH OF THE BARS. MINIMUM LAPS TO FABRIC SHALL BE TO OVER LAP TWO CROSS WIRES PLUS 50mm U.N.O. REINFORCEMENT SHALL BE SECURELY TIED AT ALL LAPS AND INTERSECTIONS WITH 150mm BLACK ANNEALED WIRE. THE WRITTEN APPROVAL OF THE SUPERINTENDENT SHALL BE OBTAINED FOR OTHER SPLICES WHERE THE LAP LENGTH IS NOT SHOWN. IT SHALL DEVELOP THE FULL STRENGTH OF THE REINFORCEMENT.
 - ALL UNSUPPORTED BARS SHALL BE TIED IN A TRANSVERSE DIRECTION WITH N2 300 U.N.O. REINFORCEMENT SHALL BE SUPPORTED ON APPROVED PLASTIC OR PLASTIC TYPED WIRE. STOKS AT NOT MORE THAN 200mm CENTRES BOTHWAYS IN SLABS AND AT 1500mm CENTRES IN BEAMS.
- C6.
 - REINFORCING FABRIC TO BE RECTANGULAR FABRIC TO AS 4671.
 - REINFORCING FABRIC TO BE SQUARE FABRIC TO AS 4671.
 - REINFORCING FABRIC TO BE 500 DEFORMED WIRE REINFORCING SQUARE FABRIC OF DUCTILITY CLASS 1 TO AS 4671.
 - REINFORCING FABRIC TO BE 500 DEFORMED WIRE REINFORCING RECTANGULAR FABRIC OF DUCTILITY CLASS 1 TO AS 4671.
 - REINFORCING FABRIC TO BE 500 DEFORMED WIRE REINFORCING SQUARE FABRIC OF DUCTILITY CLASS N TO AS 4671.
 - REINFORCING FABRIC TO BE 500 DEFORMED WIRE REINFORCING RECTANGULAR FABRIC OF DUCTILITY CLASS N TO AS 4671.
 - REINFORCING FABRIC TO BE 250 DEFORMED BARS OF DUCTILITY CLASS N TO AS 4671.

- C7. FABRIC SHALL BE SUPPLIED IN FLAT SHEETS. ROLLS WILL NOT BE ACCEPTED.
- C8. TYPICAL REINFORCEMENT NOTATION
 - 500A 200 INDICATES
 - 500A DENOTES NUMBER OF BARS REQUIRED
 - 200 DENOTES GRADE OF REINFORCEMENT
 - 30L DENOTES BAR SPACING IN MILLIMETRES
 - 20L DENOTES BAR SPACING IN MILLIMETRES
 - TYPICAL ABBREVIATIONS:
 - B ... DENOTES BARS IN BOTTOM LAYER
 - T ... DENOTES BARS IN TOP LAYER
 - ALT ... DENOTES BARS ALTERNATING
 - NF ... DENOTES BARS IN NEAR FACE
 - FF ... DENOTES BARS IN FAR FACE
 - EF ... DENOTES BARS IN EACH FACE
 - FOR LAB WALLS, CHAMBERS, REGLETTS, DRIP GROOVES, ETC., REFER TO THE ARCHITECT'S DRAWINGS.
 - LAP LENGTHS FOR DEFORMED BARS FOLLOWS

BAR TYPE AND SIZE	VERTICAL BARS	HORIZONTAL BARS WITH MORE THAN 200mm OF CONCRETE BELOW BAR	OTHER LOCATIONS	90° COG. LENGTH
N12	500	500	500	200
N16	750	600	750	200
N20	1000	750	1000	200
N24	1200	1000	1200	200
N28	1400	1250	1400	200
- C9. REINFORCEMENT COVER
- C10. LAP LENGTHS FOR DEFORMED BARS FOLLOWS

SEDIMENT RUN-OFF CONTROL NOTES

- S81. THE CONTRACTOR SHALL INSTALL AND MAINTAIN SOIL EROSION AND SEDIMENT CONTROL MEASURES GENERALLY IN ACCORDANCE WITH GUIDELINES OF THE LANDCOM MANAGING GREEN STORMWATER MANUAL AND AS NECESSARY TO PREVENT RUN-OFF FROM SITE OF SEDIMENT RESULTING FROM THE WORKS. SUCH MEASURES SHALL ALSO COMPLY WITH REQUIREMENTS OF COUNCIL, LANDCOM 'SILENT BOOK' AND EPA. THIS WORK SHALL BE DONE PRIOR TO ANY EARTHWORKS COMMENCING ON SITE.
- S82. GRADE FINISHED SURFACE TO BE DRAINAGE EVENLY WITHOUT CHANNELLING. (SILT) PIPES STORMWATER SYSTEMS IS CONSTRUCTED, NOMINAL GRADIENTS FROM HIGH POINT OF 5%L.
- S83. MAINTAIN THE EROSION CONTROL DEVICES INDICATED ON THE DRAWINGS TO THE SATISFACTION OF THE SITE SUPERINTENDENT AND THE LOCAL AUTHORITIES.
- S84. WHEN PROPOSED STORMWATER PITS ARE CONSTRUCTED, PREVENT SITE RUNOFF ENTERING UNLESS SALT FENCES ARE ERECTED AROUND PITS AND ON ROAD.
- S85. STREET PROTECTION WITH SHAMER EXIST ORDS & STREET FIT INLET PROTECTION TO BE MAINTAINED FOR THE DURATION OF THE CONTRACT.

WARNING

UNLESS NOTIFIED TO THE CONTRARY IN WRITING, THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR ANY BREACHES OF THE PROTECTION OF ENVIRONMENT OPERATING ACT 1987. PLEASE NOTE FAILURE TO IMPLEMENT OR MAINTAIN APPROPRIATE EROSION/SEDIMENT CONTROL MEASURES IS A BREACH OF THE ACT. SUCH A BREACH IS LIABLE FOR A ON-THE-SPOT FINE AND/OR PENALTY.

1	14/02/2023	SCHEMATIC DESIGN ISSUE	V1	V1	RF
Rev.	Date	Description	Drawn	Checked	Approved



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Approved	MAY 2022	

Client: NSW DEPARTMENT OF EDUCATION

Project: CRONULLA HIGH SCHOOL
CAPTAIN COOK DRIVE, CRONULLA NSW

Title: CIVIL CONSTRUCTION NOTES

Status: SCHEMATIC DESIGN
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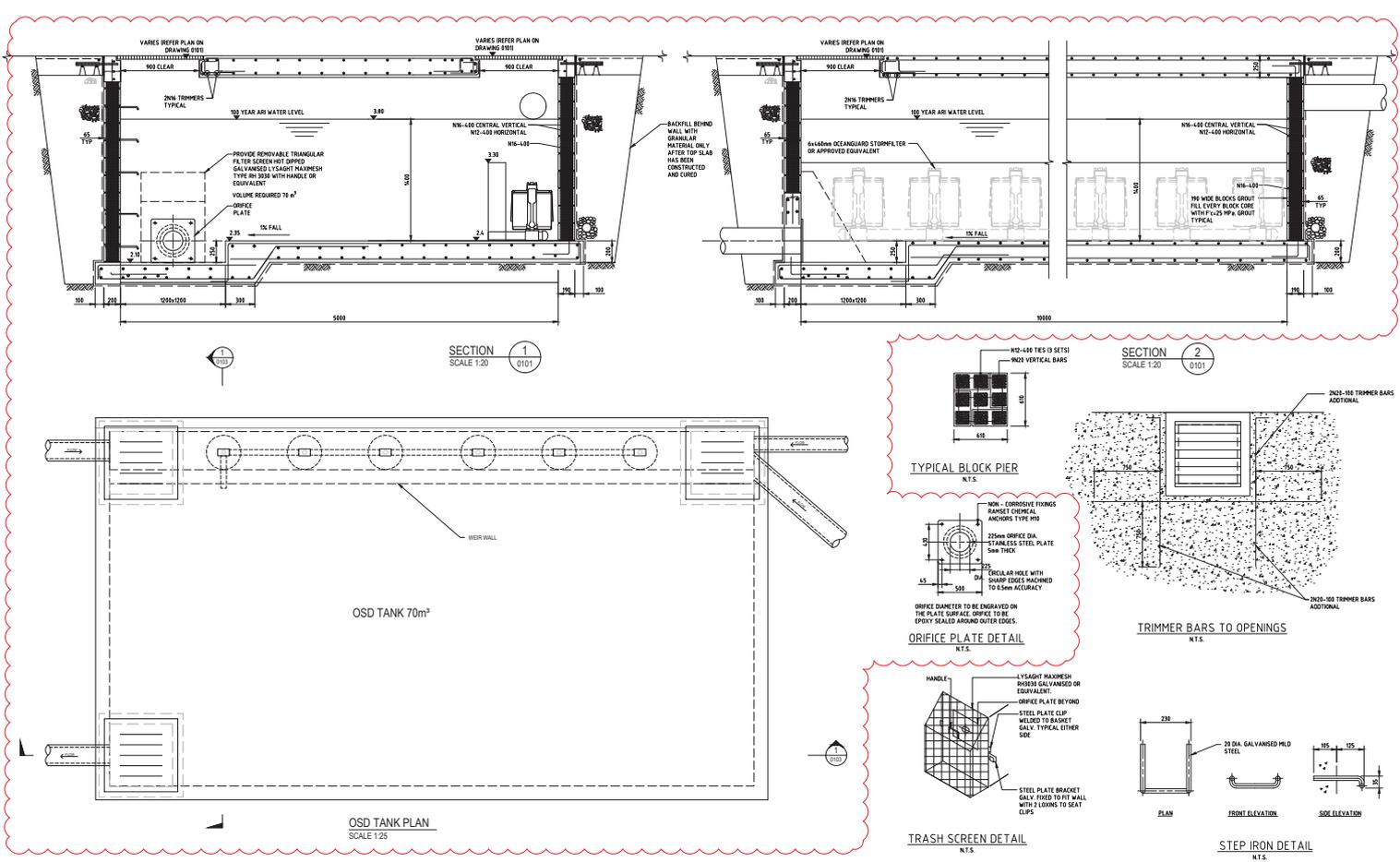
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Drawing Number: 80821341-CI-0002

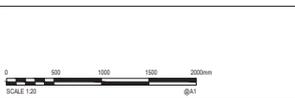
Revision: 1

DATE/DESIGNED: 14/04/2022/REDA/CRONULLA/BALESTRACCI/CI-0103-001

DRAWN BY: 14/04/2022/REDA/CRONULLA/BALESTRACCI/CI-0103-001



Rev	Date	Description	Des	Wrtt	Appr
1	14/04/2022	SCHEMATIC DESIGN ISSUE	REDA	REDA	REDA
2	14/04/2022	SCHEMATIC DESIGN ISSUE	REDA	REDA	REDA
3	14/04/2022	SCHEMATIC DESIGN ISSUE	REDA	REDA	REDA
4	14/04/2022	ISSUED FOR COORDINATION	REDA	REDA	REDA
5	14/04/2022	ISSUED FOR COORDINATION	REDA	REDA	REDA



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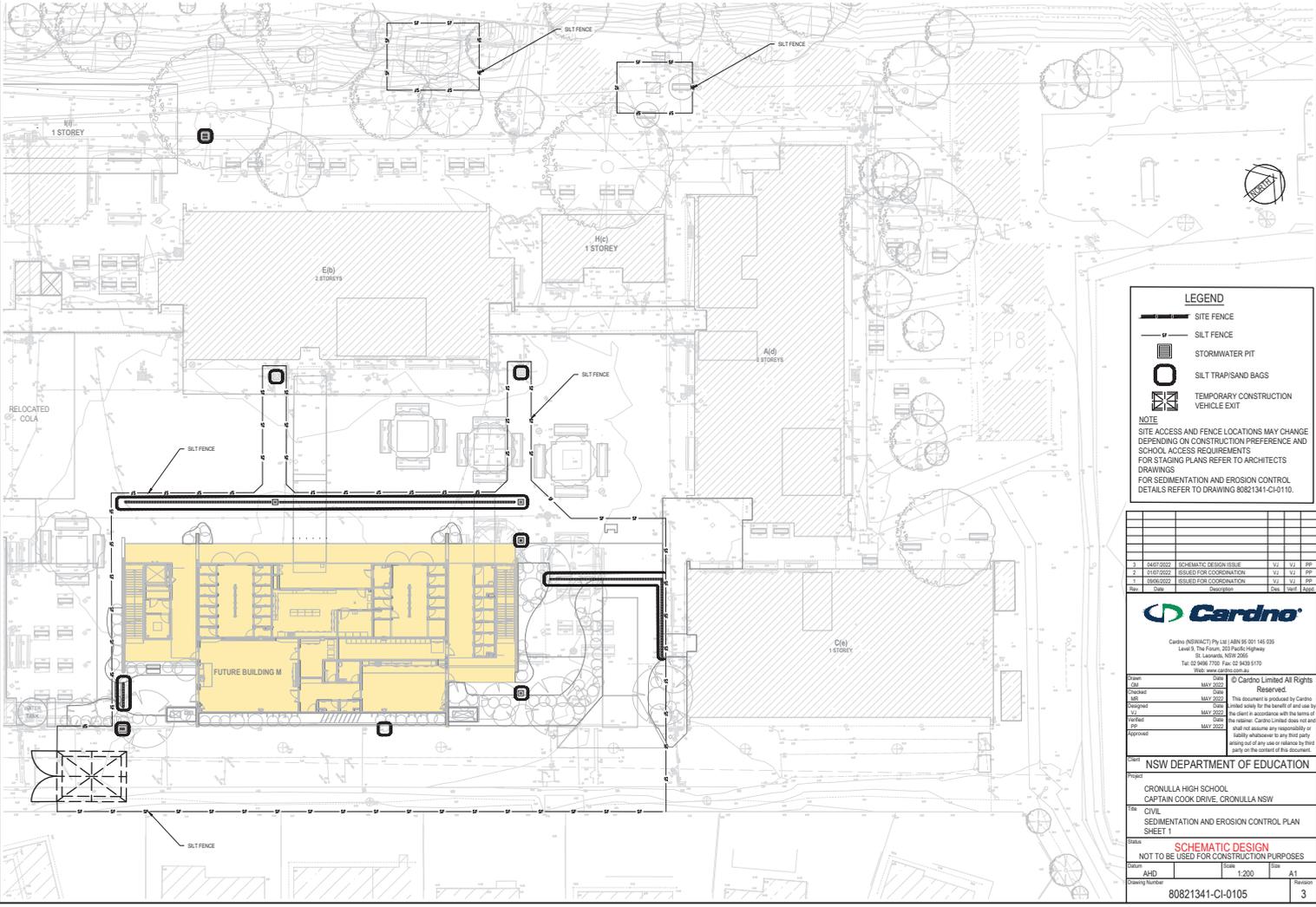


Drawn	Date	Client
REDA	MAY 2022	NSW DEPARTMENT OF EDUCATION
Checked	Date	Project
REDA	MAY 2022	CRONULLA HIGH SCHOOL CAPTAIN COOK DRIVE, CRONULLA NSW
Designed	Date	Discipline
REDA	MAY 2022	CIVIL
W/C	Date	Task
REDA	MAY 2022	OSD TANK SECTIONS AND DETAILS
Approved	Date	Signature
REDA	MAY 2022	

Status	Scale	Sheet
SCHEMATIC DESIGN	1:200	A1
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Client Ref:	80821341-CI-0103	Revision
		4

DATE PUBLISHED: 14/05/2022 01:01 PM / 10:08 AM (AEST)

DRAWN BY: A. HILL, REVISED BY: A. HILL, CHECKED BY: A. HILL, APPROVED BY: A. HILL, PROJECT NO: 80821341-CI-0105, DRAWING NO: 80821341-CI-0105



LEGEND

- SITE FENCE
- SILT FENCE
- STORMWATER PIT
- SILT TRAP/SAND BAGS
- TEMPORARY CONSTRUCTION VEHICLE EXIT

NOTE
 SITE ACCESS AND FENCE LOCATIONS MAY CHANGE DEPENDING ON CONSTRUCTION PREFERENCE AND SCHOOL ACCESS REQUIREMENTS
 FOR STAGING PLANS REFER TO ARCHITECTS DRAWINGS
 FOR SEDIMENTATION AND EROSION CONTROL DETAILS REFER TO DRAWING 80821341-CI-0110.

Rev.	Date	Description	Drawn	Checked	Approved
3	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ	PP
2	01/05/2022	ISSUED FOR COORDINATION	VJ	VJ	PP
1	16/05/2022	ISSUED FOR COORDINATION	VJ	VJ	PP

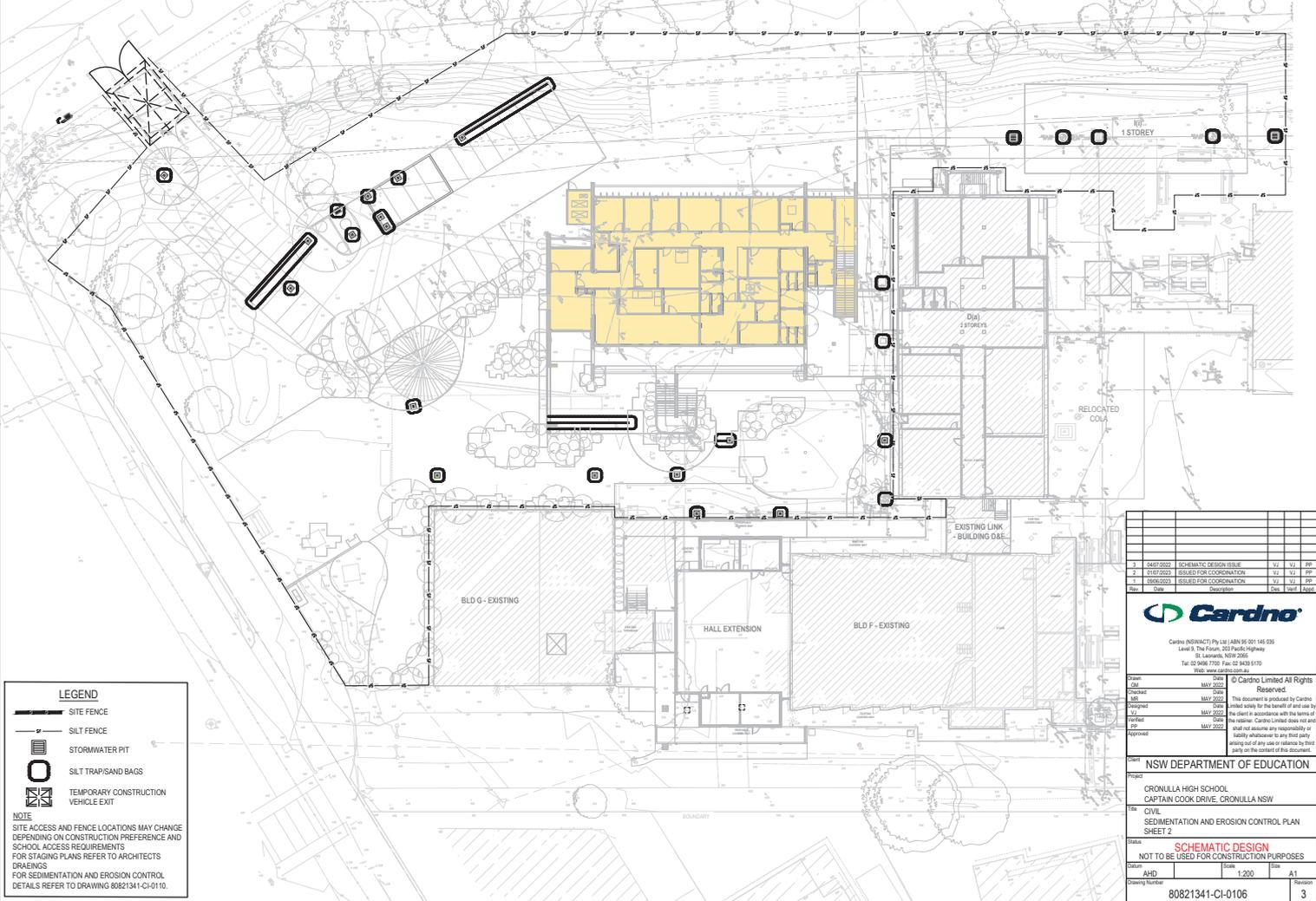
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Client: NSW DEPARTMENT OF EDUCATION
Project: CRONULLA HIGH SCHOOL
 CAPTAIN COOK DRIVE, CRONULLA NSW
Title: SEDIMENTATION AND EROSION CONTROL PLAN SHEET 1
Scale: SCHEMATIC DESIGN
 NOT TO BE USED FOR CONSTRUCTION PURPOSES
Sheet: AHD, Size: 1:200, Total: A1
Drawing Number: 80821341-CI-0105, **Revision:** 3

DATE PLOTTED: 14/05/2023 01:10:17 USER: JORDAN.MERBAKIC



LEGEND

- SITE FENCE
- SILT FENCE
- STORMWATER PIT
- SILT TRAP/SAND BAGS
- TEMPORARY CONSTRUCTION VEHICLE EXIT

NOTE

SITE ACCESS AND FENCE LOCATIONS MAY CHANGE DEPENDING ON CONSTRUCTION PREFERENCE AND SCHOOL ACCESS REQUIREMENTS FOR STAGING PLANS REFER TO ARCHITECTS DRAWINGS FOR SEDIMENTATION AND EROSION CONTROL DETAILS REFER TO DRAWING 80821341-CI-0110.

3	04/07/2022	SCHEMATIC DESIGN ISSUE	V1	V1	PP
2	01/05/2022	ISSUED FOR COORDINATION	V1	V1	PP
1	06/05/2022	ISSUED FOR COORDINATION	V1	V1	PP
Rev.	Desc.	Created	Drawn	Checked	Approved

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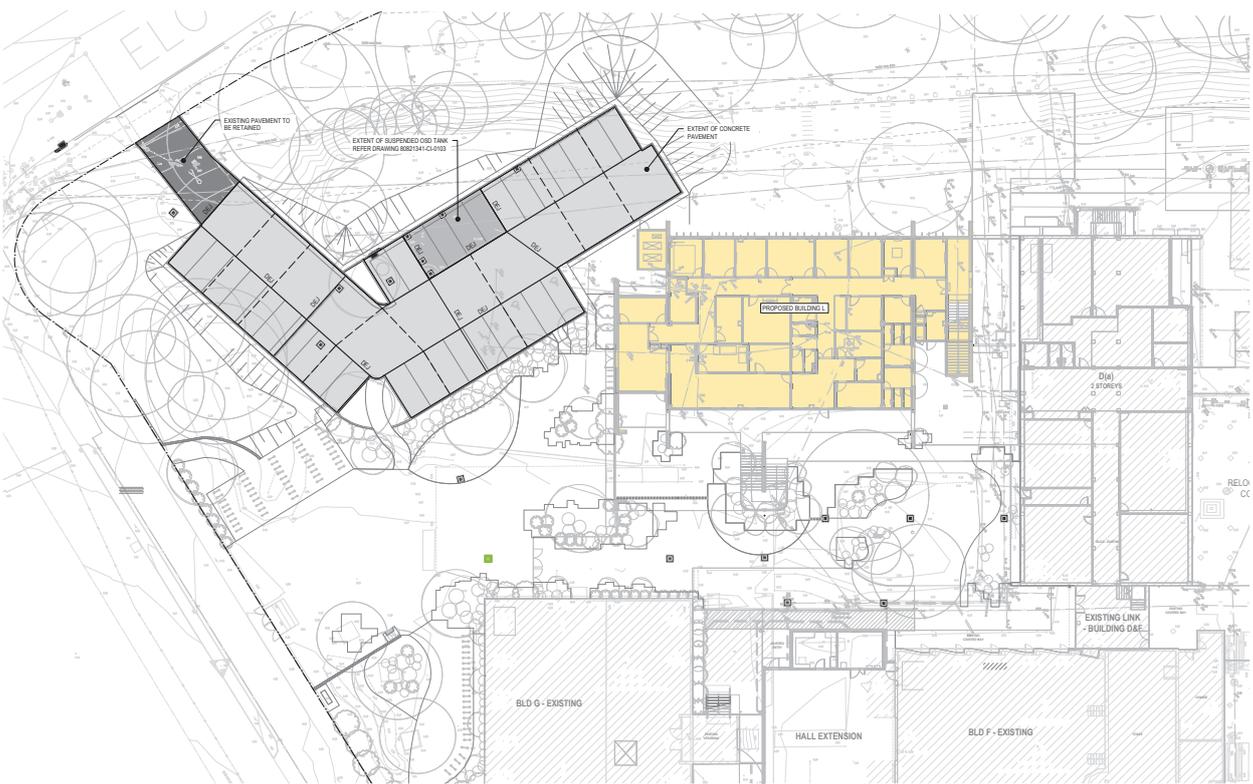
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Client: NSW DEPARTMENT OF EDUCATION
 Project: CRONULLA HIGH SCHOOL
 CAPTAIN COOK DRIVE, CRONULLA NSW
 Title: SEDIMENTATION AND EROSION CONTROL PLAN SHEET 2
 Status: **SCHEMATIC DESIGN**
 NOT TO BE USED FOR CONSTRUCTION PURPOSES

Scale:	AHD:	1:200	Sheet:	A1	
Drawing Number:	80821341-CI-0106			Revision:	3

80821341-CI-0106 - SEDIMENTATION AND EROSION CONTROL PLAN SHEET 2 - CRONULLA HIGH SCHOOL
 14/05/2023 01:10:17 USER: JORDAN.MERBAKIC

DATE PLOTTED: 14/05/2022 01:51:57 PM / JOB: 80821341-CI-0109 / USER: JORDAN.MERBAKOVIC



LEGEND:

- EXISTING PAVEMENT TO REMAIN
- PROPOSED CONCRETE PAVEMENT
- SUSPENDED OSD TANK SLAB

No.	Date	Description	By	Appr.
2	04/07/2022	SCHEMATIC DESIGN ISSUE	VJ	VJ
1	01/07/2022	ISSUED FOR COORDINATION	VJ	VJ

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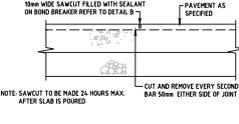
Drawn	Check	Design	Issue	Approved
DM	DM	DM	DM	DM

Client	NSW DEPARTMENT OF EDUCATION
Project	CRONULLA HIGH SCHOOL CAPTAIN COOK DRIVE, CRONULLA NSW
Title	CIVIL CAR PARK PAVEMENT PLAN AND DETAILS
Status	SCHEMATIC DESIGN NOT TO BE USED FOR CONSTRUCTION PURPOSES
Scale	AHD: 1:200, Plan: A1
Drawing Number	80821341-CI-0109
Revision	2

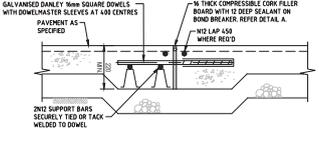


150MM SLAB, REINFORCED WITH 5L42 MESH POURED ON 100MM SAND BLINDING OVER WALKER PROOF MEMBRANE ON 100MM ASPHALT COMPACTED TO 98% CBR. INCREASE SLAB THICKNESS TO 200MM FOR AT LEAST 15M FROM THE ENDS.
 SUBGRADE TO BE PREPARED FOR A CBR OF 10% IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT

JOINTS AS SHOWN SHALL BE SAWN JOINTS UNLESS NOTED OTHERWISE

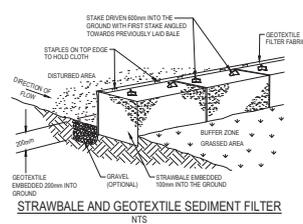


3mm WIDE SAWCUT FILLED WITH SEALANT ON BONDING BREAKER REFER TO DETAIL B.
 PAVEMENT AS SPECIFIED.
 NOTE: SAWCUT TO BE MADE 24 HOURS MAX. AFTER SLAB IS POURED.
 CUT AND REMOVE EVERY SECOND BAR SOME EITHER SIDE OF JOINT

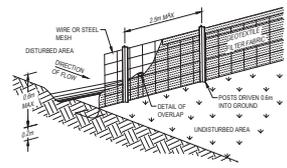


GALVANIZED STEEL 16mm SQUARE BOWELS WITH DOWELMASTER SLEEVES AT 400 CENTRES.
 PAVEMENT AS SPECIFIED.
 10mm THICK COMPRESSIBLE CORK FILLER BOWLS WITH 12 DEEP SEALANT ON BOND BREAKER. REFER DETAIL A.
 100mm L450 WHERE REQD.
 2002 SUPPORT BARS SECURELY TIED OR TACK WELDED TO BOWEL

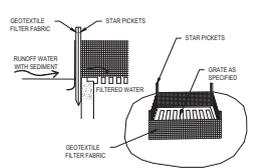
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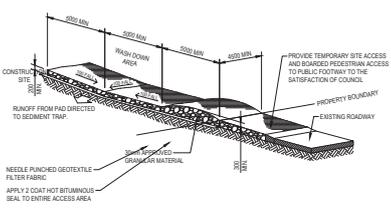
STRAWBALE AND GEOTEXTILE SEDIMENT FILTER
NTS



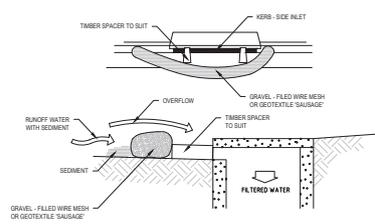
SEDIMENT FENCE
NTS



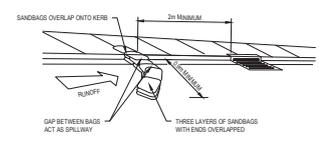
GEOTEXTILE FILTER PIT SURROUND
NTS



STABILISED SITE ACCESS AND TRUCK WASH DOWN AREA
NTS



MESH AND GRAVEL INLET FILTER
NTS



SANDBAG SEDIMENT TRAP FOR KERB INLET ON GRADE
NTS

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No.	Date	Description	Drawn	Checked	Appr.
1	04/07/2022	SCHEMATIC DESIGN ISSUE	1/2	1/2	PP
2	07/07/2022	SCHEMATIC DESIGN ISSUE	1/2	1/2	PP
3	08/08/2022	ISSUED FOR COORDINATION	1/2	1/2	PP

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Drawn	GM	Date	MAY 2022
Checked	MR	Date	MAY 2022
Designed	1/2	Date	MAY 2022
Released	PP	Date	MAY 2022
Approved		Date	

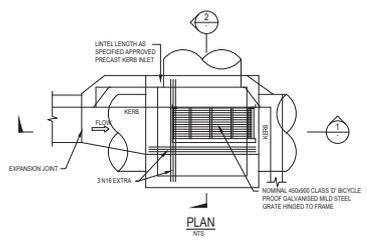
Client: **NSW DEPARTMENT OF EDUCATION**

Project: **CRONULLA HIGH SCHOOL CAPTAIN COOK DRIVE, CRONULLA NSW**

Discipline: **CIVIL SEDIMENTATION AND EROSION CONTROL DETAILS**

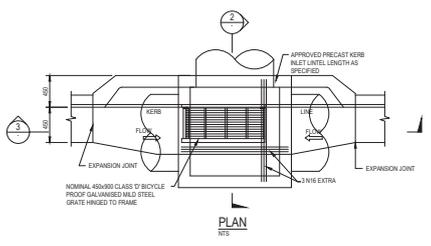
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Project Number	80821341-CI-0110		

DATE PLOTTED: 14.05.2022 10:59 AM USER: JORDAN.MEDARAC



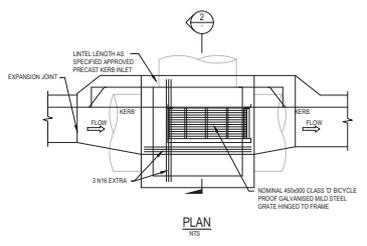
PLAN
KIP-A

KERB INLET PIT (KIP-A) (CORNER)



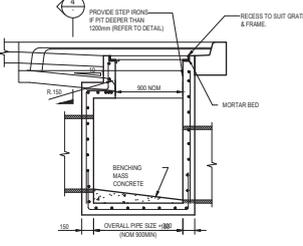
PLAN
KIP-B

KERB INLET PIT (KIP-B) (SAG)

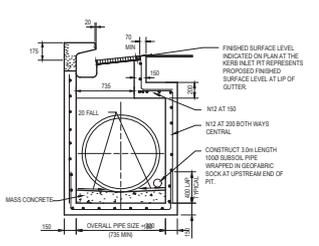


PLAN
KIP-C

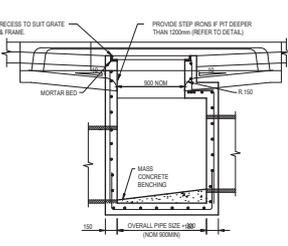
KERB INLET PIT (KIP-C) (ON GRADE)



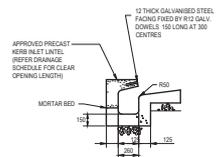
SECTION 1
SCALE 1:20



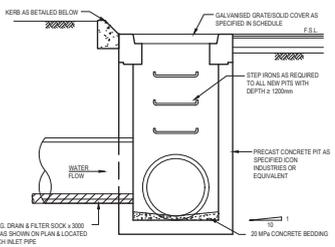
SECTION 2
SCALE 1:20



SECTION 3
SCALE 1:20



SECTION 4
SCALE 1:20



PRECAST STORMWATER PIT
TO BE INSTALLED AND BEDDED IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS. PROVIDE GALVANISED IRON STEP IRONS FOR PITS DEEPER THAN 1200mm.

KIP-A: 14/05/2022 10:59 AM; KIP-B: 14/05/2022 10:59 AM; KIP-C: 14/05/2022 10:59 AM; PRECAST STORMWATER PIT: 14/05/2022 10:59 AM

No.	Date	Description	By	App'd
1				
2				
3				
4	04/07/2022	SCHEMATIC DESIGN ISSUE	JJ	JJ
5	01/07/2022	ISSUED FOR COORDINATION	JJ	JJ
6				

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Drawn	Date	Checked	Date
SM	MAY 2022		
Designed		MR	MAY 2022
Designed		JJ	MAY 2022
Designed		PP	MAY 2022
Approved			

NSW DEPARTMENT OF EDUCATION
Project: CRONULLA HIGH SCHOOL
CAPTAIN COOK DRIVE, CRONULLA NSW
Title: CIVIL
STORMWATER STANDARD DEALS
SHEET 2

Status	Scale	Sheet
SCHEMATIC DESIGN NOT TO BE USED FOR CONSTRUCTION PURPOSES	1:200	A1
Drawing Number:	80821341-CI-0112	Revised:
		2